

LEGEND			
QTY	DESCRIPTION	PART #	FLOW PT.
4	2W	419348	2
5	1N	419346	1
4	1/2N	419345	1
2	230	419350	2
9	1F	419344	1
-	-	-	-

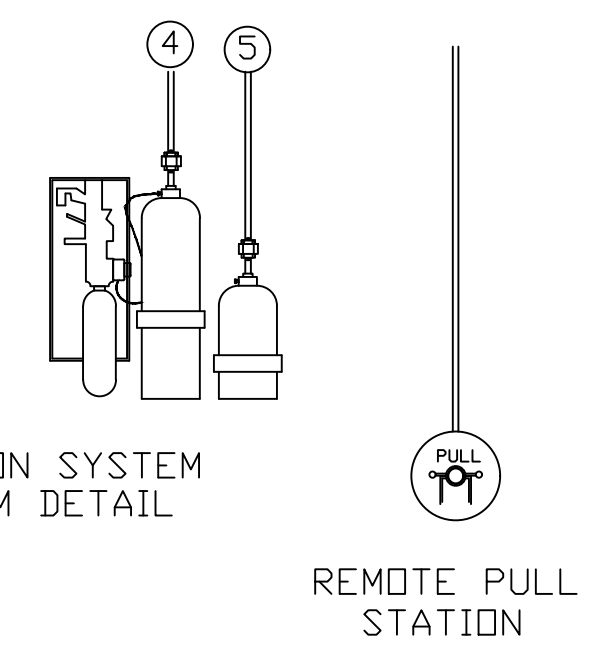
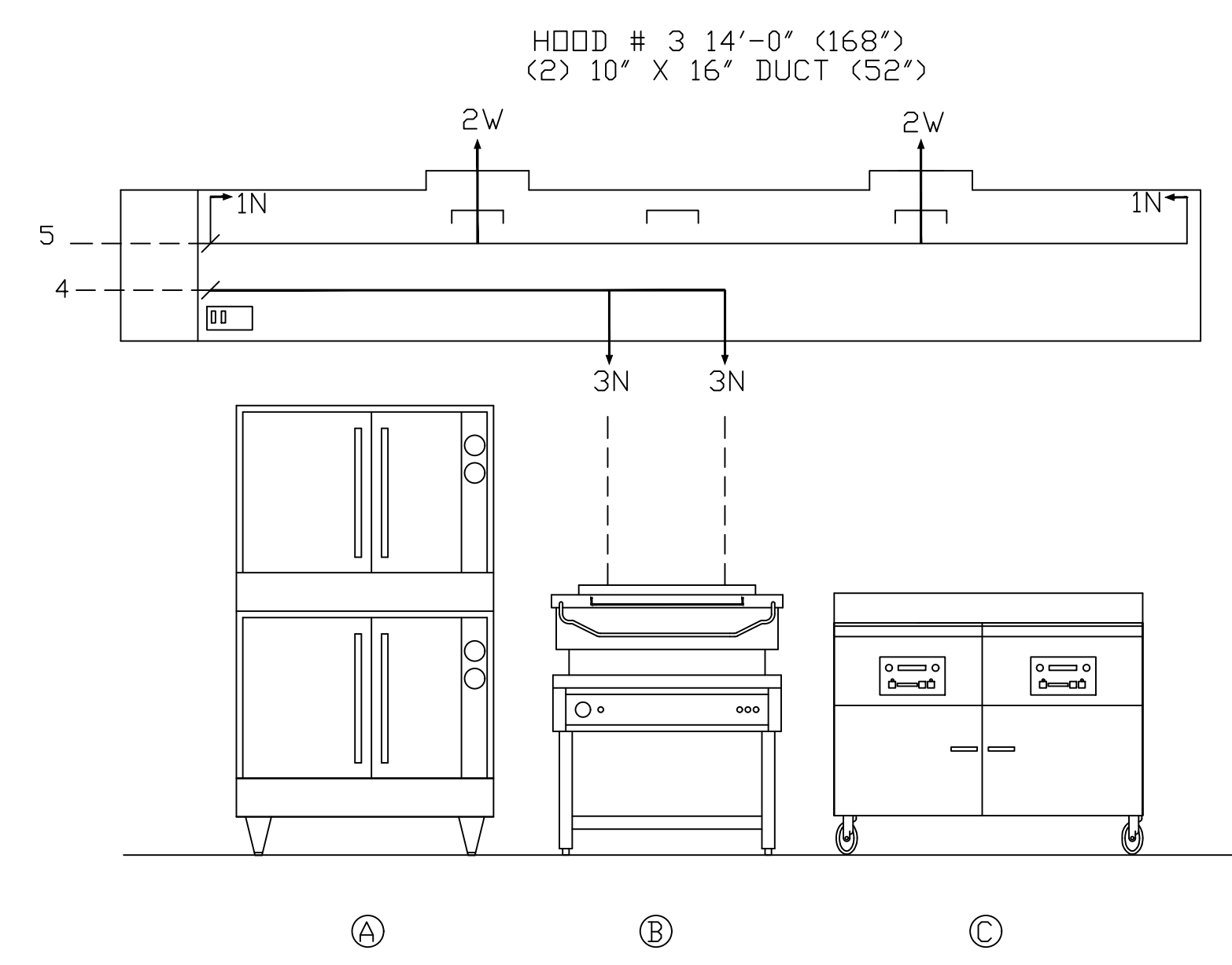
EQUIPMENT LEGEND	
QTY	DESCRIPTION
A	8 BURNER RANGE
B	PASTA STATION
C	2 BURNER RANGE
D	36" CHARBROILER
E	15" FRYER
F	60" CHEESE MELTER
G	18" CONV. OVEN
H	CONDIMENT HOLD

SUPPLY #1  
11 FLOW POINTS

SUPPLY #2  
9 FLOW POINTS

SUPPLY #3  
8 FLOW POINTS

TOTAL 27 FLOWS  
9.0 GAL. ANSUL SYSTEM



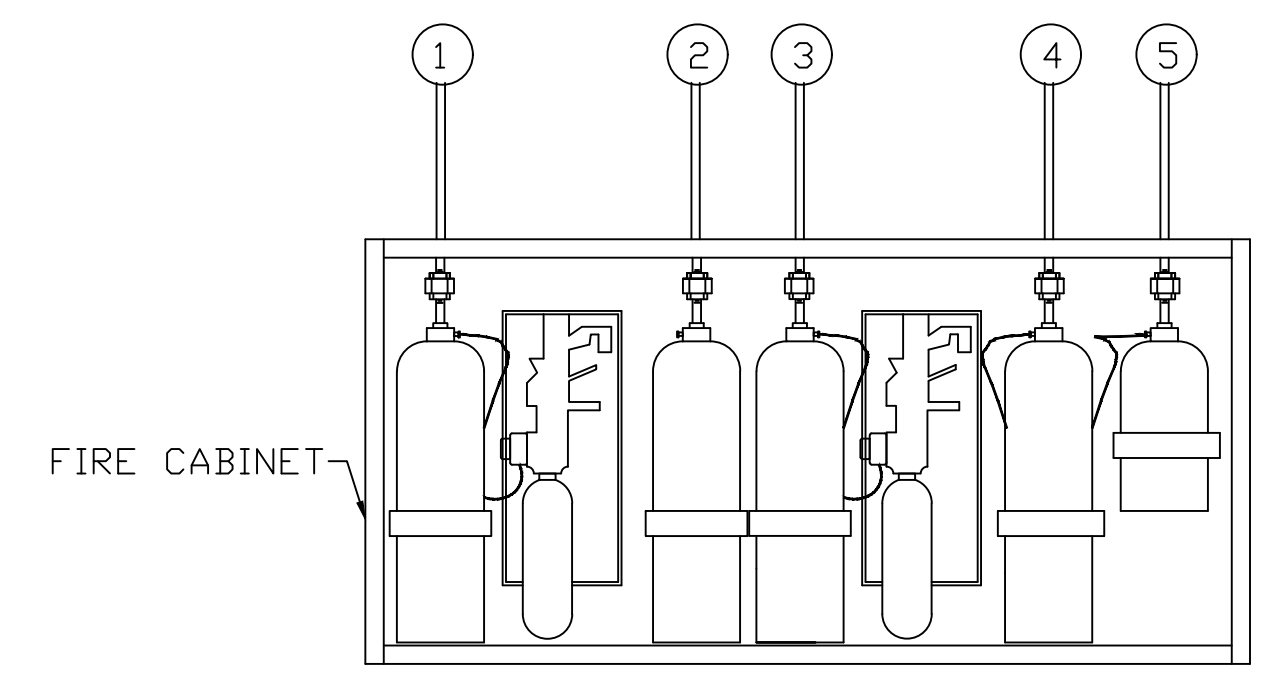
LEGEND			
QTY	DESCRIPTION	PART #	FLOW PT.
2	2W	419348	2
2	1N	419346	1
2	3N	419349	3
-	-	-	-

EQUIPMENT LEGEND	
QTY	DESCRIPTION
A	CONVECTION OVEN
B	TILT SKILLET
C	REETHERMALIZER

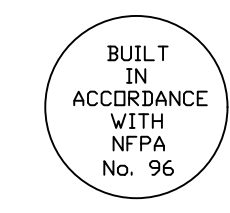
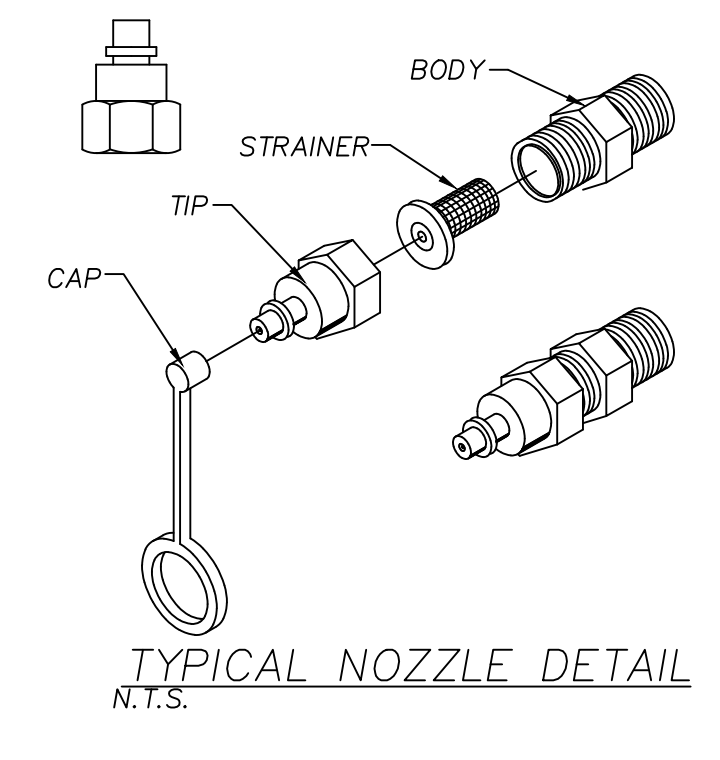
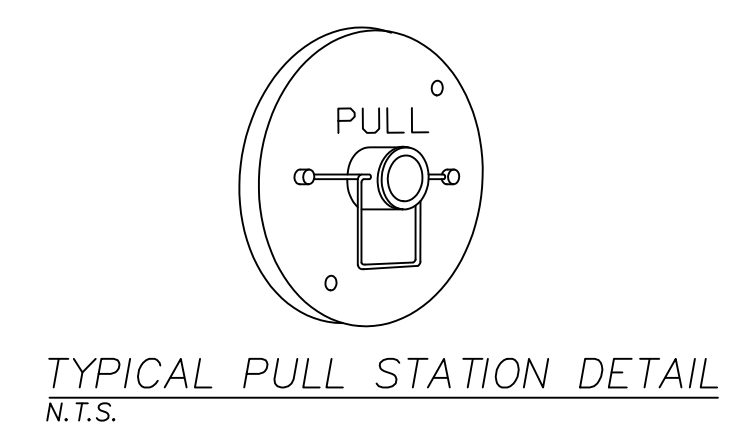
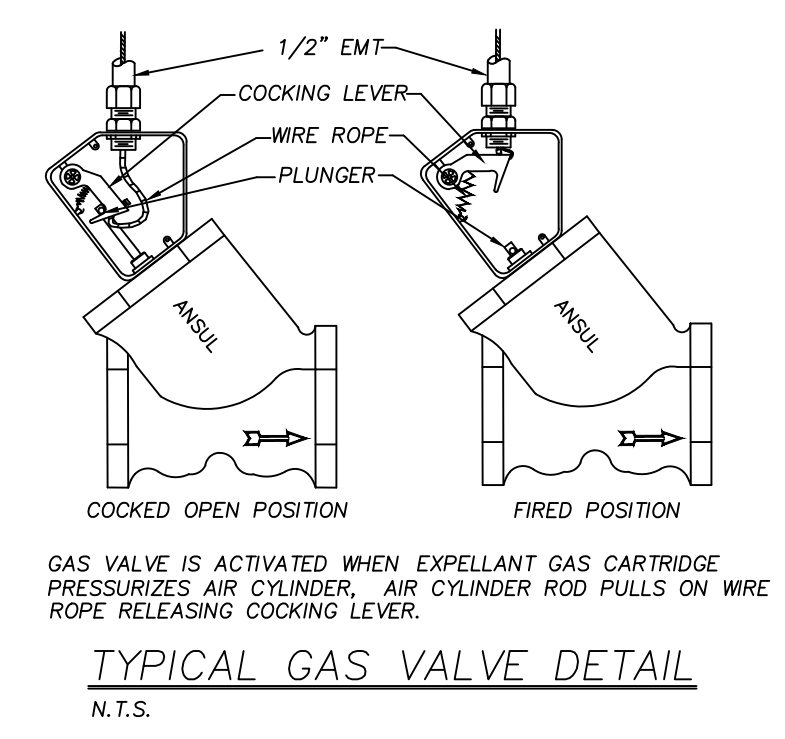
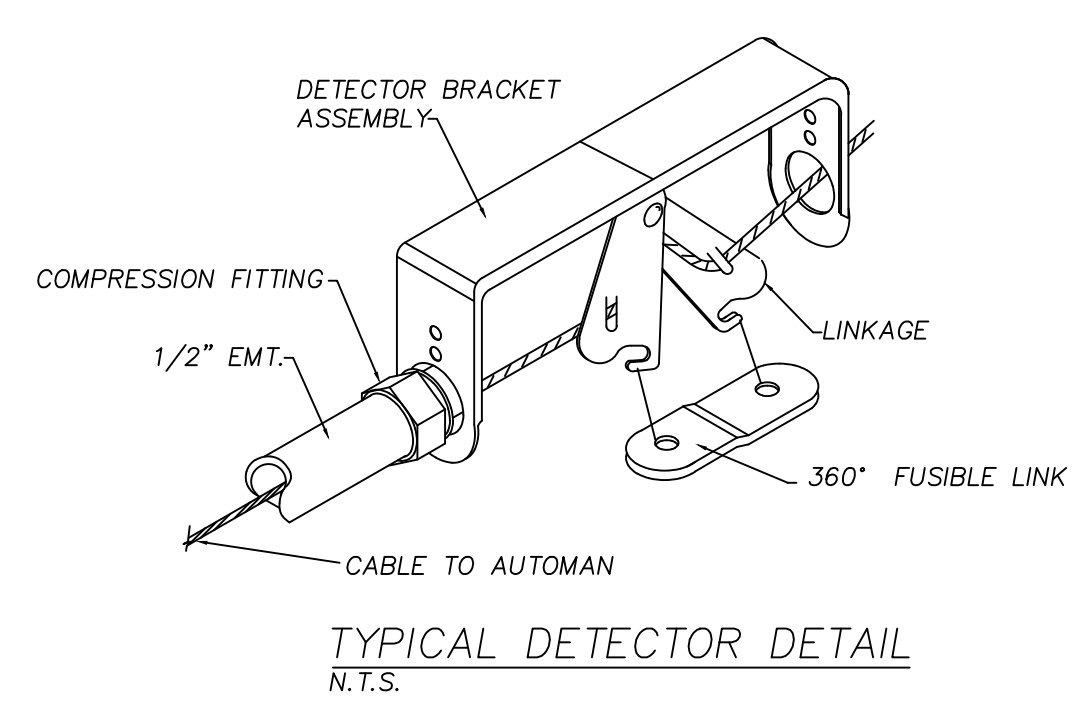
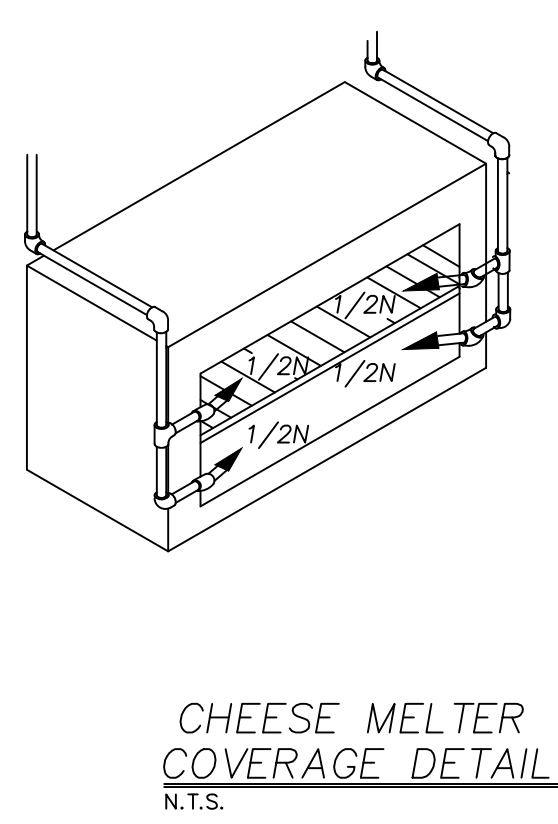
SUPPLY # 4  
6 FLOW POINTS

SUPPLY # 5  
6 FLOW POINTS

TOTAL 12 FLOWS  
4.5 GAL. ANSUL SYSTEM



NOTE:  
ALL PIPING IN 3/8" SCH. 40 BLACK PIPE  
EXHAUST FAN TO REMAIN ON, MAKEUP AIR, GAS AND ALL ELECTRIC TO SHUTDOWN WITH SYSTEM ACTUATION.  
NOZZLE PLACEMENT MEETS ANSUL UL300 CRITERIA  
NO HORIZONTAL PIPING WITH IN THE HOOD

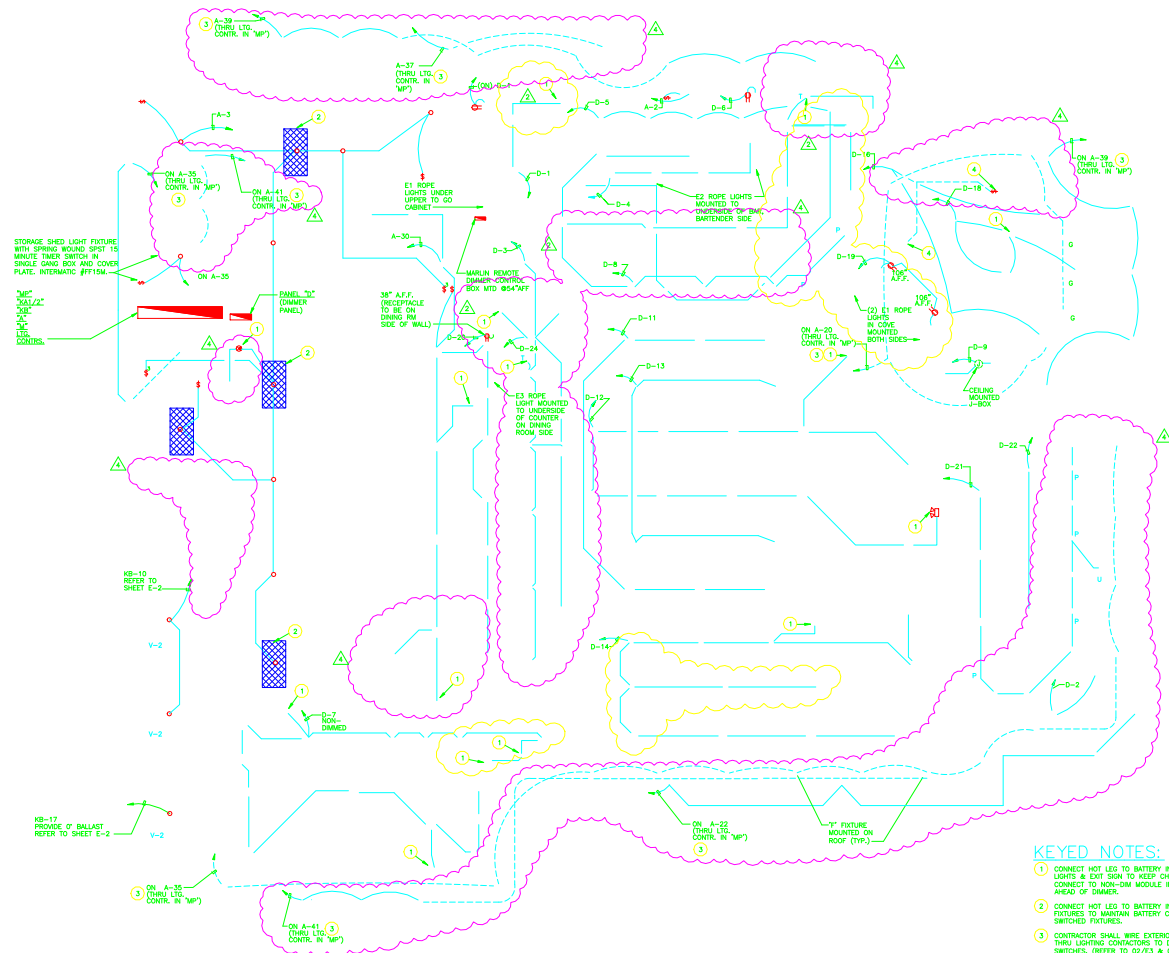


**GREASE MASTER**  
POST OFFICE BOX 309 INDIAN TRAIL, NC 28079  
TELEPHONE (704) 821-9211 FAX (704) 821-6618  
EMAIL - info@greasemaster.com

MODEL: **JOHNNY CARINO'S PROTO 9.0**

DATE 8/14/03	SIZE D	JOB NO. OR QUOTE NO.	REV -
DRAWN BY RB	SCALE NTS	SHEET 2 OF 2	

THIS DRAWING MAY BE USED IN WHOLE OR PART, PROVIDED THE GREASE MASTER NAME & LOGO ARE VISIBLE IN ANY REPRODUCTION.

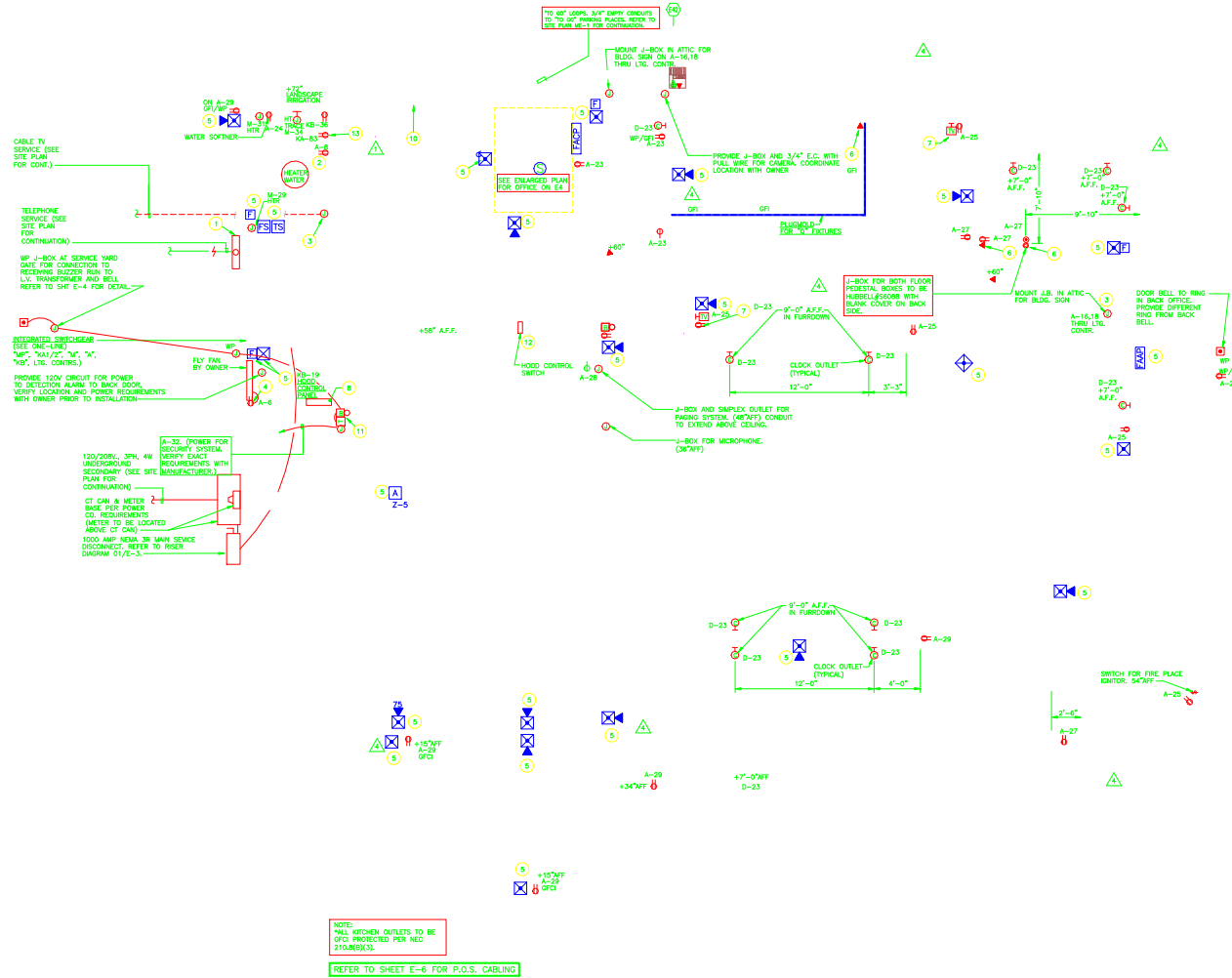


- KEYED NOTES:**
- 1 CONNECT HOT LEG TO BATTERY IN EMERGENCY LIGHTS & EXIT SIGN TO KEEP CHARGE ON BATTERY. CONNECT TO NON-DIM MODULE IN DIMMER PANEL TO AREA OF DIMMER.
  - 2 CONNECT HOT LEG TO BATTERY IN EMERGENCY FIXTURES TO MAINTAIN BATTERY CHARGE ON SWITCHED FIXTURES.
  - 3 CONTRACTOR SHALL WIRE EXTERIOR LIGHT FIXTURES THRU LIGHTING CONDUCTORS TO DIMMER PANEL TIME SWITCHES. (REFER TO 02/E3 & 02/E4).
  - 4 CONTRACTOR TO SWITCH TYPE "3"-3" LIGHT FIXTURE AT INSIDE OF EXTERIOR ACCESS DOOR CLOSEST TO LIGHT FIXTURE LOCATION. REFER TO PLAN ME.2 FOR ACCESS DOOR LOCATIONS.

**01 FLOOR PLAN - LIGHTING**  
 SCALE: 1/4" = 1'-0"

Xref: XTRBU50.dwg





NOTE:  
ALL KITCHEN OUTLETS TO BE  
GFCI PROTECTED PER NEC  
210.8(B)(3).  
REFER TO SHEET E-6 FOR P.O.S. CABLING

**KEYED NOTES:**

- 1) 12" W x 1 1/2" H x 6" D WP TELEPHONE SERVICE PULLBOX MOUNTED ON EXTERIOR WALL.
- 2) RECEPTACLE FOR RECIRCULATING PUMP. VERIFY LOCATION AND BOARDING HEIGHT.
- 3) POINT OF SETUP-UP FOR CABLE CONDUIT. (ONE (1) 2" E.C. WITH PULLBARE). VERIFY LOCATION. RUN TO PULL-BOX ABOVE CEILING. COORDINATE WITH CABLE CO.
- 4) RECEPTACLE FOR FLY FAN MOUNTED ABOVE DOOR. VERIFY BOARDING HEIGHT. MAKE ALL FINAL CONNECTIONS.
- 5) FIRE ALARM (ADVISED). REFER TO SHEET FS-1 FOR EXACT IDENTIFICATION, LOCATION AND MOUNTING HEIGHTS. E.C. WITH PULLBARE TO ABOVE CEILING. (TYPICAL)
- 6) 3/4" CONDUIT WITH PULLBARE TO EACH COMMUNICATION DEVICE. (TYPICAL)
- 7) RECEPTACLE AND TV OUTLET +84" AFF (VERIFY LOCATION WITH OWNER).
- 8) HOOD CONTROL PANEL. PROVIDE WIRING TO EX-1,2,3,4 & SF-11. CONNECT TO ASSES. FIRE SYSTEMS & HOOD FAN SWITCH. REFER TO FLOOR DRAWING FOR WIRING DIAGRAM.
- 9) 3/4" E.C. WITH PULLBARE TO ABOVE CEILING (FOR SOUND SYSTEM CONTROL).
- 10) DISH MACHINE (EV-5) TO BE INTERLOCKED WITH HOOD CONTROL SWITCH FOR ON/OFF OPERATION. COORDINATE LOCATION OF DISCONNECTS, WITH EQUIPMENT MANUFACTURER.
- 11) LV. TRANSFORMER AND SERVICE BLOWER WALL MOUNT AT 84" AFF. FURNISH AND INSTALL CONTROL WIRING IN CONDUIT AND ALL FINAL CONNECTIONS TO PUSH-BUTTON AT RECEIVING GATE.
- 12) FUSE SWITCH ON HOOD. PROVIDE WIRING TO HOOD CONTROL PANEL.
- 13) GFI RECEPTACLE TO OWNER PROVIDED METER.

**GENERAL NOTES:**

- A. A COMPLETE FIRE ALARM SYSTEM AS REQUIRED BY NEPA AND LOCAL FIRE MARSHALL TO BE PROVIDED BY OWNER UNDER SEPARATE CONTRACT. THE ELECTRICAL CONTRACTOR SHALL PROVIDE AND INSTALL ALL ROUGH-IN PER FIRE ALARM CONTRACTOR'S SHOP DRAWING.
- B. REFER TO "K" DRAWINGS FOR ADDITIONAL KITCHEN EQUIPMENT INFORMATION. VERIFY ALL ELECTRICAL REQUIREMENTS AND DEVICE LOCATION AND TYPE WITH EQUIPMENT SUPPLIER PRIOR TO ORDERING. FURNISH AND INSTALL POWER AND CONTROL WIRING AND ALL FINAL CONNECTIONS AS REQUIRED. VERIFY ALL MOUNTING HEIGHTS AND TYPE OF CONNECTION PRIOR TO FINAL BIDS. REFER TO KITCHEN ELECTRICAL ROUGH-IN SCHEDULE FOR CIRCUITS WIRE AND CONDUIT.
- C. FURNISH AND INSTALL ALL HOOD ELECTRICAL POWER AND CONTROL REQUIREMENTS. WIRING IN CONDUIT, AND ALL FINAL CONNECTIONS FOR COMPLETE SYSTEM, INCLUDING FINE SUPPRESSION. VERIFY WITH MECHANICAL CONTRACTOR.
- D. FURNISH AND INSTALL ALL POWER AND CONTROL REQUIREMENTS INCLUDING ALL EFC AND SF INTERLOCKING, CONTROLS, SWITCHES, RELAYS, ETC. MOUNT AND CONNECT ALL T-STAYS. COORDINATE WITH MECHANICAL CONTRACTOR.
- E. NO EXTRA OR ALLOWANCES WILL BE ALLOWED THE ELECTRICAL CONTRACTOR FOR FAILURE TO PROPERLY COORDINATE ALL POWER AND CONTROL REQUIREMENTS OF ASSOCIATED SUB-CONTRACTORS. VERIFY PRIOR TO FINAL BIDS ALL GAO REQUIREMENTS.
- F. FURNISH AND INSTALL ALL DISCONNECTS, J-BOXES, POWER AND CONTROL WIRING IN CONDUIT AND ALL FINAL CONNECTIONS TO FIVE MACHINE REMOTE ROOF-MOUNTED CONDENSER. VERIFY LOCATION.
- G. ALL COUNTERTOP RECEPTACLES SHALL BE MOUNTED HORIZONTALLY IN BACKSPASH. GENERAL CONVENTION RECEPTACLES SHALL BE MOUNTED VERTICALLY 15" TO CENTER LINE. ALL COVER PLATES ON STAINLESS OR DARK COLORED WALLS SHALL BE DARK BROWN COLORED. ALL KITCHEN EQUIPMENT COVERPLATES SHALL BE STAINLESS STEEL.
- H. ELECTRICAL CONTRACTOR SHALL PROVIDE AND INSTALL ALL ROUGH-IN REQUIRED FOR SECURITY SYSTEM (BY OTHERS). ELECTRIC DOOR STRIKE TO BE CONTROLLED BY KEYS. MOMENTARY SWITCH AND WIRING TO EXIT ALARM AT RECEIVING DOOR. IF OTHER GATE OR DOOR HAS AN UNAUTHORIZED OPENING, ALARM BELL AND PUSH BUTTON USED FOR STANDBY ACCESS SHALL BE USED AS MANUATOR FOR ALARM MODE. COORDINATE WITH ARCHITECT AND SECURITY SYSTEM INSTALLER.
- I. PROVIDE A 2" Ø ROUGHMET HOLE IN COUNTER FOR WIRING, TYPICAL EXCEPT AS SHOWN.



**ELECTRICAL SYMBOLS**

SYMBOL	DESCRIPTION
FLUORESCENT FIXTURE	FLUORESCENT FIXTURE
RECEPTACLE	RECEPTACLE
SWITCH	SWITCH
...	...
RECEPTACLE FOR FAX MACHINE (+42)	RECEPTACLE FOR FAX MACHINE (+42)
...	...

NOTE: NOT ALL SYMBOLS MAY APPEAR ON DRAWINGS

**KITCHEN ELECTRICAL ROUGH-IN SCHEDULE**

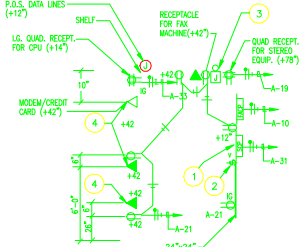
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1	...	1	...	...	...	...	...
2	...	1	...	...	...	...	...
3	...	1	...	...	...	...	...
4	...	1	...	...	...	...	...
...	...	...	...	...	...	...	...
19	...	1	...	...	...	...	...
20	...	1	...	...	...	...	...
...	...	...	...	...	...	...	...
40	...	1	...	...	...	...	...
41	...	1	...	...	...	...	...
42	...	1	...	...	...	...	...
...	...	...	...	...	...	...	...

...

**LIGHTING SCHEDULE**

ITEM NO.	DESCRIPTION	QUANTITY	UNIT	MARK	DATE	BY	CHK'D
1	...	1	...	...	...	...	...
2	...	1	...	...	...	...	...
3	...	1	...	...	...	...	...
4	...	1	...	...	...	...	...
...	...	...	...	...	...	...	...
19	...	1	...	...	...	...	...
20	...	1	...	...	...	...	...
...	...	...	...	...	...	...	...
40	...	1	...	...	...	...	...
41	...	1	...	...	...	...	...
42	...	1	...	...	...	...	...
...	...	...	...	...	...	...	...

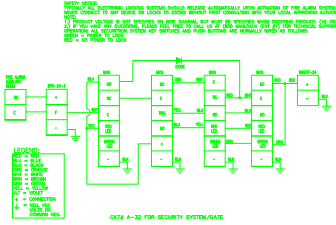
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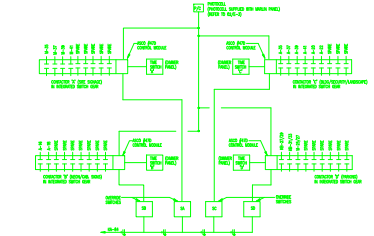
**KEYED NOTES:**

1. SECURITY ALARM CONTROL PANEL AND SENSORS TO BE INSTALLED AS SHOWN ON SHEET 17-01. ALL SENSORS TO BE MOUNTED TO THE CEILING UNLESS SHOWN OTHERWISE.
2. SECURITY ALARM CONTROL PANEL AND SENSORS TO BE INSTALLED AS SHOWN ON SHEET 17-01. ALL SENSORS TO BE MOUNTED TO THE CEILING UNLESS SHOWN OTHERWISE.
3. SECURITY ALARM CONTROL PANEL AND SENSORS TO BE INSTALLED AS SHOWN ON SHEET 17-01. ALL SENSORS TO BE MOUNTED TO THE CEILING UNLESS SHOWN OTHERWISE.
4. SECURITY ALARM CONTROL PANEL AND SENSORS TO BE INSTALLED AS SHOWN ON SHEET 17-01. ALL SENSORS TO BE MOUNTED TO THE CEILING UNLESS SHOWN OTHERWISE.

**OFFICE PLAN - ELECTRICAL**  
SCALE: 1/8" = 1'-0"

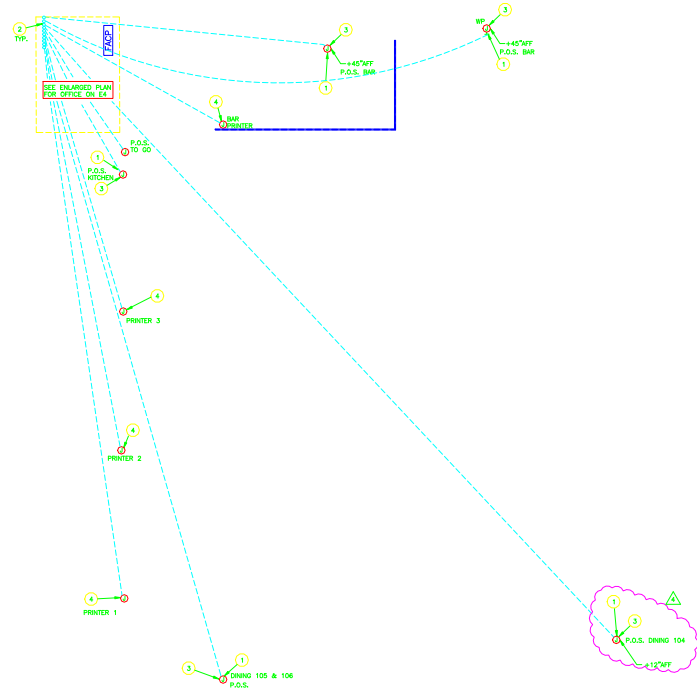


**1 SECURITY GATE CONTROL DETAIL**  
NO SCALE



**2 EXTERIOR LIGHTING CONTROL DETAIL**  
NO SCALE





**KEYED NOTES:**

- 1 3/4" FERROUS METAL E.G. WITH PULL WIRE FOR POS SYSTEM CABLE. CABLES SHALL BE FROM EACH POS TO A HORIZONTAL TO THE OFFICE. CONDUITS SHOULD BE PLACED IN THE SLAB AND STUBBED UP INSIDE A WALL.
- 2 STUB CONDUITS TO ABOVE CEILING INTO TAP CAN. PROVIDE 2" CONDUIT FROM TAP CAN DOWN OFFICE WALL. TERMINATE INTO QUAD JUNCTION BOX. REFER TO 3/2-4 FOR LOCATION OF J-BOX.
- 3 EACH POS STATION TO HAVE A WALL MTD. DUPLEX I.G. RECEPTACLE AND DOUBLE GANG METALLIC J-BOX WITH SINGLE-GANG PLASTER RING.
- 4 EACH PRINTER TO HAVE A SIMPLEX I.G. RECEPTACLE AND DOUBLE GANG METALLIC J-BOX WITH SINGLE-GANG PLASTER RING.

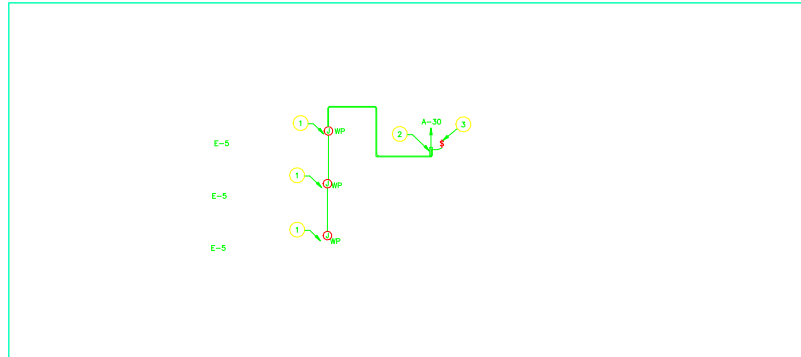
**GENERAL NOTES:**

- A ALL COUNTERTOP RECEPTACLES SHALL BE MOUNTED HORIZONTALLY IN BACKSPASH. GENERAL CONVENIENCE RECEPTACLES SHALL BE MOUNTED VERTICALLY 18" TO CENTER LINE. ALL COVER PLATES ON STAINED OR DARK COLORED WALLS SHALL BE DARK BROWN COLORED. ALL KITCHEN EQUIPMENT COVERPLATES SHALL BE STAINLESS STEEL.
- B PROVIDE #12 ISOLATED GROUND WIRE TO POS/PRINTER OUTLETS.
- C ALL J-BOXES FOR PRINTERS AND POS ARE TO BE MOUNTED ON THE BACK SIDE OF THE COUNTER WALL. AT 34" AFF UNLESS NOTED OTHERWISE.
- D PROVIDE A 2" GROMMET HOLE IN COUNTER FOR WIRING, TYPICAL EXCEPT AT BAR.

**01 FLOOR PLAN -- P.O.S.**  
SCALE 1/4" = 1'-0"

REFER TO SHEET E-2 FOR EQUIPMENT LOCATIONS/DESIGNATIONS AND ELECTRICAL DEVICES RELATED TO P.O.S. SYSTEM

xref: XTBLX50.dwg

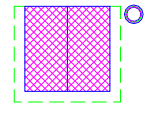
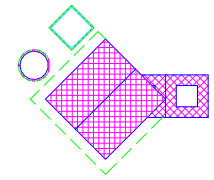
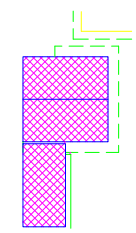
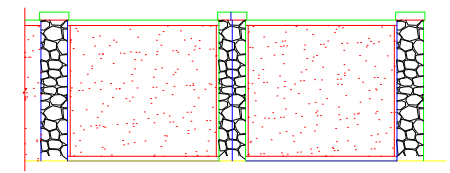
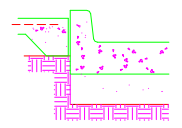
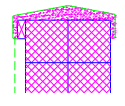
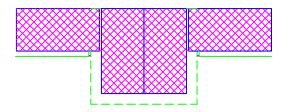
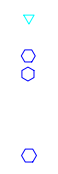
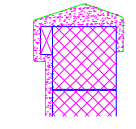
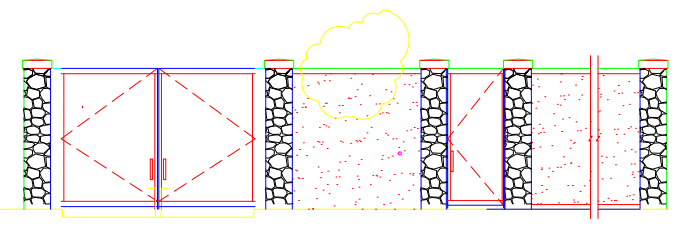
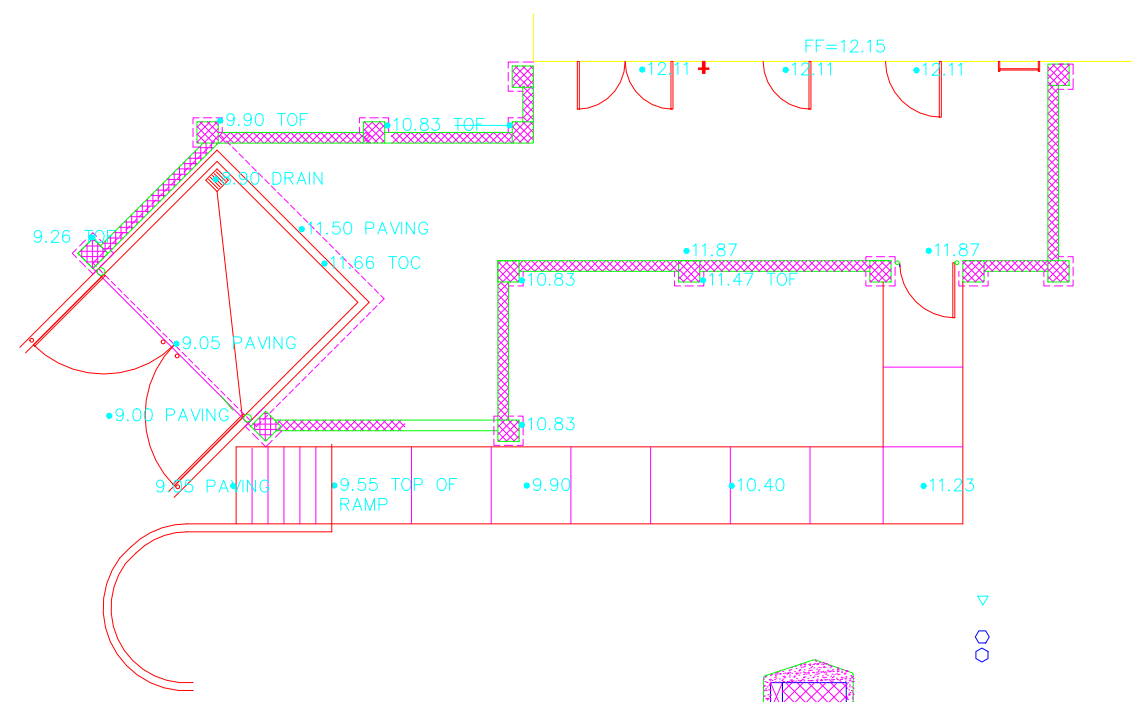
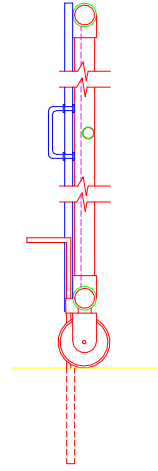


**01 FLOOR PLAN -- PATIO -- ELECTRICAL**  
 SCALE: 1/4" = 1'-0"

KEYED NOTES:

- ① WEATHER-PROOF JUNCTION BOX FOR CONNECTION TO ROPE LIGHTING. KEEP CONDUIT TIGHT TO PATIO STRUCTURE AND CONCEAL CONDUIT AND JUNCTION BOXES.
- ② PROVIDE SLEEVE FOR PENETRATION THROUGH BUILDING UP HIGH NEAR PATIO STRUCTURE. SEAL AND CAULK ALL OPENING AS REQUIRED FOR MAINTAINING A WEATHER TIGHT SEAL.
- ③ PROVIDE IN-WALL TIMER, 20 AMP., SINGLE POLE INTERMATIC TIMER #E120C WITH TAMPER-PROOF LOCKABLE COVER.

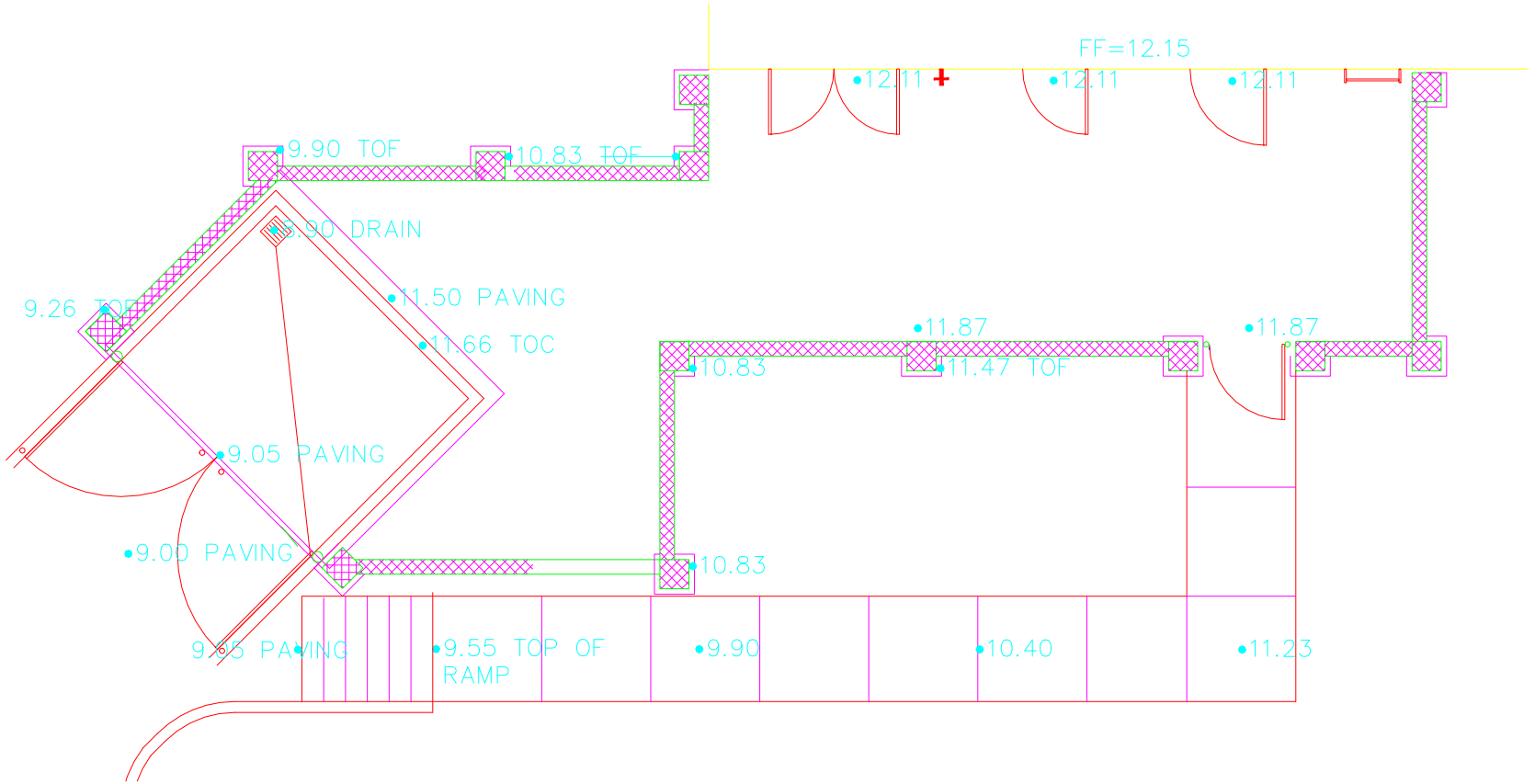
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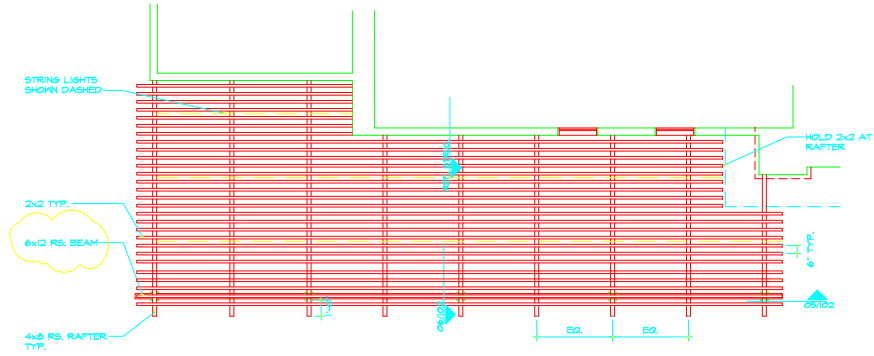
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0334

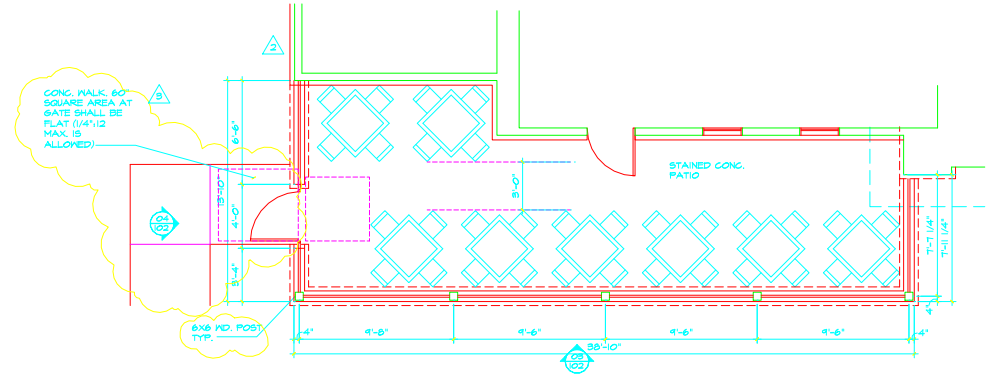
JOHNNY CARINO'S CASA  
GRANDE, AZ





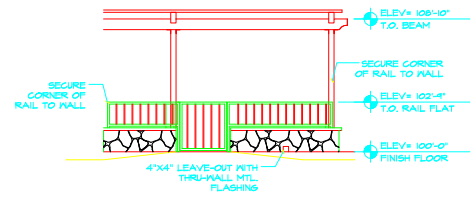


**02 PATIO FRAMING PLAN**  
1/4"=1'-0"

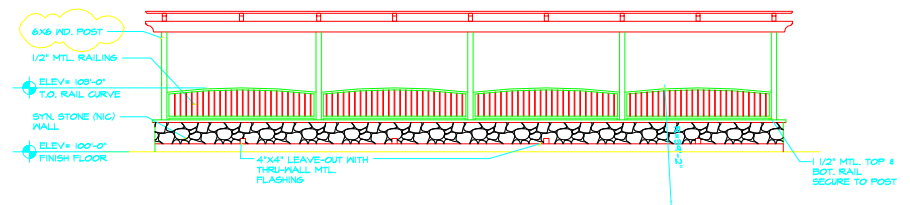


**01 PATIO FLOOR PLAN**  
1/4"=1'-0"

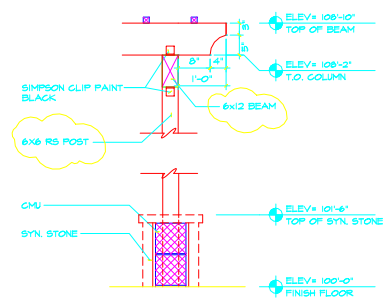
△ CITY COMMENTS  
12 NOV. 2008  
△ CITY COMMENTS  
16 DEC. 2008



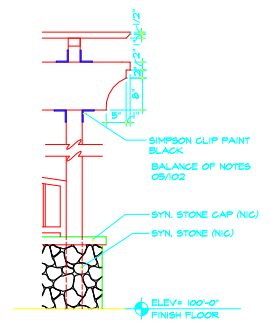
**04 PATIO ELEVATION**  
1/4"=1'-0"



**03 PATIO ELEVATION**  
1/2"=1'-0"

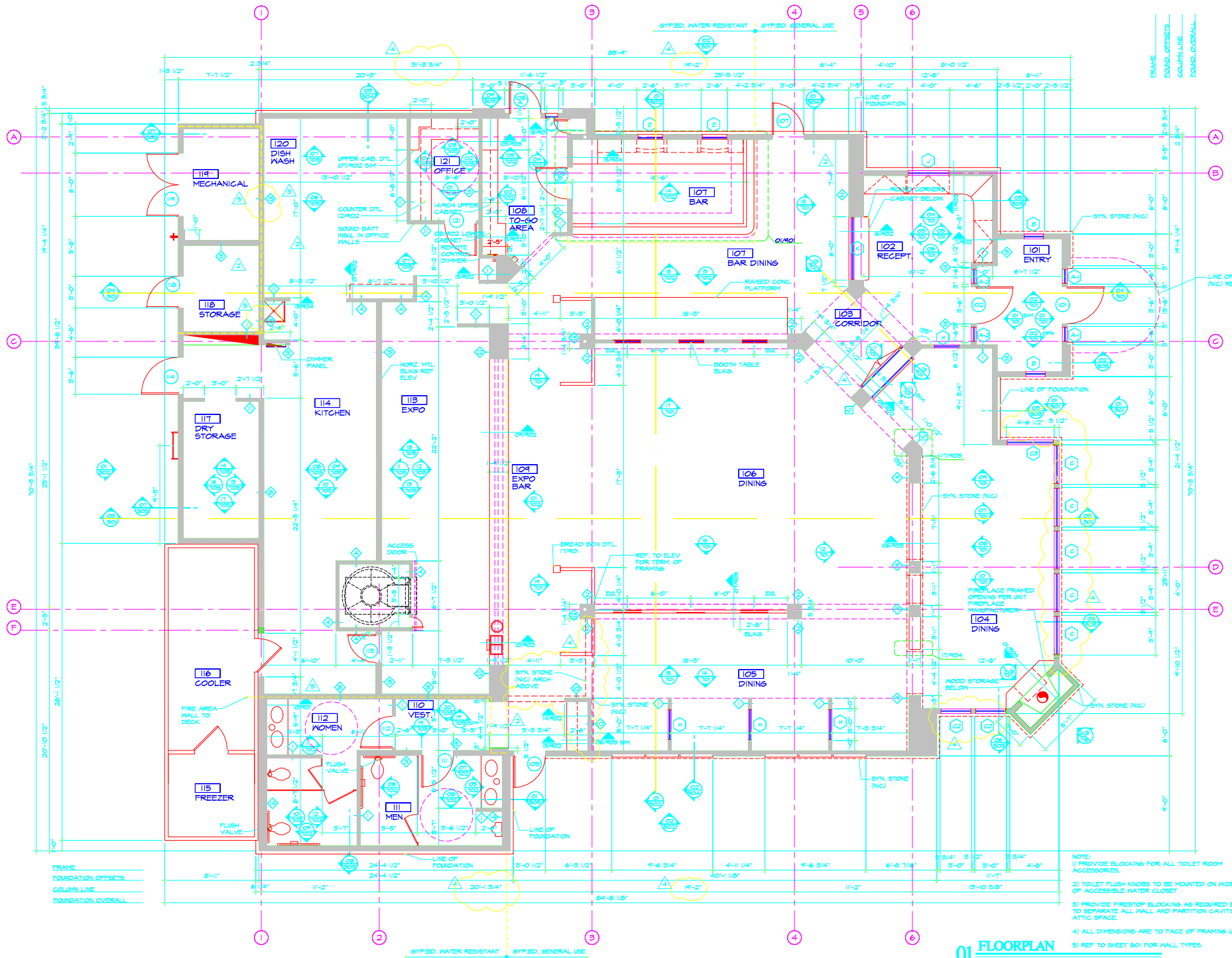


**06 PATIO COLUMN DETAIL**  
3/4"=1'-0"



**05 PATIO COLUMN DETAIL**  
3/4"=1'-0"

APPROVED BY: \_\_\_\_\_  
 DATE: \_\_\_\_\_



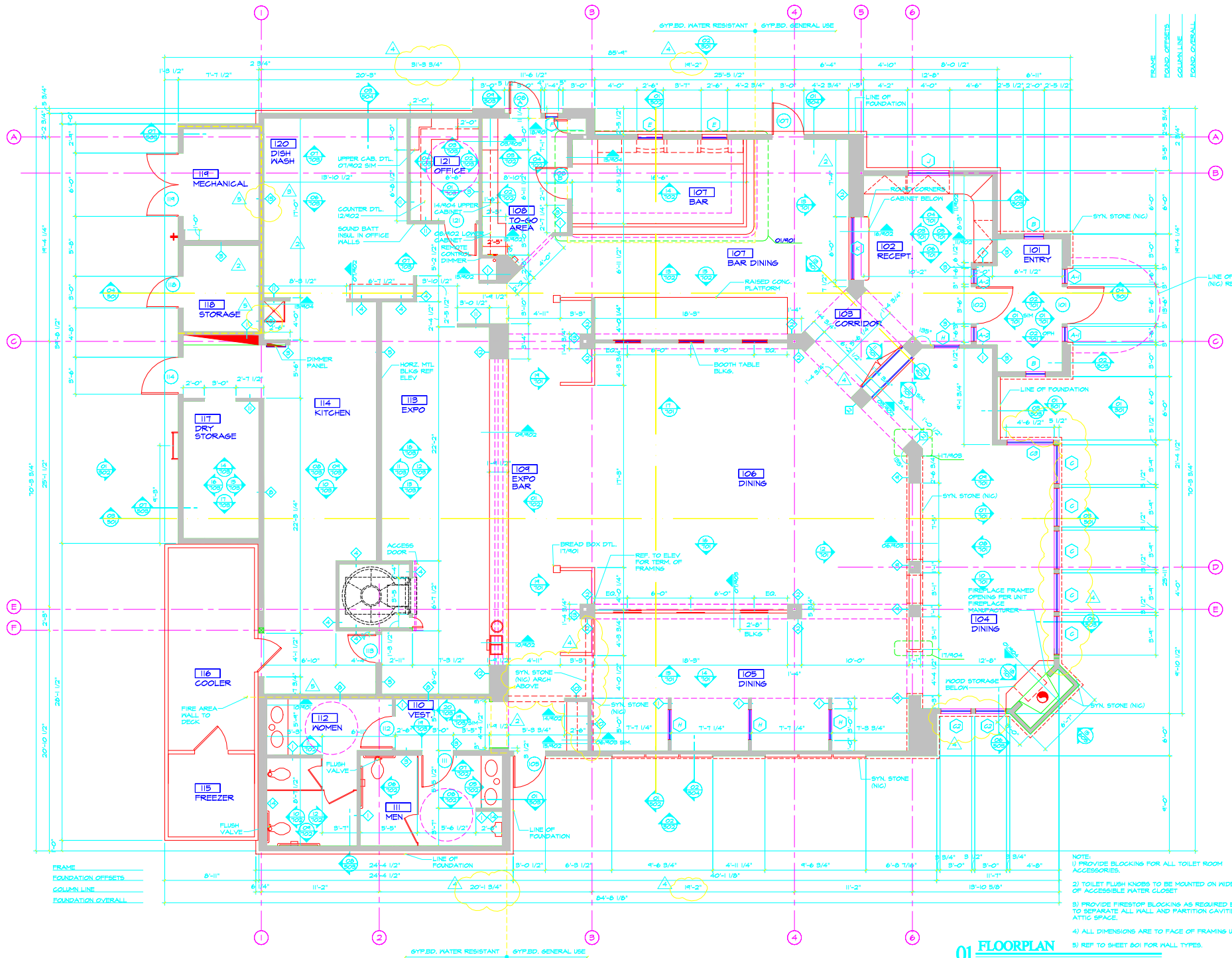
- ▲ CITY COMMENTS 12 NOV. 2003
- ▲ CITY COMMENTS 16 DEC. 2003
- ▲ 28 JAN. 2004 OWNER COMMENTS
- ▲ 28 JAN. 2004 CITY COMMENTS

- NOTE:
- 1) PROVIDE BLOCKING FOR ALL TOILET ROOM ACCESSORIES.
  - 2) TOILET FLUSH KNOBS TO BE MOUNTED ON WIDE SIDE OF ACCESSIBLE WATER CLOSET
  - 3) PROVIDE FIRESTOP BLOCKING AS REQUIRED BY CODE TO SEPARATE ALL WALL AND PARTITION CAVITIES FROM ATTIC SPACE.
  - 4) ALL DIMENSIONS ARE TO FACE OF FRAMING UNL.
  - 5) REF. TO SHEET 001 FOR WALL TYPES.

01 FLOORPLAN  
 1/4"=1'-0"

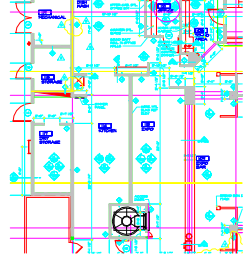
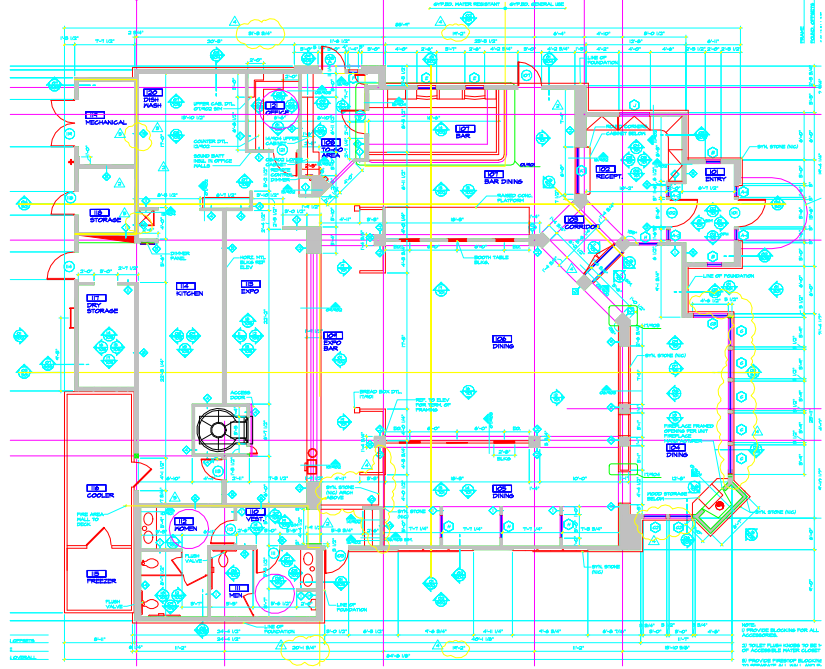
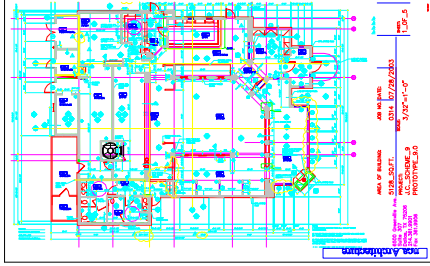


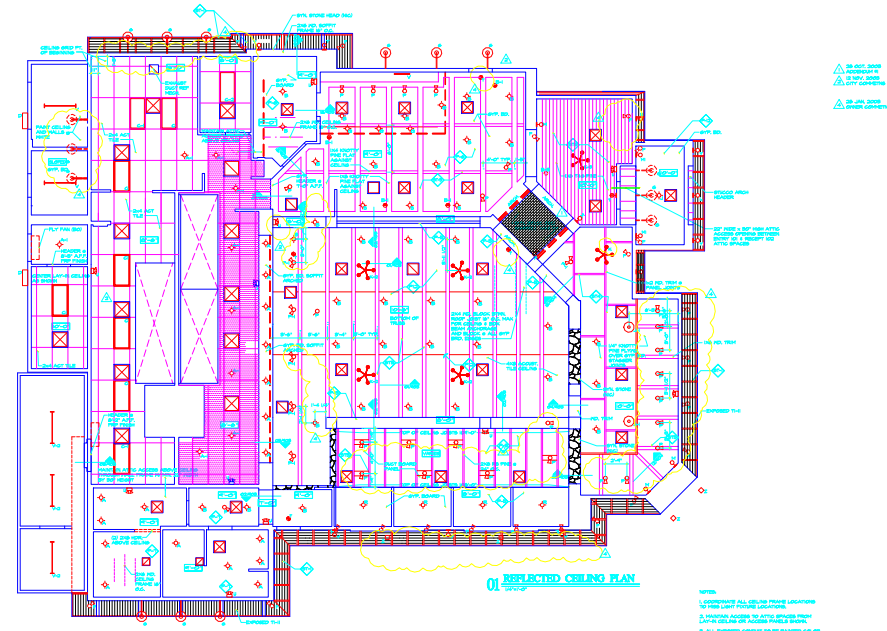
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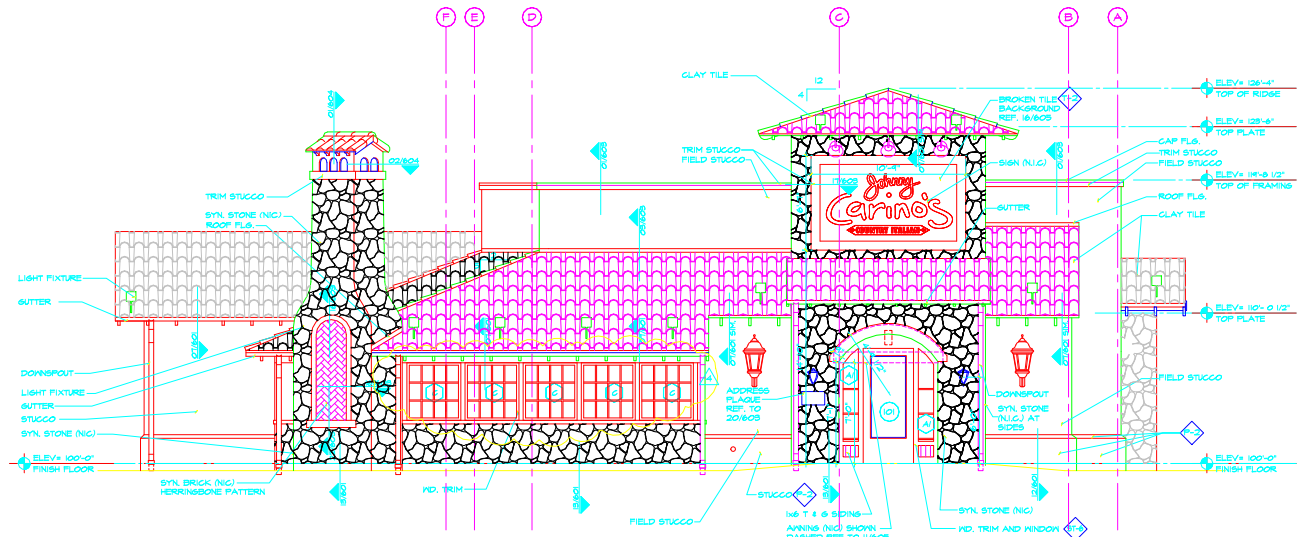


- ▲ CITY COMMENTS 12 NOV. 2003
- ▲ CITY COMMENTS 16 DEC. 2003
- ▲ 28 JAN. 2004 OWNER COMMENTS
- ▲ 28 JAN. 2004 CITY COMMENTS

- NOTE:
- 1) PROVIDE BLOCKING FOR ALL TOILET ROOM ACCESSORIES.
  - 2) TOILET FLUSH KNOBS TO BE MOUNTED ON WIDE SIDE OF ACCESSIBLE WATER CLOSET
  - 3) PROVIDE FIRESTOP BLOCKING AS REQUIRED BY CODE TO SEPARATE ALL WALL AND PARTITION CAVITIES FROM ATTIC SPACE.
  - 4) ALL DIMENSIONS ARE TO FACE OF FRAMING UNL.
  - 5) REF TO SHEET 001 FOR WALL TYPES.







TYPICAL FINISHES UNLESS NOTED OTHERWISE

FIELD STUCCO	P-1
TRIM/SILL STUCCO	P-2
SYNTHETIC STONE	SN-1
EXTERIOR WOOD TRIM	ST-1
EXTERIOR DOORS	ST-6
EXTERIOR WINDOWS AND FRAME	P-10
WINDOW SHUTTERS	P-5
PARAPET GAP FLASHING	P-2
ROOF FLUS-SLOPE ROOF TO PARAPET	P-1
ROOF FLUS-SLOPE ROOF TO STONE	P-2
RAFTER TAILS, FASCIA AND SOFFIT BELOW	ST-1
COMP. SHINGLE & CLAY TILE ROOF	ST-1
ROOF MOUNTED LIGHTS	P-4
LOUVERS/VENTS MATCH ADJACENT WALL FINISH	ST-1
GUTTERS/DOWNSPOUTS GALVANIZED FINISH	

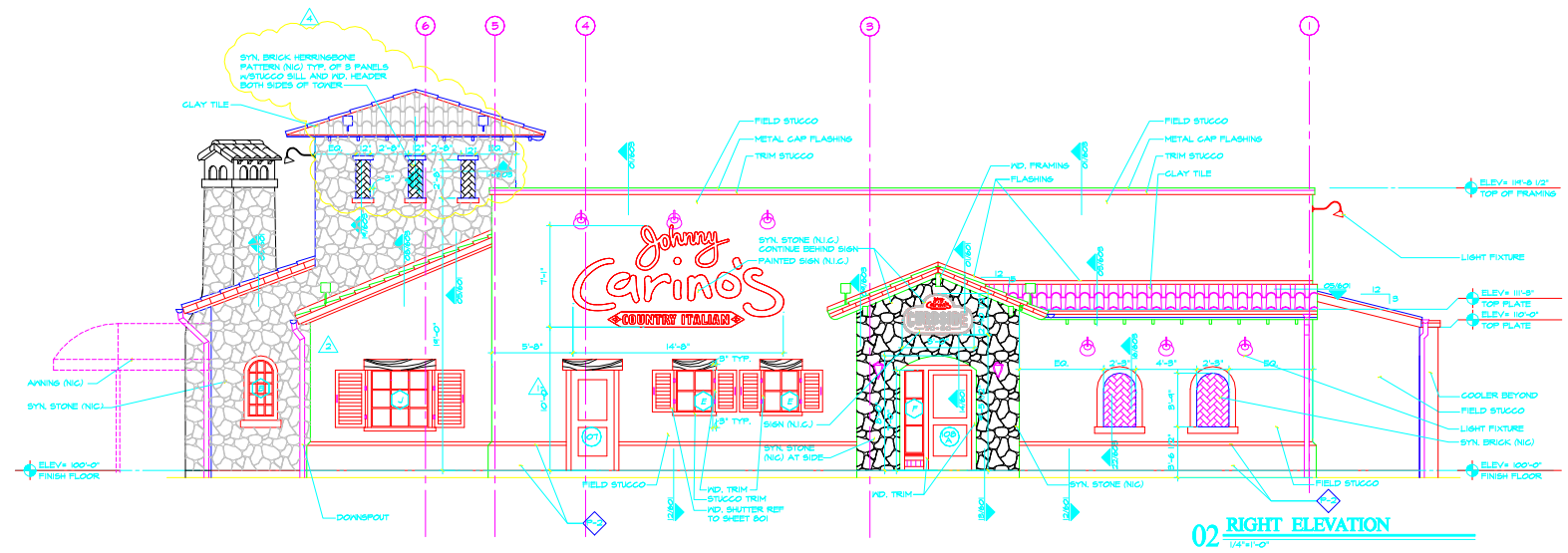
GENERAL NOTES

1. ALL DIMENSIONS ARE TO ROUGH FRAME UNLESS NOTED OTHERWISE.
2. ALL FLASHING TO BE PAINTED TO MATCH ADJACENT WALL PAINT COLOR UNLESS NOTED OTHERWISE.

- ▲ 28 OCT. 2009 ADDENDUM #1
- ▲ 12 NOV. 2009 CITY COMMENTS
- ▲ 28 JAN. 2004 OWNER COMMENTS

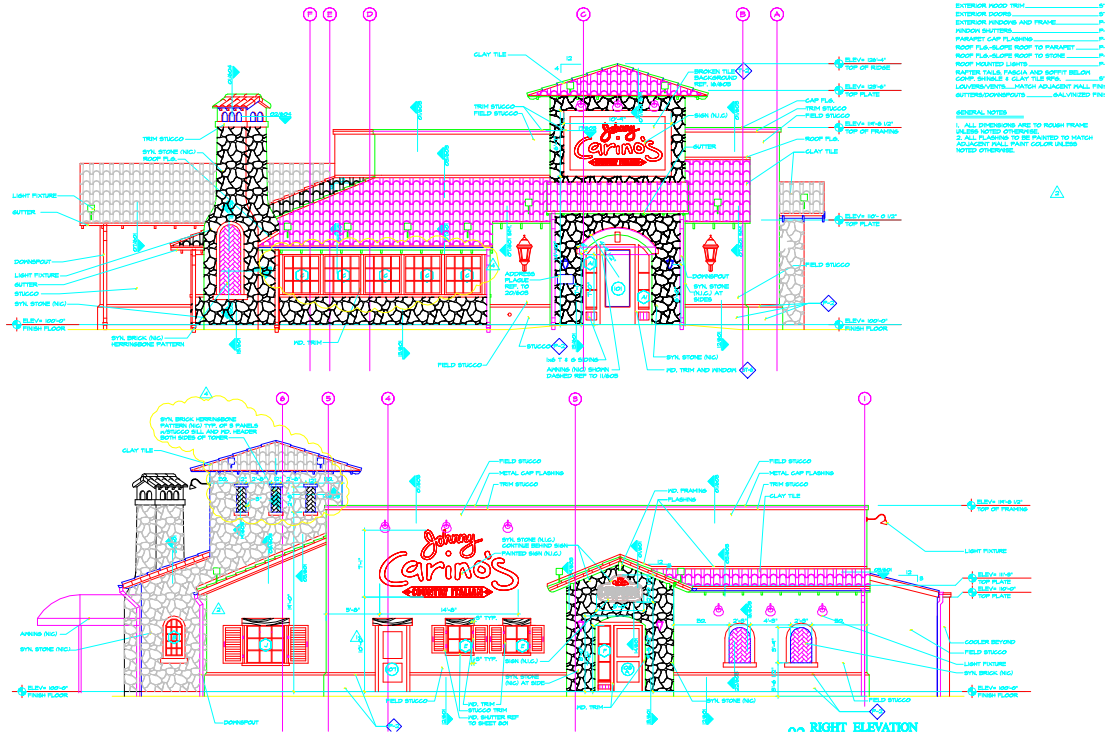
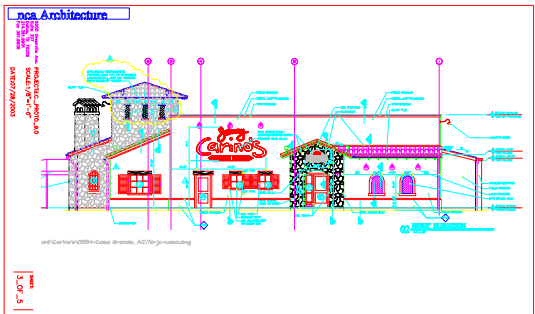
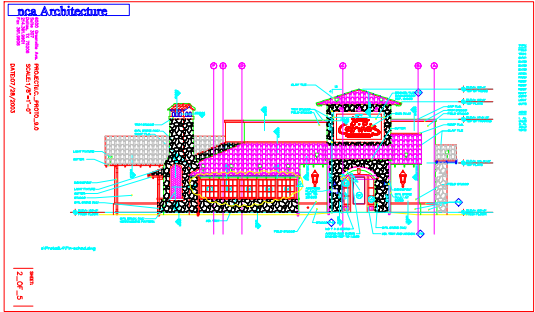
Xref Z:\Restaurant\Carino's\ProtoB.4\Fin-sched.dwg

**01 FRONT ELEVATION**  
1/4"=1'-0"



**02 RIGHT ELEVATION**  
1/4"=1'-0"

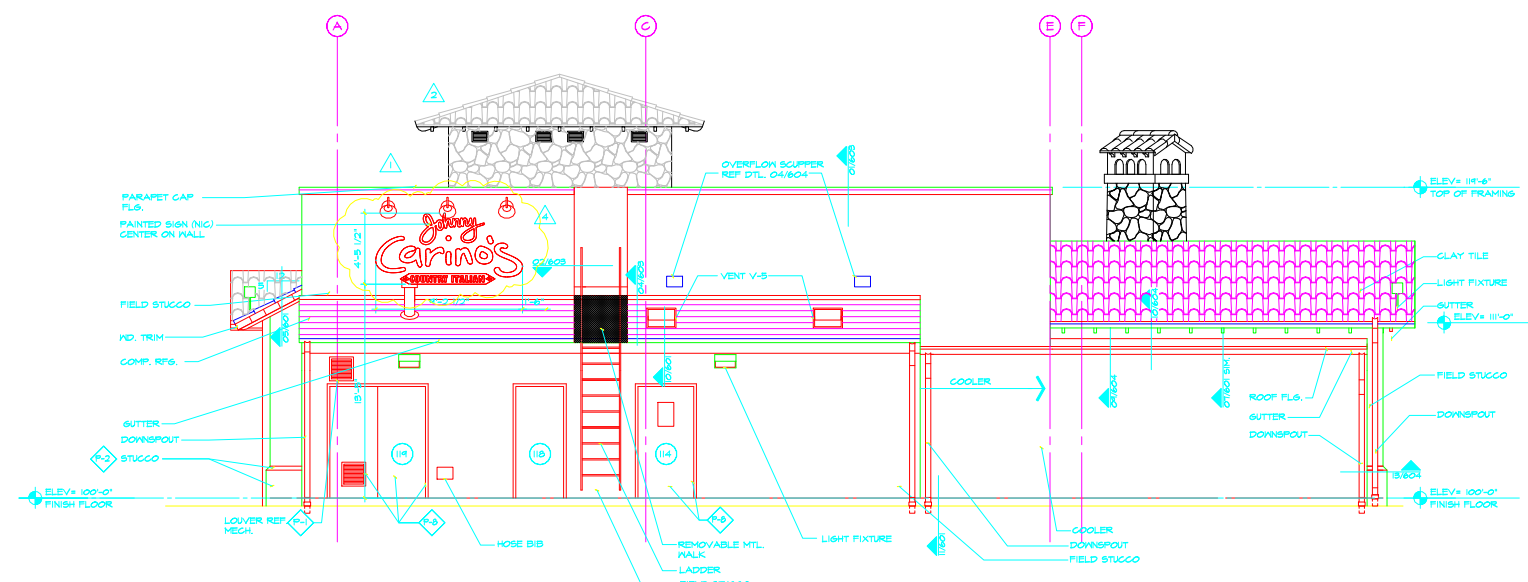
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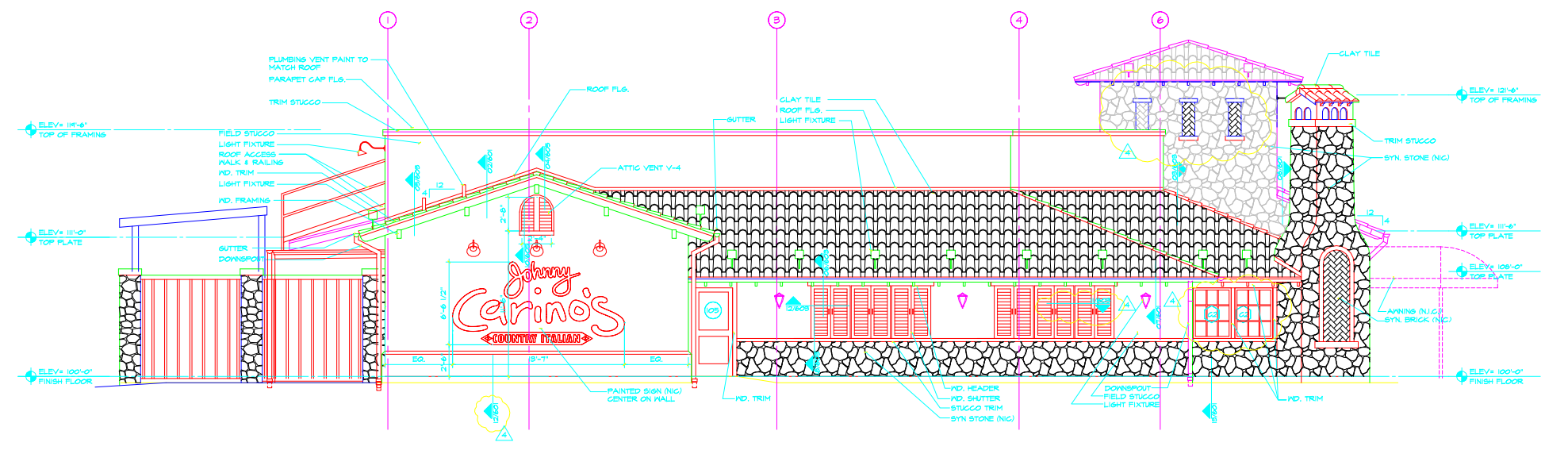


GENERAL NOTES  
 1. ALL DIMENSIONS ARE TO ROUGH FRAME UNLESS NOTED OTHERWISE.  
 2. ALL FLASHING TO BE PAINTED TO MATCH ADJACENT WALL PAINT COLOR UNLESS NOTED OTHERWISE.

TYPICAL FINISHES UNLESS NOTED OTHERWISE	
FIELD STUCCO	P-1
TRIM/BILL STUCCO	P-2
SYNTHETIC STONE	SN-1
EXTERIOR WOOD TRIM	ST-1
EXTERIOR DOORS	ST-6
EXTERIOR WINDOWS AND FRAME	ST-10
WINDOW SHUTTERS	P-5
PARAPET GAP FLASHING	P-2
ROOF FLG.-SLOPE ROOF TO PARAPET	P-1
ROOF FLG.-SLOPE ROOF TO STONE	P-2
ROOF MOUNTED LIGHTS	P-1
RAFTER TAILS, FASCIA AND SOFFIT BELOW	ST-1
COMP. SHINGLE & CLAY TILE RFS.	ST-1
LOUVERS/VENTS	MATCH ADJACENT WALL FINISH
GUTTERS/DOWNSPOUTS	GALVANIZED FINISH



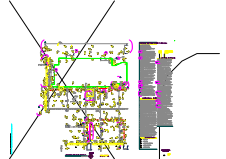
**01 REAR ELEVATION**  
 1/4"=1'-0"



**02 LEFT ELEVATION**  
 1/4"=1'-0"

28 OCT. 2008  
 ADDENDUM #1  
 12 NOV. 2008  
 CITY COMMENTS  
 28 JAN. 2004  
 OWNER COMMENTS





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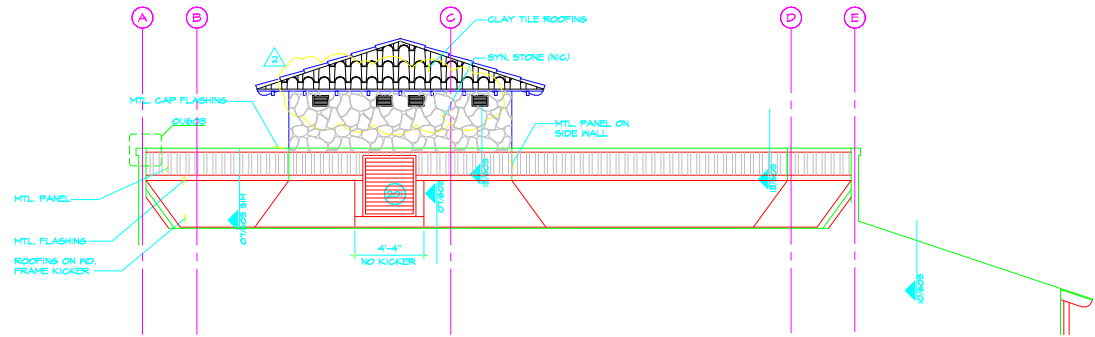
2

□

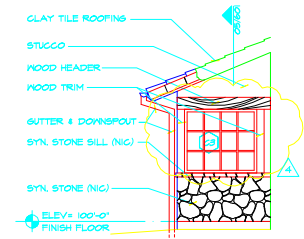
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W

g

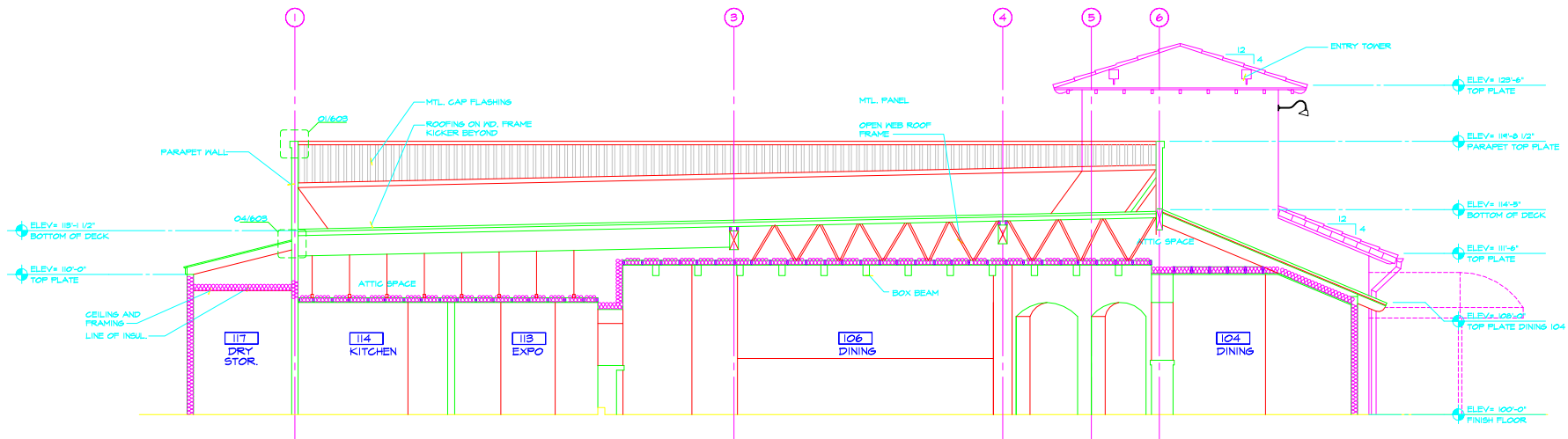


**02 TOWER BACKSIDE ELEVATION**  
1/4"=1'-0"

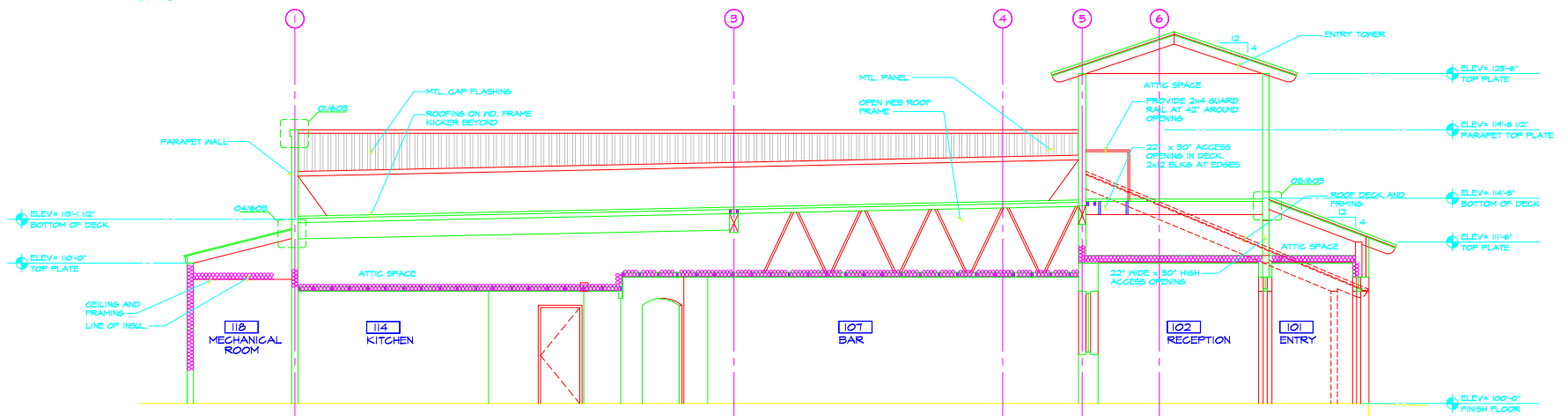


**01 PARTIAL ELEVATION**  
1/4"=1'-0"

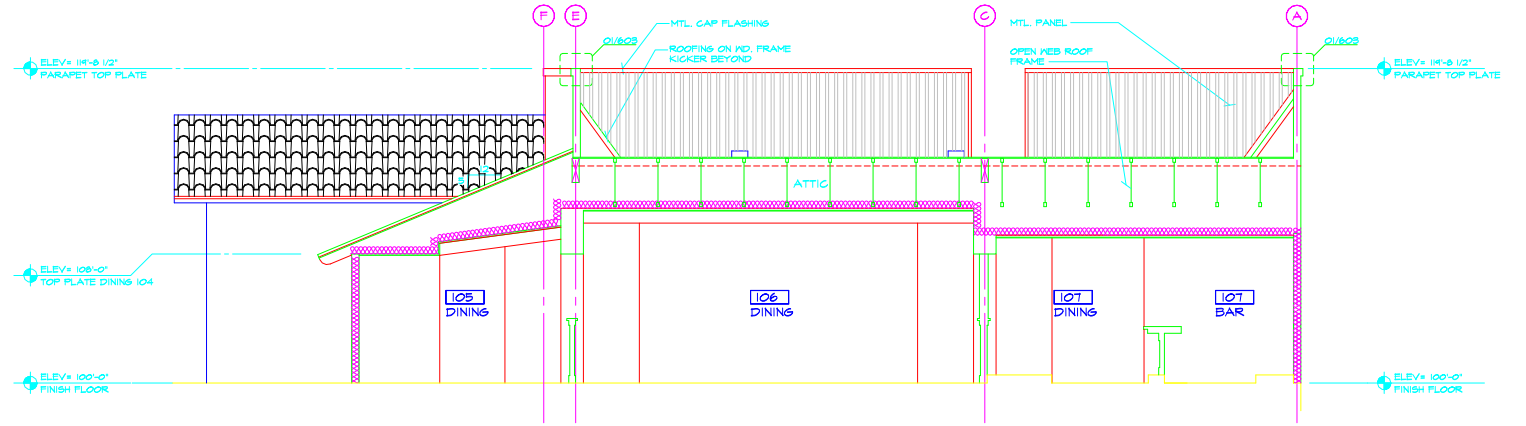
12 NOV. 2003  
CITY COMMENTS  
28 NOV. 2004  
OWNER COMMENTS



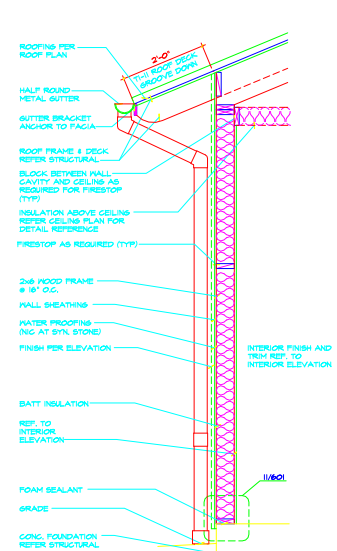
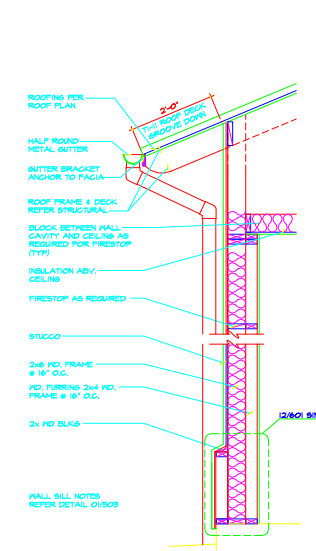
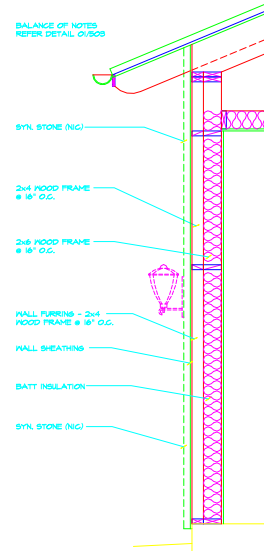
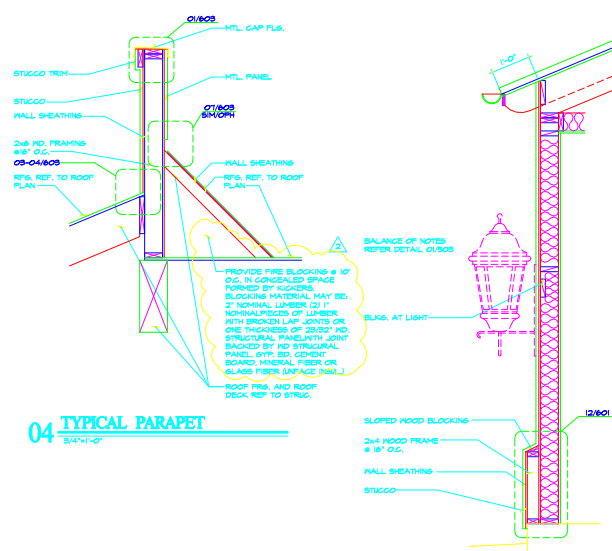
**03 BUILDING SECTION**  
1/4"=1'-0"



**04 BUILDING SECTION**  
1/4"=1'-0"



**01 BUILDING SECTION**  
1/4"=1'-0"



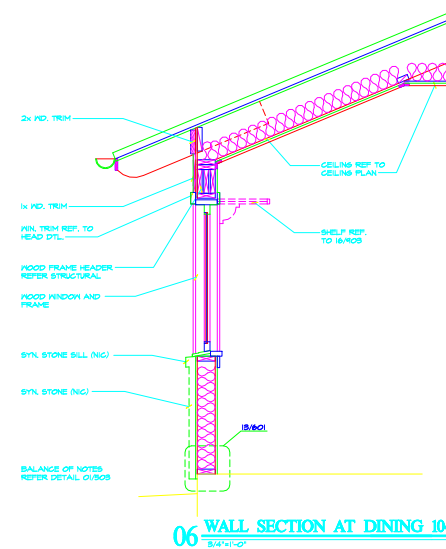
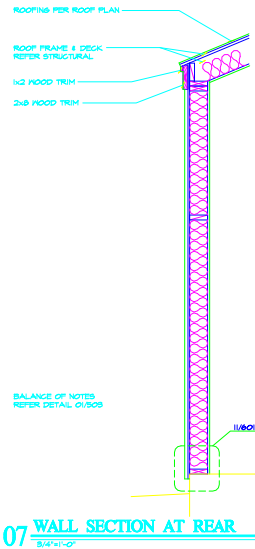
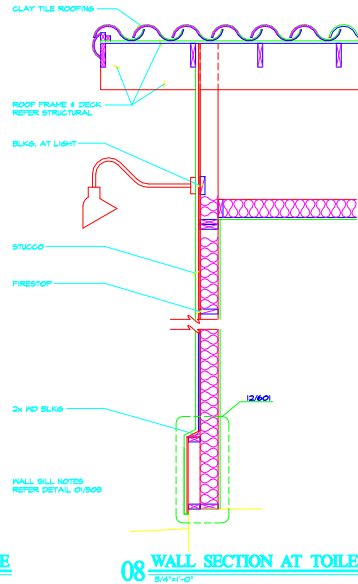
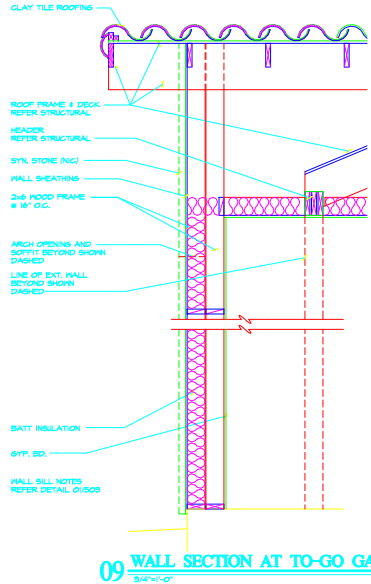
05 NOT USED  
3/4"x1'-0"

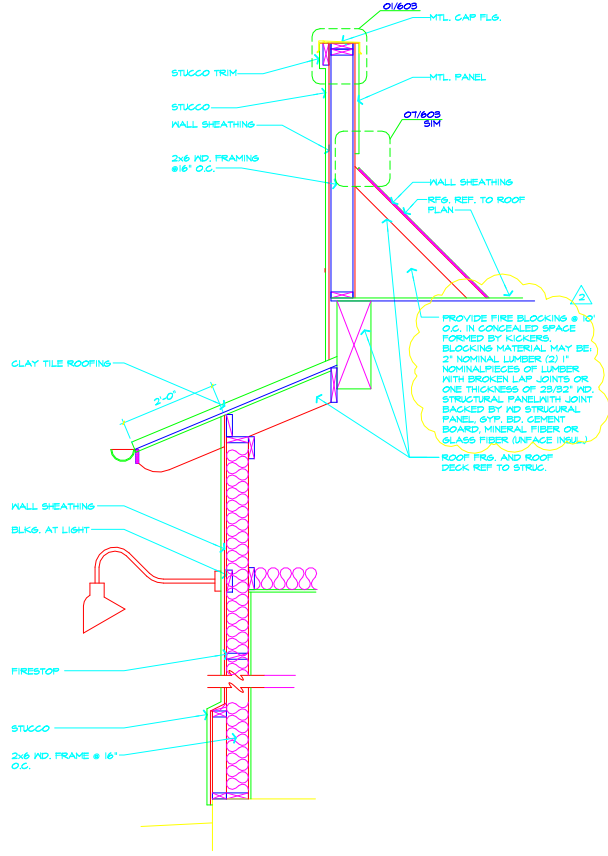
03 WALL SECTION ADJACENT TO TOWER  
3/4"x1'-0"

02 WALL SECTION AT ENTRY SHED  
3/4"x1'-0"

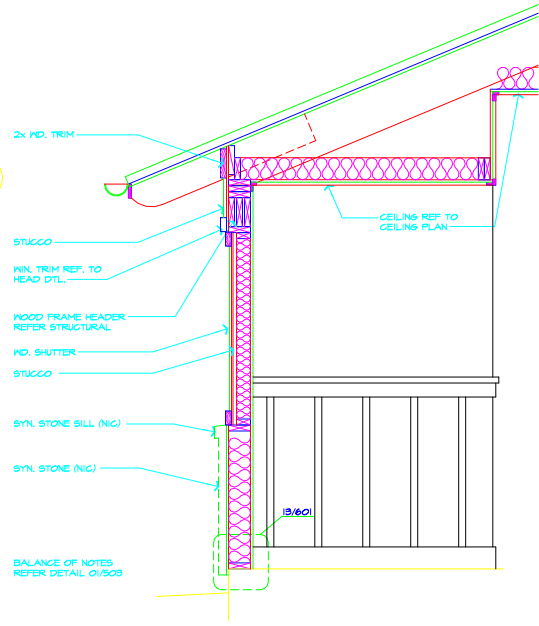
01 WALL SECTION AT TOILET AREA  
3/4"x1'-0"

01 TYPICAL WALL SECTION  
3/4"x1'-0"

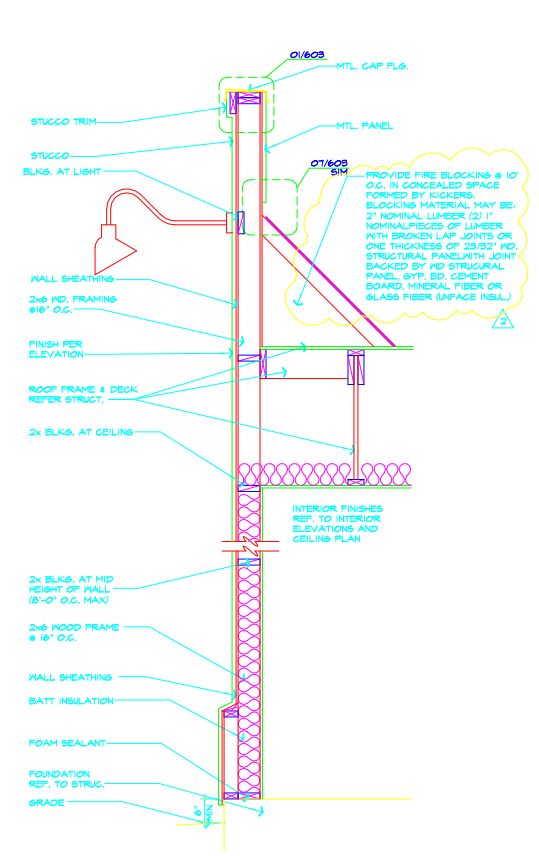




**03 SECTION AT DISHROOM**  
3/4"x1'-0"



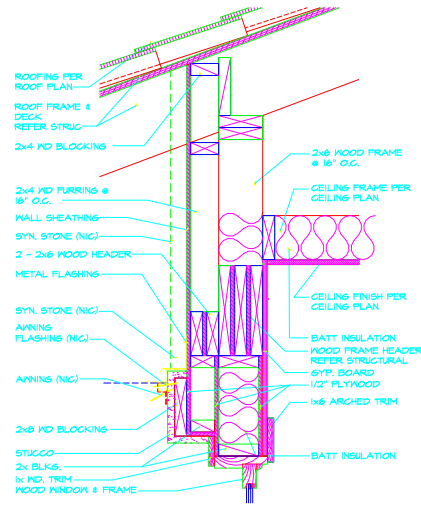
**02 WALL SECTION AT WD. SHUTTERS**  
3/4"x1'-0"



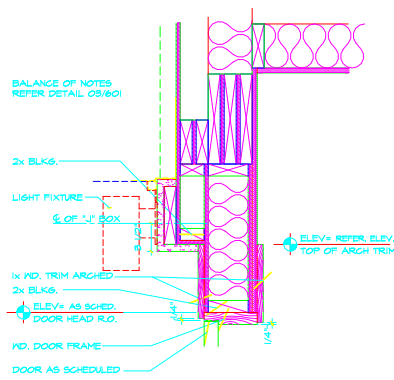
**01 WALL SECTION AT BAR**  
3/4"x1'-0"

CITY COMMENTS  
12 NOV. 2008

**04 NOT USED**  
1 1/2" x 1'-0"

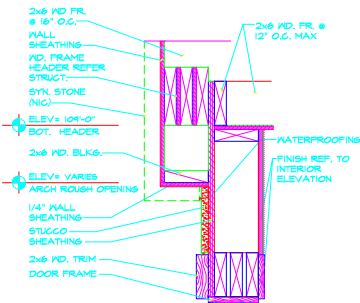


**03 HEAD WINDOW 'A-1'**  
1 1/2" x 1'-0"



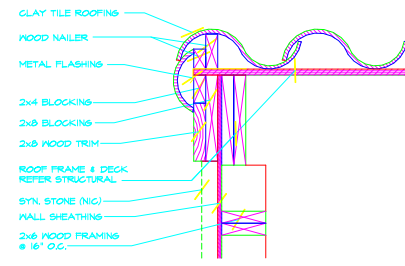
**09 HEAD DOOR 101**  
1 1/2" x 1'-0"

**10 NOT USED**

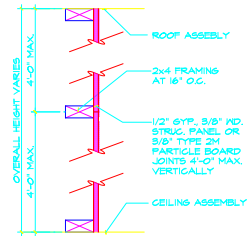


**14 HEADER AT TO-GO**  
1 1/2" x 1'-0"

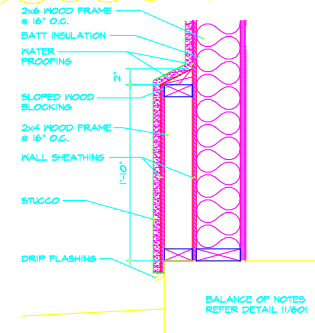
**02 RIDGE AT TOILETS**  
1 1/2" x 1'-0"



**06 CLAY TILE ROOF EAVE AT SYN. STONE**  
1 1/2" x 1'-0"

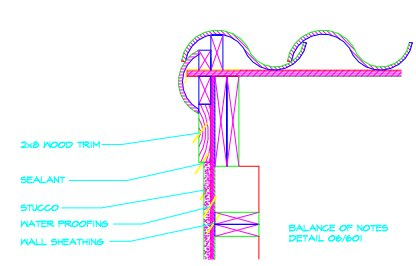


**08 DRAFTSTOPPING**  
3/4" x 1'-0"

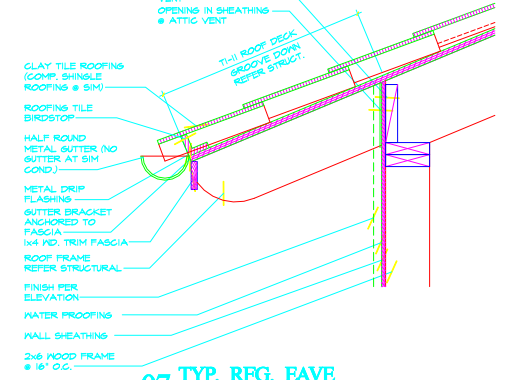


**12 WALL BASE-STUCCO LEDGE**  
1 1/2" x 1'-0"

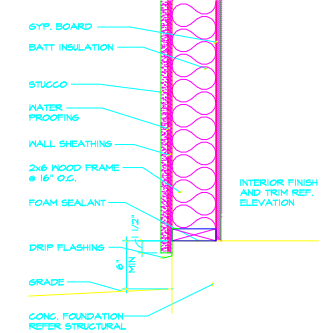
**01 RIDGE AT TO-GO**  
1 1/2" x 1'-0"



**05 CLAY TILE ROOF EAVE AT STUCCO**  
1 1/2" x 1'-0"

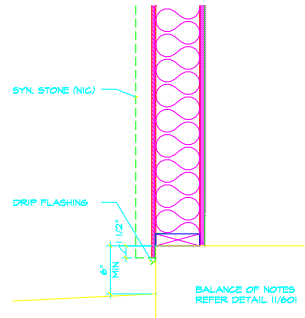


**07 TYP. RFG. EAVE**  
1 1/2" x 1'-0"

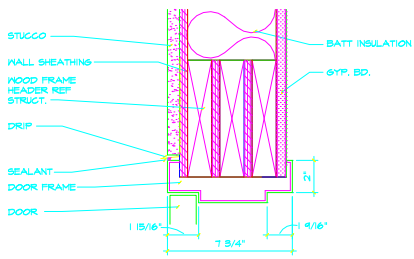


**11 WALL BASE-STUCCO**  
1 1/2" x 1'-0"

**13 WALL BASE-SYN. STONE**  
1 1/2" x 1'-0"

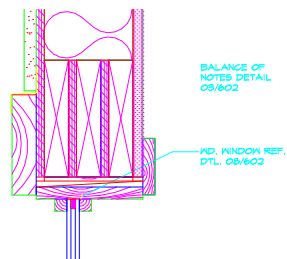




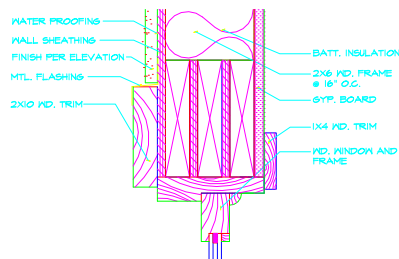


**05 HEAD DOOR 114, 118, 119, 123**  
3'1"-0"

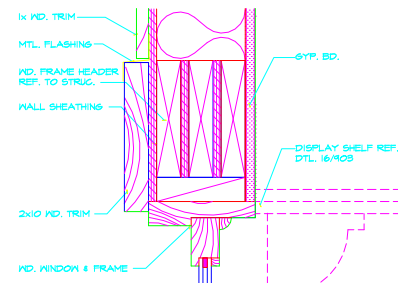
NOTE: COORDINATE INSTALLATION OF DOOR 122A FRAME WITH DELIVERY AND INSTALLATION OF ELECTRIC PANEL BECAUSE OF REQ'D OPENING SIZE FOR PANEL TO ENTER BUILDING.



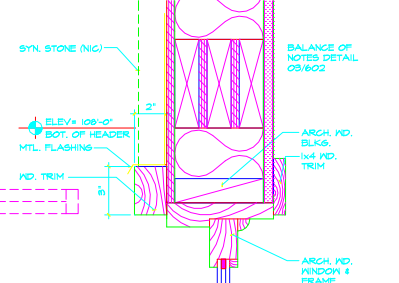
**04 HEAD WINDOW 'F'**  
3'1"-0"



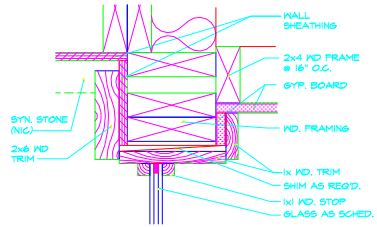
**03 HEAD WINDOW 'D,E'**  
3'1"-0"



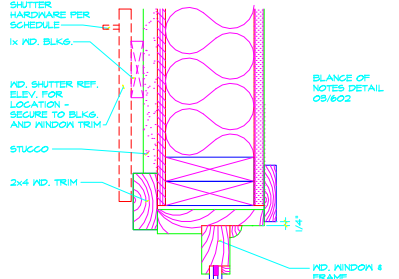
**02 HEAD WINDOW 'C'**  
3'1"-0"



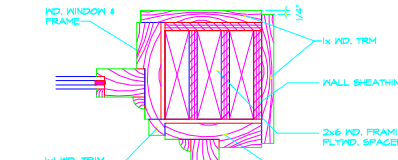
**01 HEAD WINDOW 'B' (JAMB SIM)**  
3'1"-0"



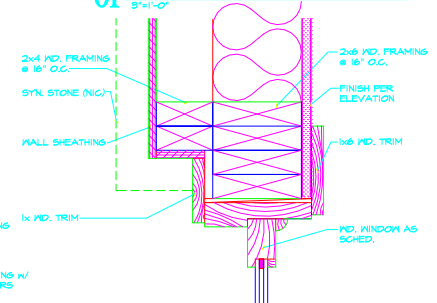
**08 JAMB WINDOW 'F'**  
3'1"-0"



**07 JAMB WINDOW 'D,E'**  
3'1"-0"

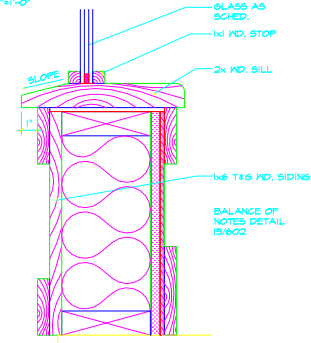


**11 JAMB WINDOW 'C'**  
3'1"-0"

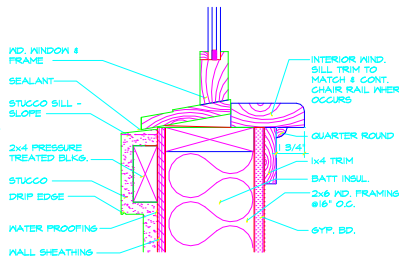


**06 JAMB WINDOW A-1**  
3'1"-0"

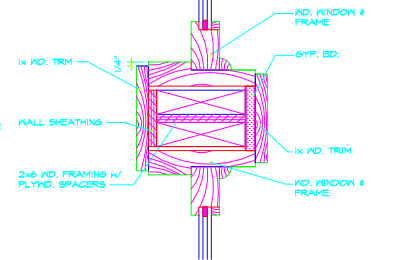
**09 NOT USED**



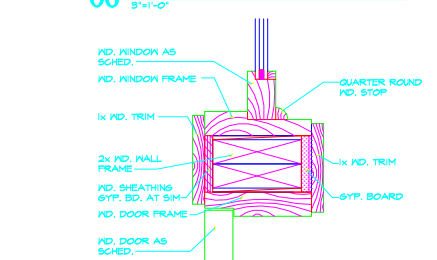
**13 SILL WINDOW 'F'**  
3'1"-0"



**12 SILL WINDOW 'D,E'**  
3'1"-0"

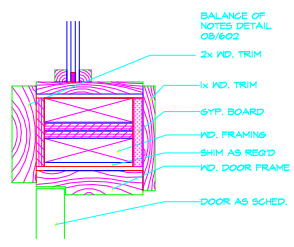


**16 SILL WINDOW 'B, C'**  
3'1"-0"

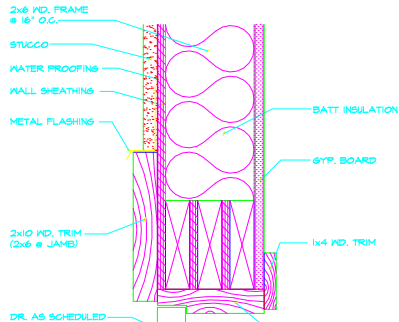


**10 JAMB DOOR 101, WIN A-1**  
3'1"-0"

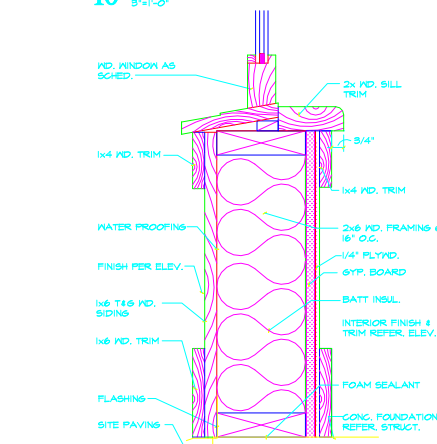
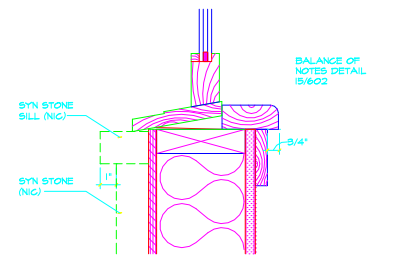
**14 NOT USED**



**18 DOOR JAMB 108A / WINDOW 'F'**  
1/4'1"-0"

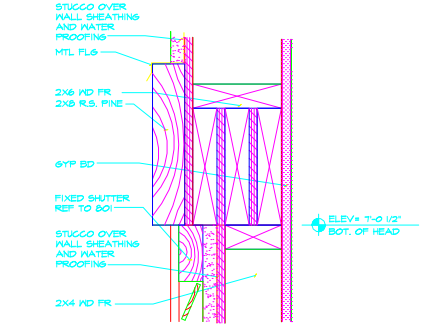


**17 DOOR HEAD 108A (JAMB SIM)**  
3'1"-0"

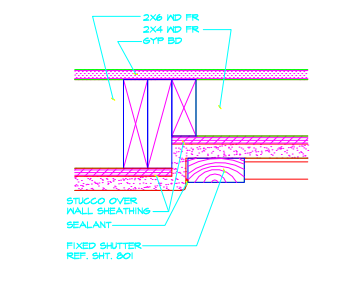


**15 SILL WINDOW 'A-1'**  
3'1"-0"

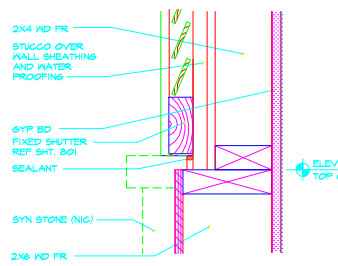
**19 NOT USED**



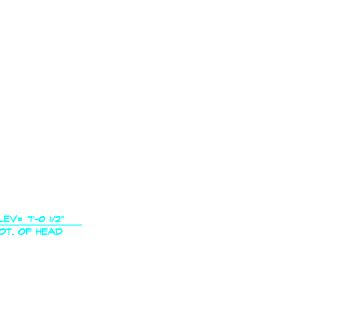
**06 HEAD AT FIXED SHUTTER**  
3/4"x1'-0"



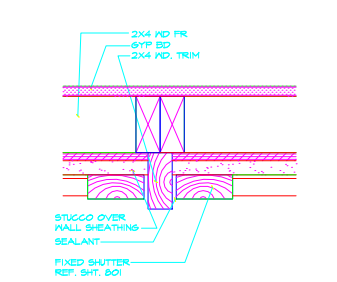
**12 JAMB AT FIXED SHUTTER**  
3/4"x1'-0"



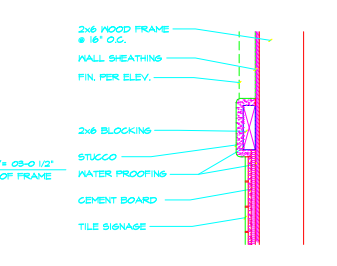
**18 SILL AT FIXED SHUTTER**  
3/4"x1'-0"



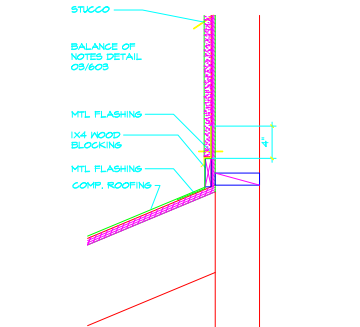
**05 NOT USED**  
1 1/2"x1'-0"



**11 JAMB AT FIXED SHUTTER**  
3/4"x1'-0"



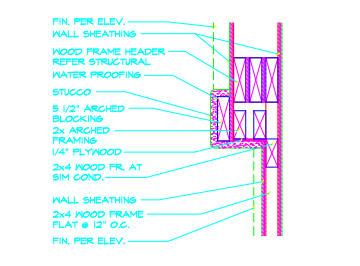
**17 TRIM AT TILE SIGN**  
1 1/2"x1'-0"



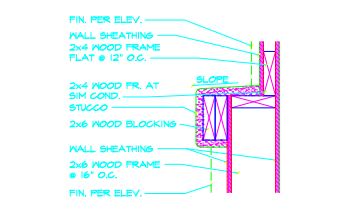
**04 COMP. REF. AT STUCCO WALL**  
1 1/2"x1'-0"



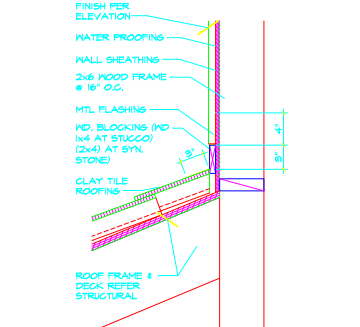
**09 CLAY TILE AT STUCCO WALL**  
1 1/2"x1'-0"



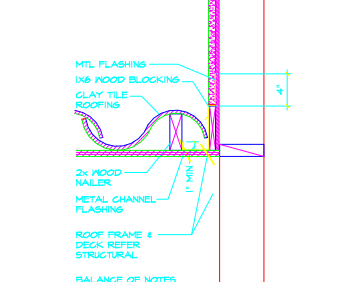
**16 HEAD AT SYN. STONE RECESS**  
1 1/2"x1'-0"



**22 SILL AT SYN. STONE RECESS**  
1 1/2"x1'-0"



**03 CLAY TILE ROOF AT STUCCO**  
1 1/2"x1'-0"



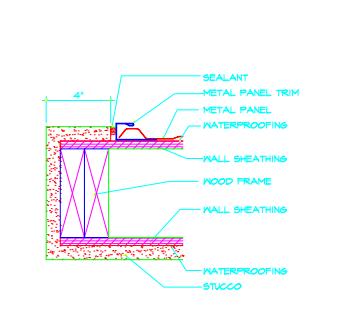
**09 CLAY TILE AT STUCCO WALL**  
1 1/2"x1'-0"



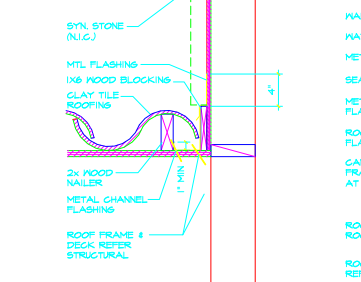
**15 NOT USED**  
1 1/2"x1'-0"



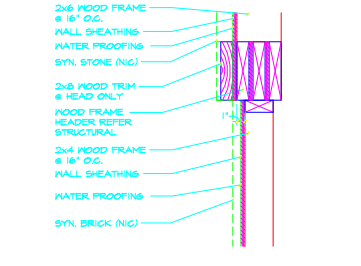
**21 NOT USED**  
1 1/2"x1'-0"



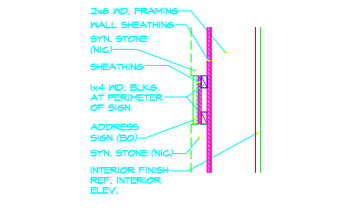
**02 PARAPET AT ROOF ACCESS**  
3/4"x1'-0"



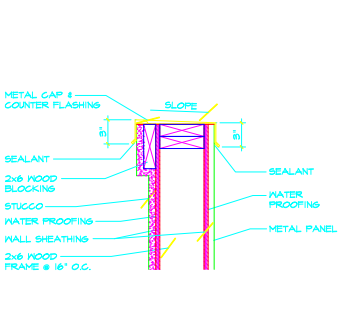
**08 CLAY TILE AT SYN. STONE**  
1 1/2"x1'-0"



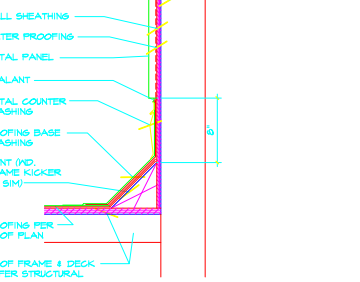
**14 SYN. STONE AT SYN. BRICK**  
1 1/2"x1'-0"



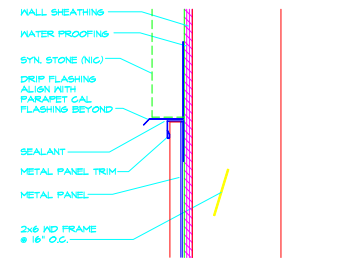
**20 ADDRESS SIGN**  
1 1/2"x1'-0"



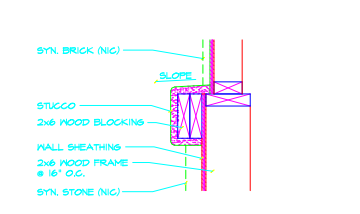
**01 TYP. PARAPET**  
1 1/2"x1'-0"



**07 MTL. PANEL WALL AT ROOF**  
1 1/2"x1'-0"

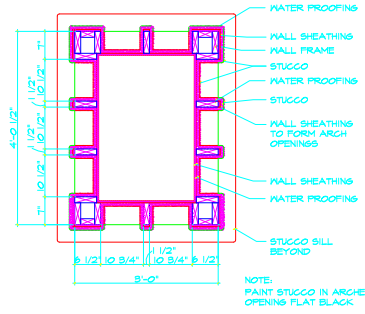
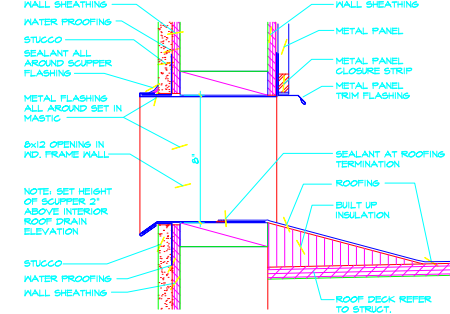
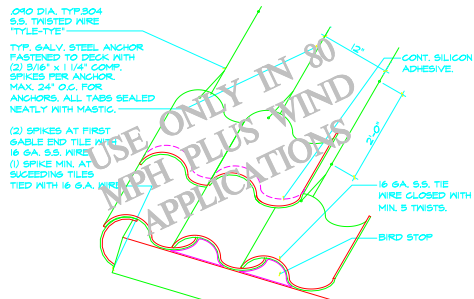
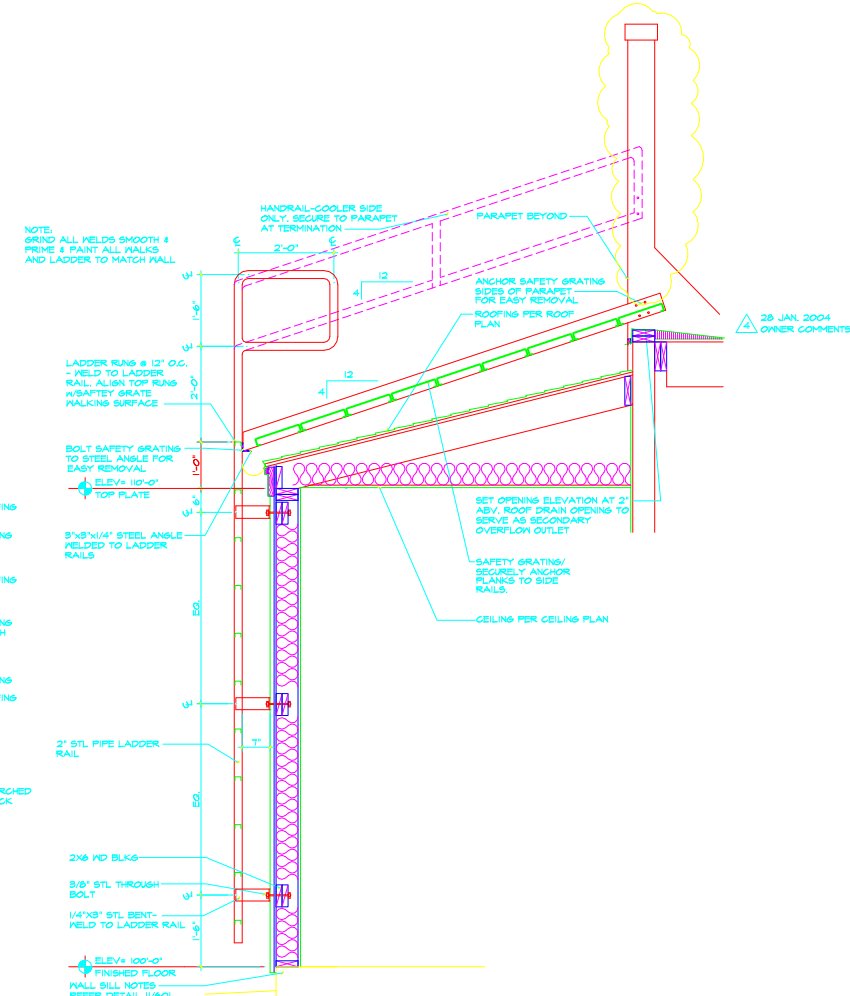
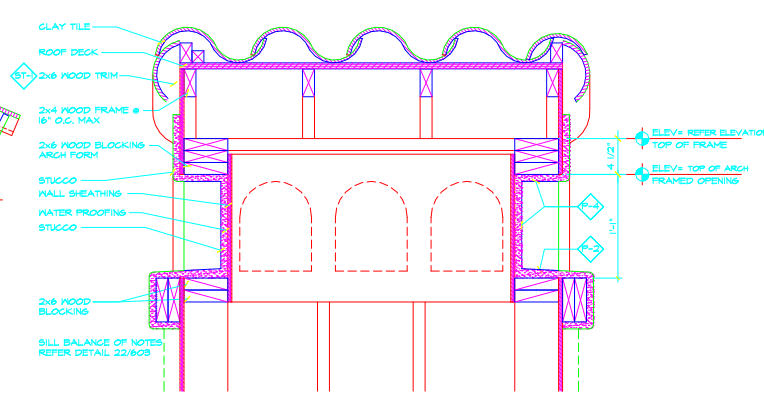
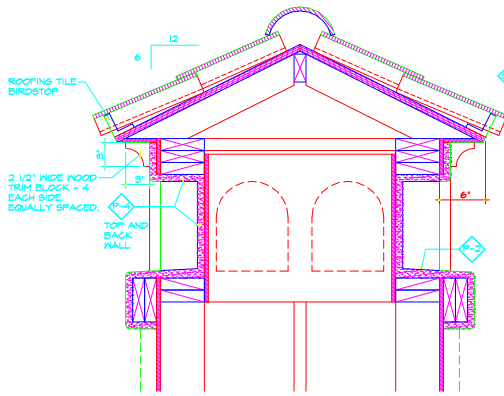


**13 SYN. STONE AT MTL. PANEL**  
3/4"x1'-0"



**19 STUCCO TRIM AT SYN. BRICK**  
1 1/2"x1'-0"

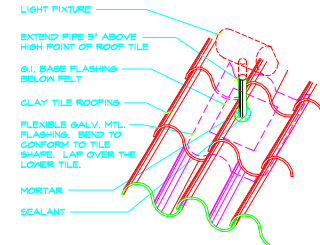
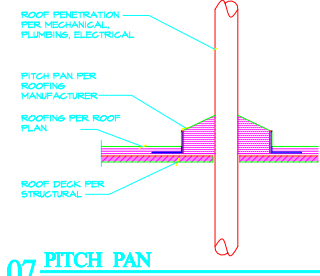
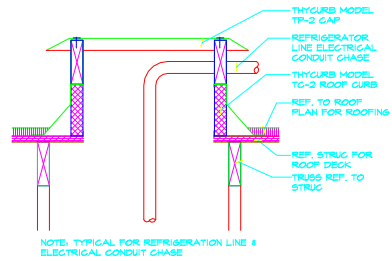
**23 NOT USED**  
1 1/2"x1'-0"



**05 ROOF TIE-DOWN**  
NOT TO SCALE

**04 OVERFLOW SCUPPER**  
3/4"x1'-0"

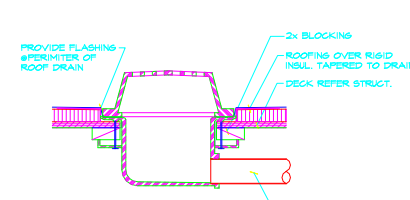
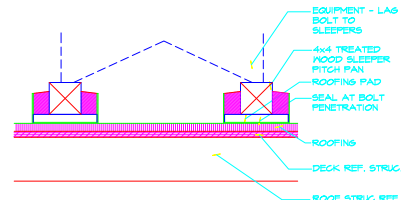
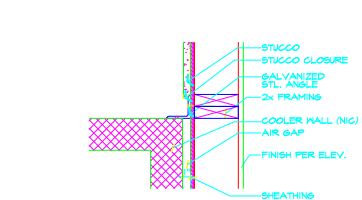
**03 CHIMNEY CAP PLAN**  
3/4"x1'-0"



**08 ROOF PENETRATION**  
1 1/2"x1'-0"

**07 PITCH PAN**  
1 1/2"x1'-0"

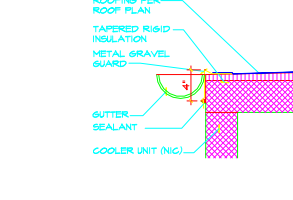
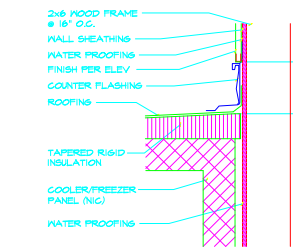
**06 ROOF MOUNTED LIGHT FIXTURE**  
NOT TO SCALE



**13 COOLER CLOSURE**  
1 1/2"x1'-0"

**12 ROOF SKID**  
1 1/2"x1'-0"

**11 ROOF DRAIN**  
1 1/2"x1'-0"

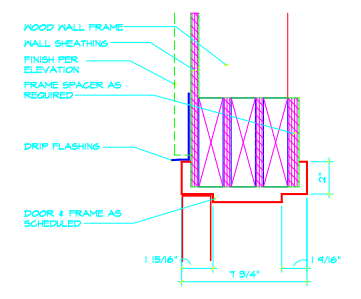
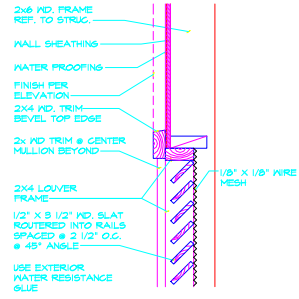


**10 COOLER ROOF TO WALL**  
1 1/2"x1'-0"

**09 COOLER ROOF EAVE**  
1 1/2"x1'-0"

**USE ONLY IN 80 MPH PLUS WIND APPLICATIONS**

28 JAN 2004 OWNER COMMENTS



06 NOT USED  
1 1/2" x 1'-0"

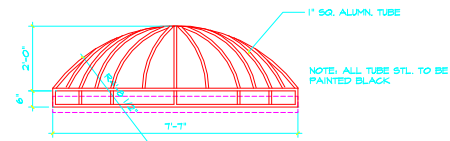
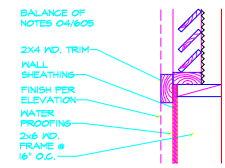
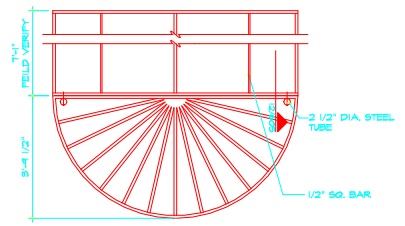
05 NOT USED  
3" x 1'-0"

04 HEAD-ATTIC VENT V-4  
1 1/2" x 1'-0"

03 NOT USED

02 NOT USED

01 HEAD DOOR 201  
3" x 1'-0"



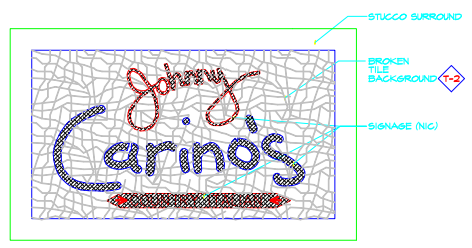
11 ENTRY AWNING (NIC)  
1/2" x 1'-0"

10 SILL-ATTIC VENT V-4  
1 1/2" x 1'-0"

09 NOT USED

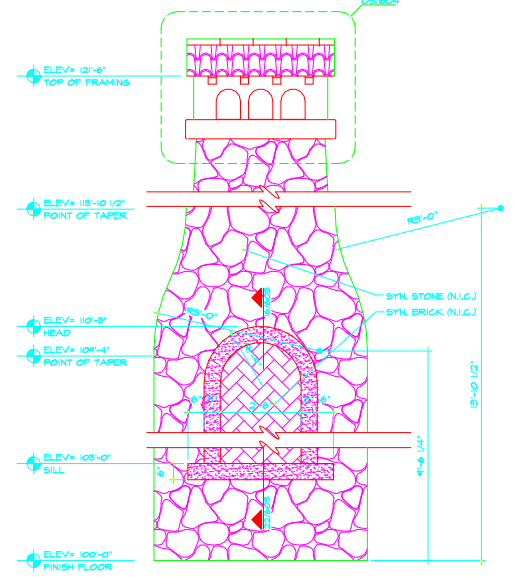
08 NOT USED

07 JAMB DOOR 201  
3" x 1'-0"

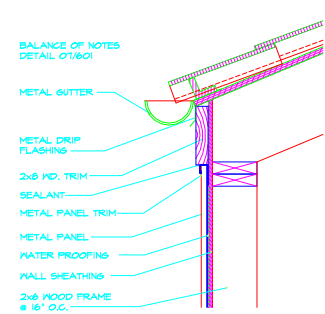


NOTE: TILE SIGN BACKGROUND BY G.C. ALL TILE TO BE BROKEN AND ASSEMBLED ON SITE

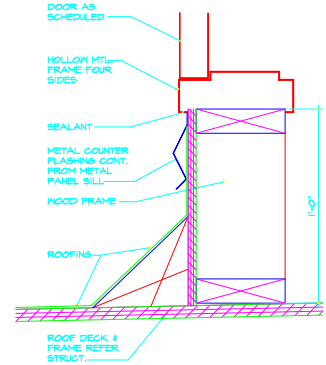
16 BROKEN TILE SIGN BACKGROUND  
1/2" x 1'-0"



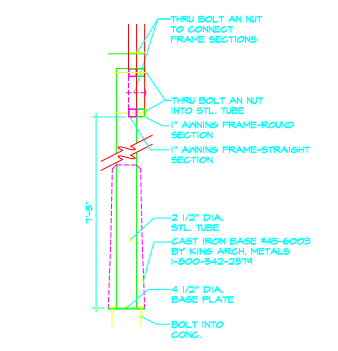
15 FIRE PLACE ELEVATION  
1/2" x 1'-0"



14 EAVE AT METAL PANEL  
1 1/2" x 1'-0"

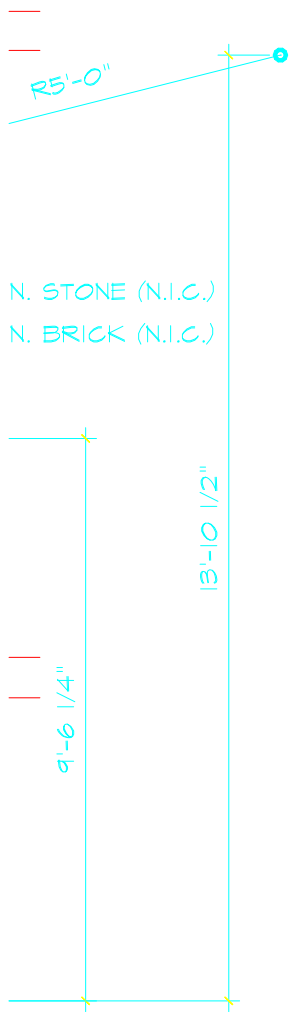


13 SILL DOOR 201  
3" x 1'-0"



12 AWNING CONNECTION (NIC)  
1 1/2" x 1'-0"

6500 Greenville Ave.  
 Suite 307  
 Dallas, TX 75206  
 214.361.9901  
 Fax 361.9906



N. STONE (N.I.C.)  
 N. BRICK (N.I.C.)

BALANCE OF NOTES  
 DETAIL 07/601

METAL GUTTER

METAL DRIP  
 FLASHING

2x8 WD. TRIM

SEALANT

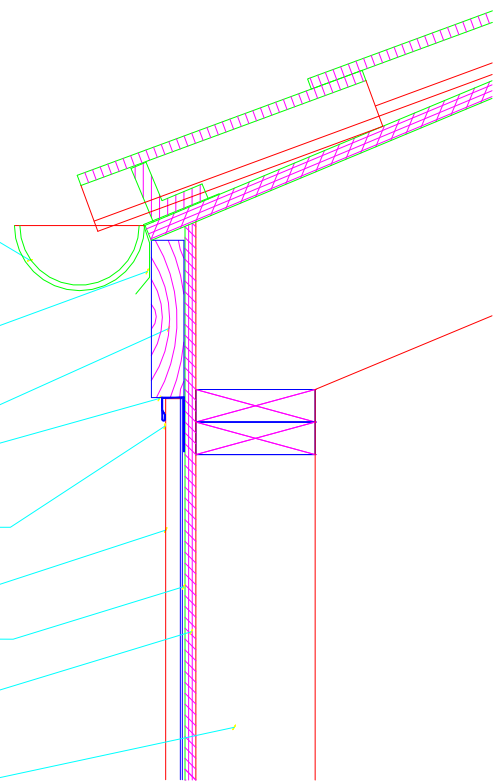
METAL PANEL TRIM

METAL PANEL

WATER PROOFING

WALL SHEATHING

2x6 WOOD FRAME  
 @ 16" O.C.



14 EAVE AT METAL PANEL 1

1 1/2"=1'-0"

AREA OF BUILDING:

JOB NO.

DATE:

C.D. REF:

C.L. REF:

0103

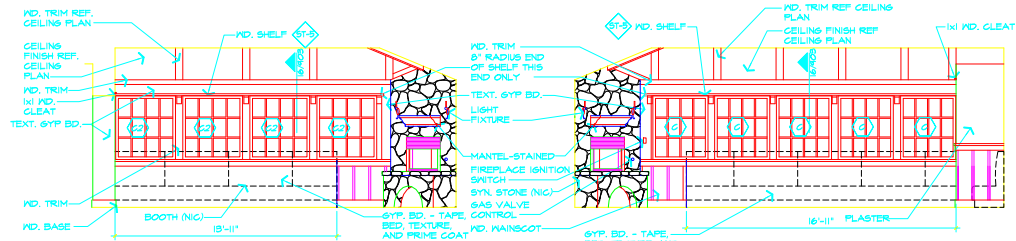
PROJECT:

SCALE:

SHEET:

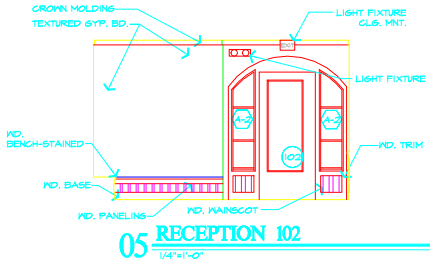
CARINO'S PROTO

8.0 - UPDATE

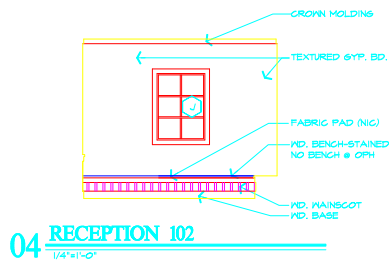


**12 DINING 104**  
1/4"=1'-0"

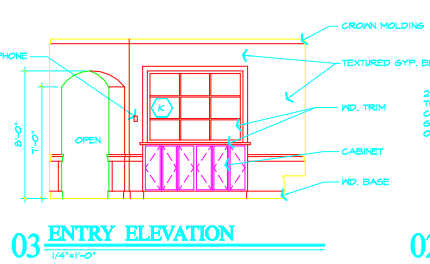
**11 DINING 104**  
1/4"=1'-0"



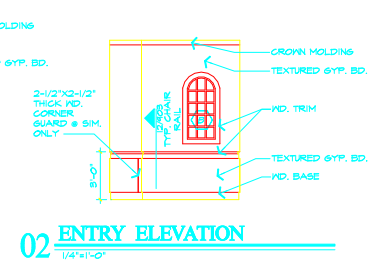
**05 RECEPTION 102**  
1/4"=1'-0"



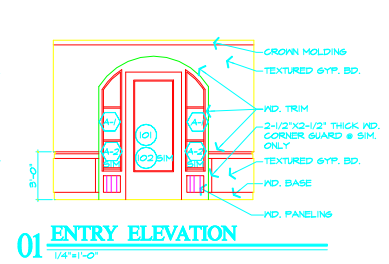
**04 RECEPTION 102**  
1/4"=1'-0"



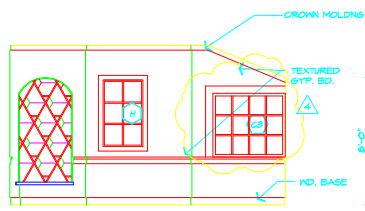
**03 ENTRY ELEVATION**  
1/4"=1'-0"



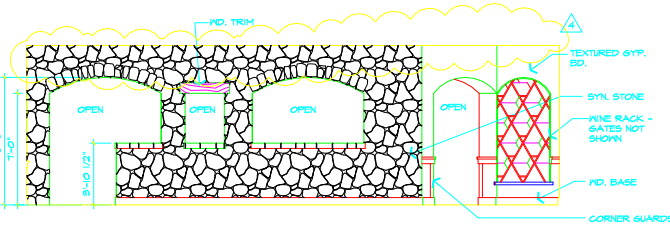
**02 ENTRY ELEVATION**  
1/4"=1'-0"



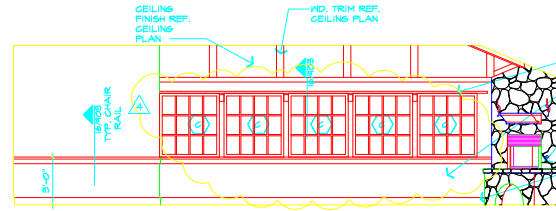
**01 ENTRY ELEVATION**  
1/4"=1'-0"



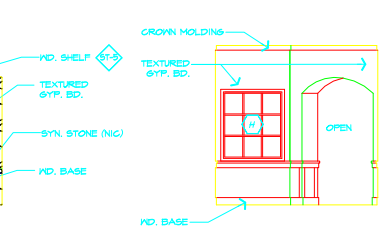
**09 DINING 104**  
1/4"=1'-0"



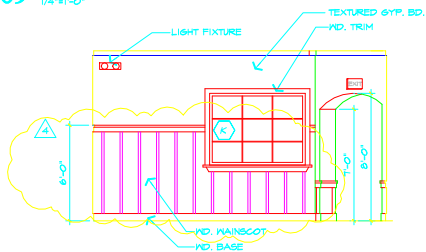
**08 DINING 104**  
1/4"=1'-0"



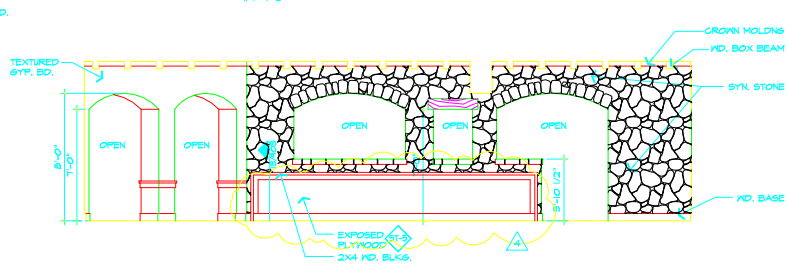
**07 DINING 104**  
1/4"=1'-0"



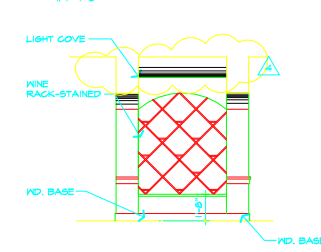
**06 RECEPTION 102**  
1/4"=1'-0"



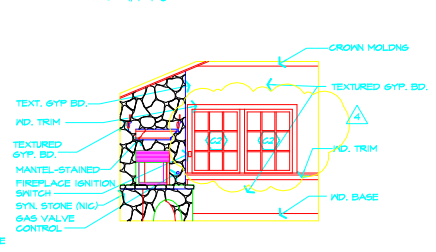
**13 BAR DINING 107**  
1/4"=1'-0"



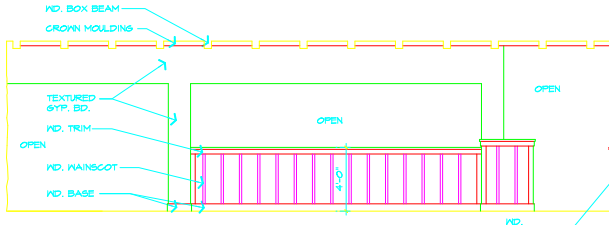
**12 DINING 105/106**  
1/4"=1'-0"



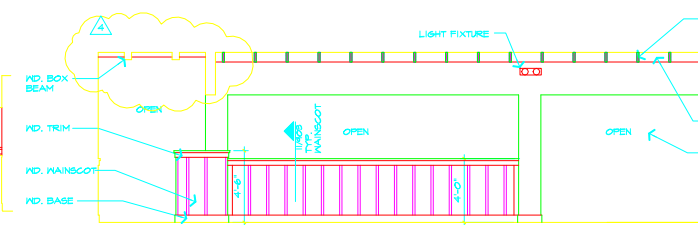
**11 CORRIDOR 103**  
1/4"=1'-0"



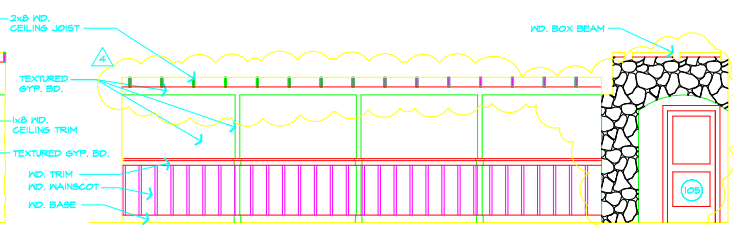
**10 DINING 104**  
1/4"=1'-0"



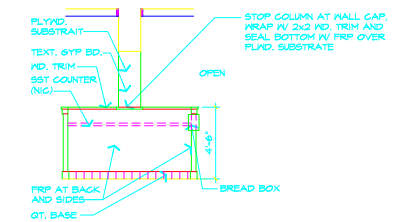
**16 DINING 106**  
1/4"=1'-0"



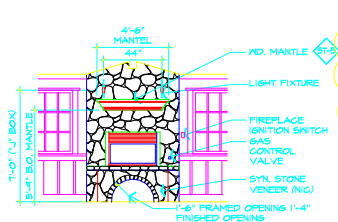
**15 DINING 105**  
1/4"=1'-0"



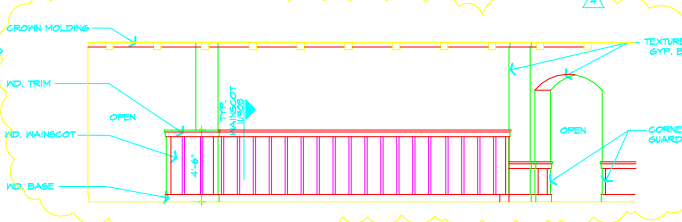
**14 DINING 105**  
1/4"=1'-0"



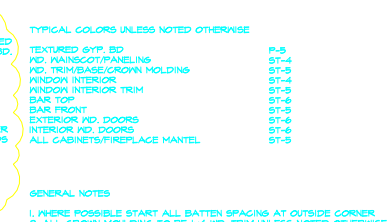
**19 DRINK STATION**  
1/4"=1'-0"



**18 DINING 104**  
1/4"=1'-0"



**17 DINING 106**  
1/4"=1'-0"

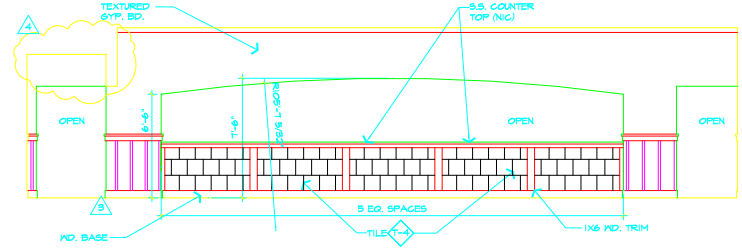
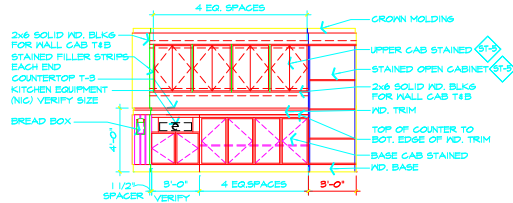
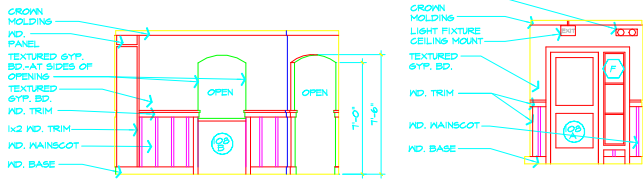


**17 DINING 106**  
1/4"=1'-0"

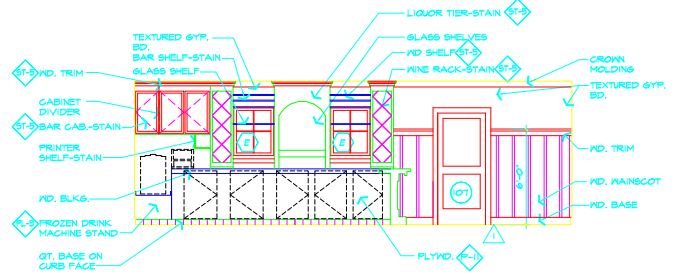
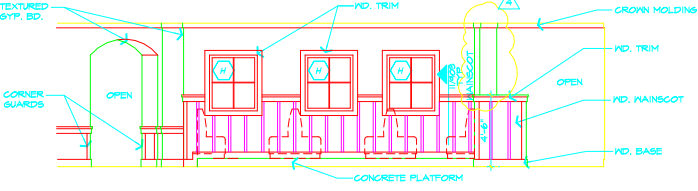
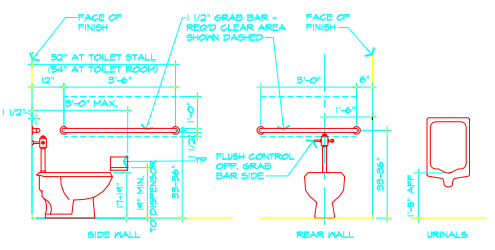
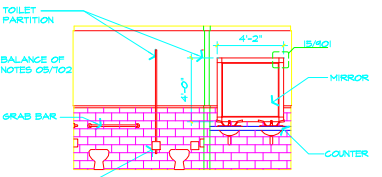
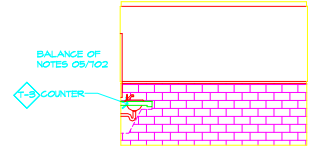
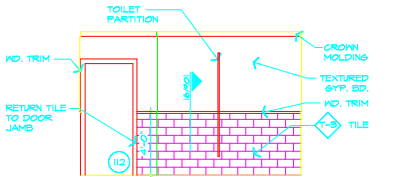
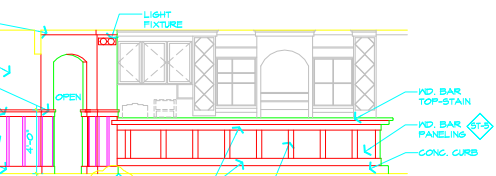
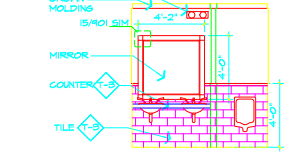
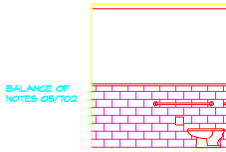
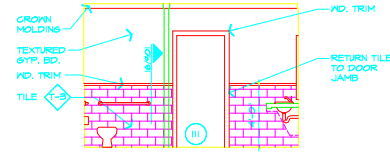
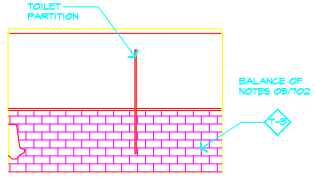
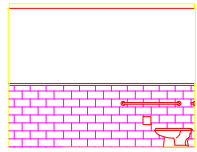
- TYPICAL COLORS UNLESS NOTED OTHERWISE
- TEXTURED GYP. BD.
  - MD. MAINSCOT/PANELING
  - MD. TRIM/BASE/CROWN MOULDING
  - WINDOW INTERIOR
  - WINDOW INTERIOR TRIM
  - BAR TOP
  - BAR FRONT
  - EXTERIOR MD. DOORS
  - INTERIOR MD. DOORS
  - ALL CABINETS/FIREPLACE MANTEL
- P-5
  - ST-4
  - ST-9
  - ST-4
  - ST-9
  - ST-6
  - ST-9
  - ST-6
  - ST-4
  - ST-5

- GENERAL NOTES
1. WHERE POSSIBLE START ALL BATTEN SPACINGS AT OUTSIDE CORNER
  2. ALL CROWN MOULDINGS TO BE 1/4" MD. TRIM UNLESS NOTED OTHERWISE
  3. ALL MD. BASE TO BE 1/2" MD. TRIM UNLESS NOTED OTHERWISE
  4. MD. MAINSCOT REFER TO DTLL 11/10/03
  5. ALL TEXTURED GYP. BD. OUTSIDE CORNERS TO BE BULLNOSE
  6. CHAIR RAIL HEIGHT 07/101 UNO.

25 JAN. 2004  
OWNER COMMENTS



28 OCT. 2009  
APPENDUM #1  
16 DEC. 2009  
CITY COMMENTS  
28 JAN. 2004  
OWNER COMMENTS



TYPICAL COLORS UNLESS NOTED OTHERWISE

TEXTURED GYP. BD.  
MD. MAINSCOT/PANELING  
MD. TRIM/BASE/CROWN MOLDING  
WINDOW INTERIOR  
WINDOW INTERIOR TRIM  
BAR TOP  
BAR FRONT  
EXTERIOR MD. DOORS  
INTERIOR MD. DOORS  
ALL CABINETS/FIREPLACE MANTEL

P-5  
ST-4  
ST-5  
ST-4  
ST-4  
ST-5  
ST-5  
ST-5  
ST-4  
ST-5  
ST-6

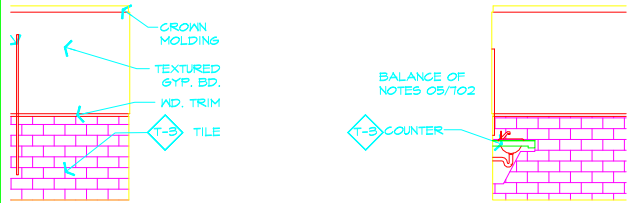
GENERAL NOTES

1. WHERE POSSIBLE START ALL BATTEN SPACING AT OUTSIDE CORNER
2. ALL CROWN MOLDING TO BE 1x4 MD. TRIM UNLESS NOTED OTHERWISE
3. ALL MD. BASE TO BE 1x6 MD. TRIM UNLESS NOTED OTHERWISE
4. MD. MAINSCOT REFER TO DTL. 11403
5. ALL TEXTURED GYP. BD. OUTSIDE CORNERS TO BE BULLNOSE

AREA OF BUILDING:	JOB NO.	DATE:	C.D. REF:	C.L. REF:
	0103			
PROJECT:	SCALE:			SHEET:
CARINO'S PROTO				
8.0 - UPDATE				

1FN

RESTROOM WOMEN

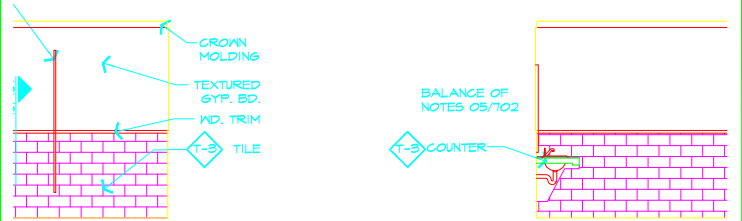


OMEN

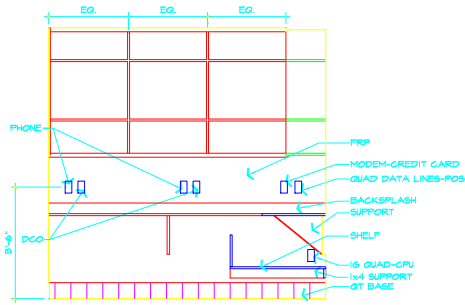
11

RESTROOM WOMEN

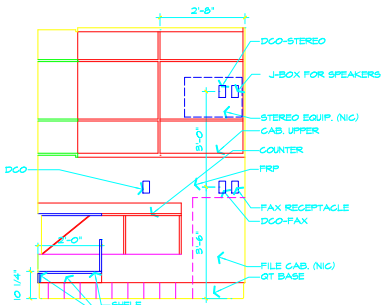
1/4"=1'-0"



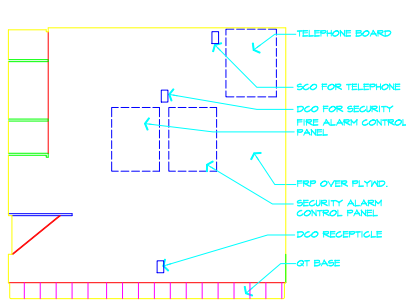




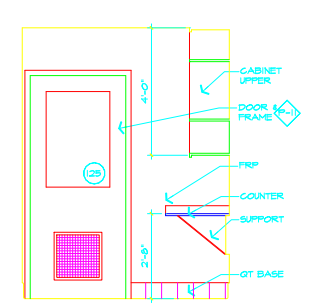
04 OFFICE ELEVATION  
1/2"x1'-0"



03 OFFICE ELEVATION  
1/2"x1'-0"

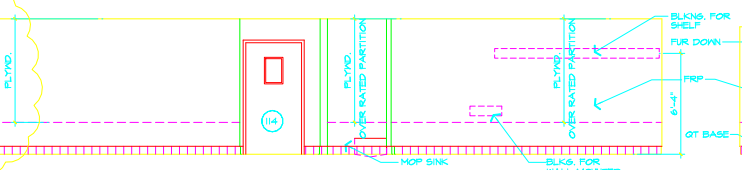
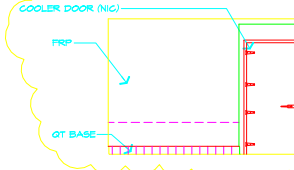


02 OFFICE ELEVATION  
1/2"x1'-0"

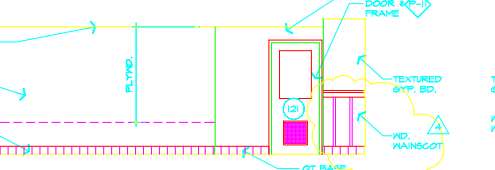


01 OFFICE ELEVATION  
1/2"x1'-0"

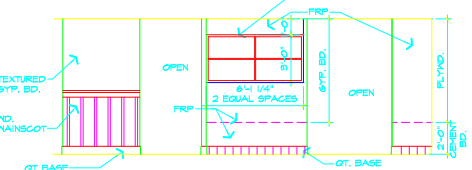
05 NOT USED  
1/4"x1'-0"



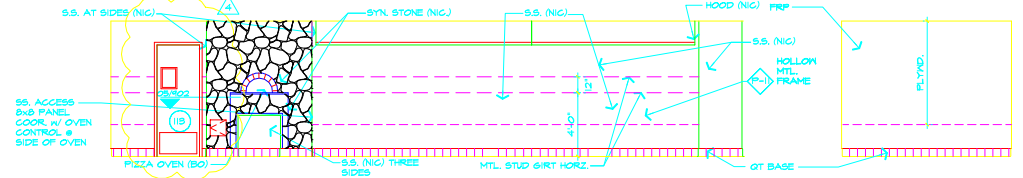
08 KITCHEN PREP/DISH WASH  
1/4"x1'-0"



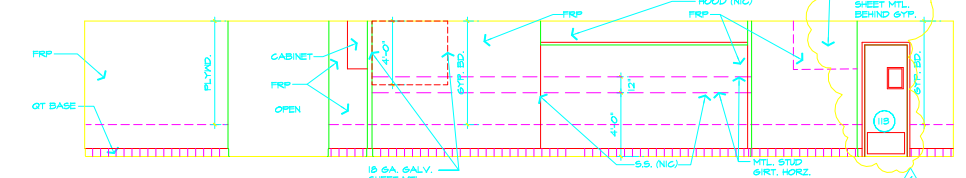
07 KITCHEN OFFICE/DISH WASH  
1/4"x1'-0"



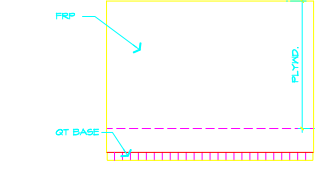
06 KITCHEN TO-GO  
1/4"x1'-0"



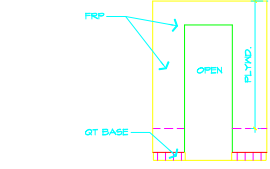
11 KITCHEN PREP  
1/4"x1'-0"



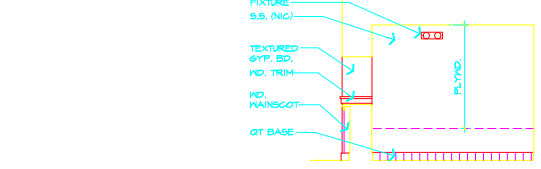
09 KITCHEN PREP/DISH WASH  
1/4"x1'-0"



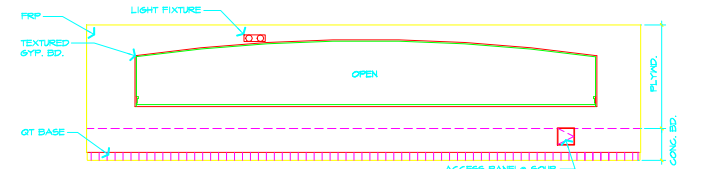
15 DRY STORAGE  
1/4"x1'-0"



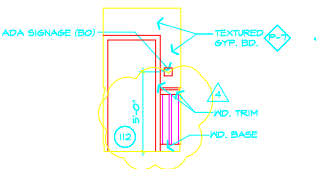
14 DRY STORAGE  
1/4"x1'-0"



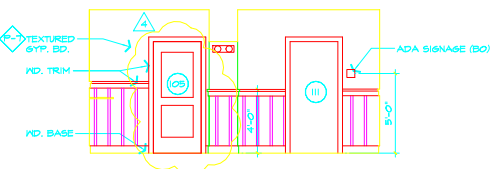
13 EXPO KITCHEN  
1/4"x1'-0"



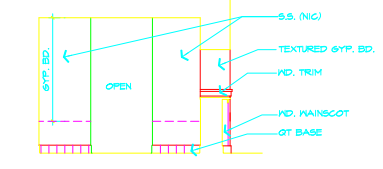
12 KITCHEN EXPO BAR  
1/4"x1'-0"



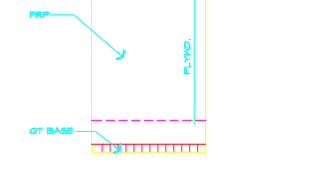
20 RESTROOM CORRIDOR  
1/4"x1'-0"



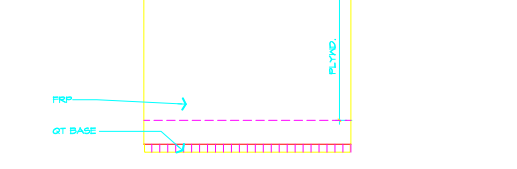
19 RESTROOM CORRIDOR  
1/4"x1'-0"



18 EXPO KITCHEN  
1/4"x1'-0"



17 DRY STORAGE  
1/4"x1'-0"

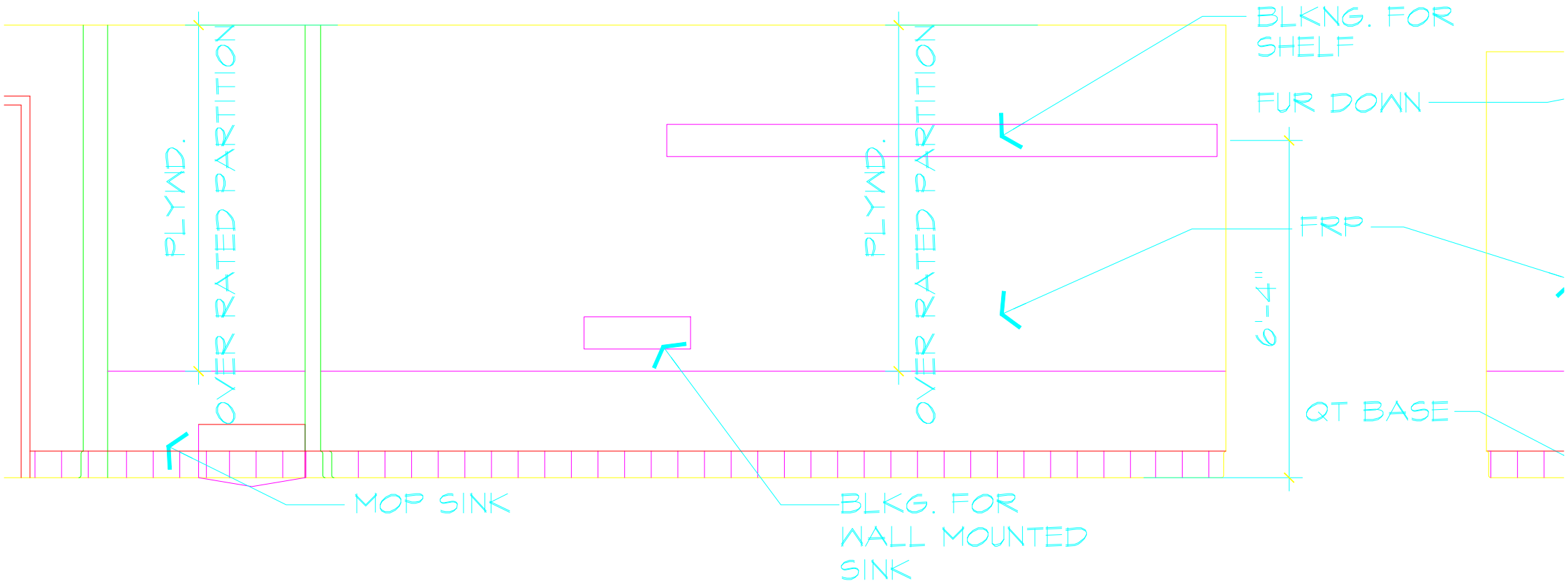


16 DRY STORAGE  
1/4"x1'-0"

- TYPICAL COLORS UNLESS NOTED OTHERWISE
- P-5 TEXTURED GYP. BO
  - ST-4 MD. MAINCOT/PANELING
  - ST-5 MD. TRIM/BASE/CROWN MOLDING
  - ST-6 WINDOW INTERIOR
  - ST-7 WINDOW INTERIOR TRIM
  - ST-8 BAR TOP
  - ST-9 BAR FRONT
  - ST-10 EXTERIOR MD. DOORS
  - ST-11 INTERIOR MD. DOORS
  - ST-12 ALL CABINETS/FIREPLACE MANTEL
- GENERAL NOTES
1. WHERE POSSIBLE START ALL BATTEN SPACINGS AT OUTSIDE CORNER
  2. ALL CROWN MOULDINGS TO BE 1/4" MD. TRIM UNLESS NOTED OTHERWISE
  3. ALL MD. BASE TO BE 1/4" MD. TRIM UNLESS NOTED OTHERWISE
  4. MD. MAINCOT REFERS TO DTL. 11/19/03
  5. ALL TEXTURED GYP. BO, OUTSIDE CORNERS TO BE BULLNOSE
  6. GIBBER BOARD SUBSTRATE TO 2'-0" A.F.F. AT ALL KITCHEN PARTITION AND WALL BASES UNLESS NOTED OTHERWISE.

# 03 OFFICE ELEVATION

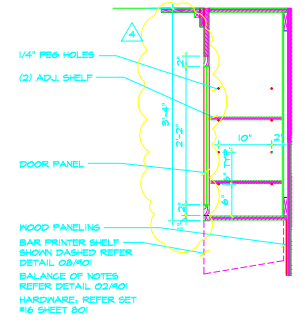
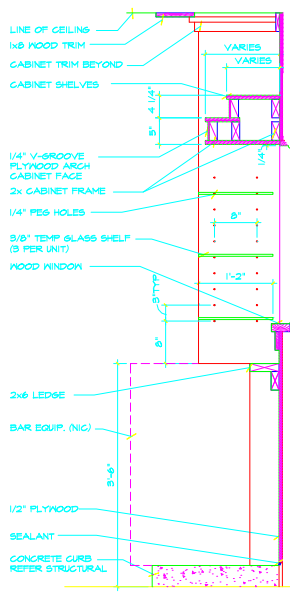
1/2" = 1'-0"



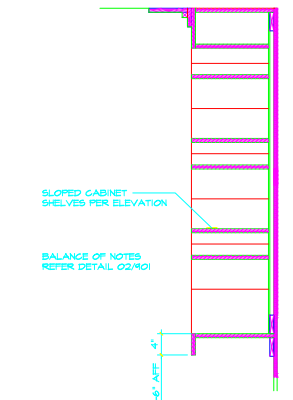
# 08 KITCHEN PREP/DISH WASH

1/4" = 1'-0"

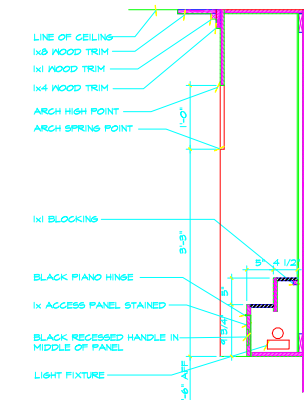




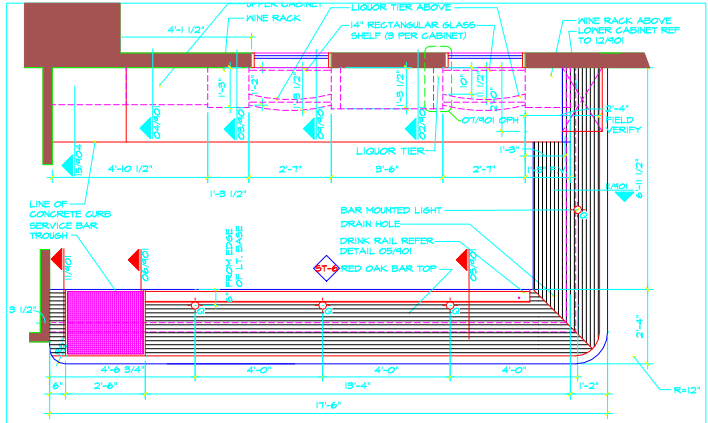
**04 BACKBAR CABINET-STAIN GRADE**  
1" = 1'-0"



**03 BACKBAR WINE RACK-STAIN GRADE**  
1" = 1'-0"



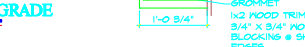
**02 BACKBAR LIQUOR TIER-STAIN GRADE**  
1" = 1'-0"



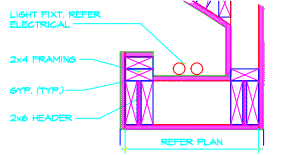
**01 ENLARGED BAR**  
1/2" = 1'-0"



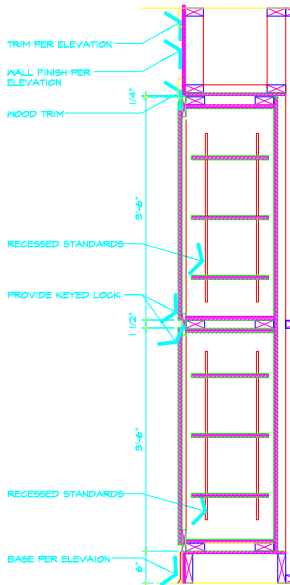
**09 BACKBAR AT WINDOW-STAIN GRADE**  
1" = 1'-0"



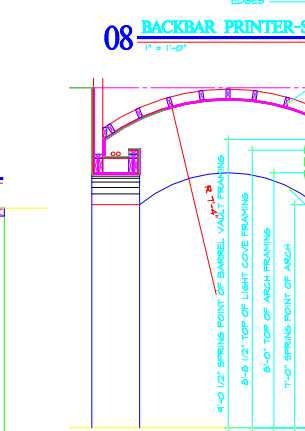
**08 BACKBAR PRINTER-STAIN GRADE**  
1" = 1'-0"



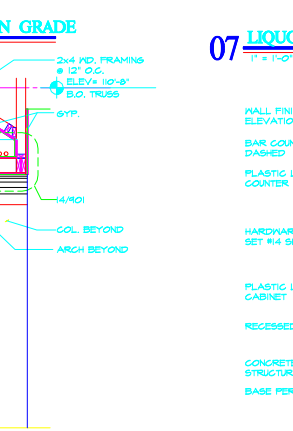
**14 LIGHT COVER**  
1 1/2" = 1'-0"



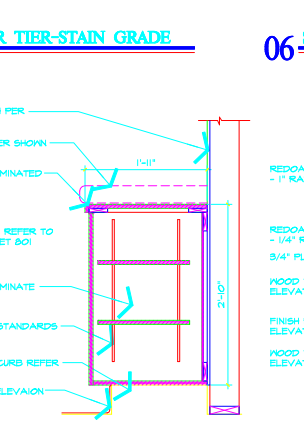
**18 TO-GO CABINET**  
1" = 1'-0"



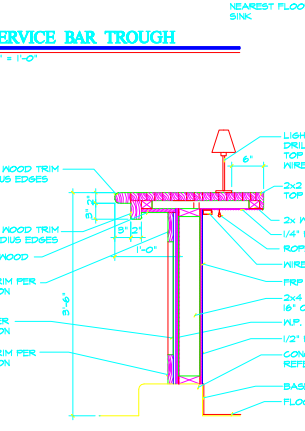
**13 BARRELL VAULT**  
1/2" = 1'-0"



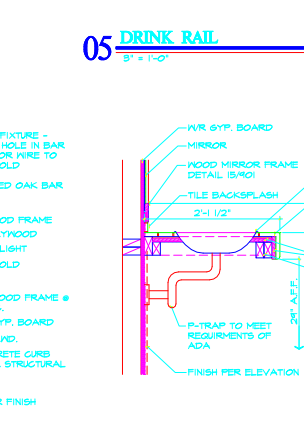
**07 LIQUOR TIER-STAIN GRADE**  
1" = 1'-0"



**12 BACK BAR LOWER CABINET-PLASTIC LAMINATE**  
1" = 1'-0"

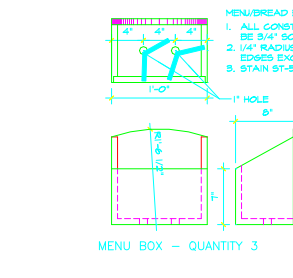


**11 BAR SECTION**  
1" = 1'-0"

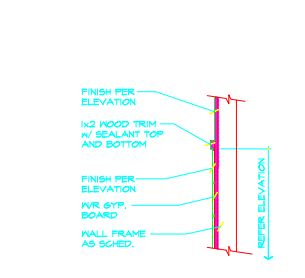
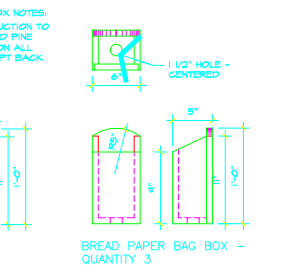


**05 DRINK RAIL**  
3\"/>

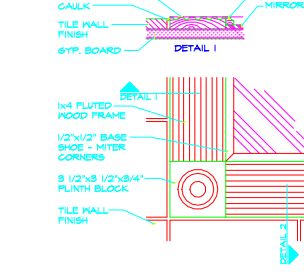
**10 TOILET ROOM COUNTER**  
1" = 1'-0"



**17 MENU/BREAD BOXES**  
1 1/2" = 1'-0"



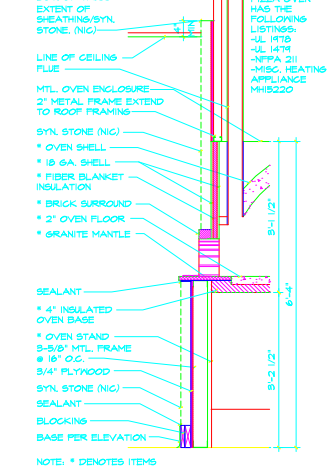
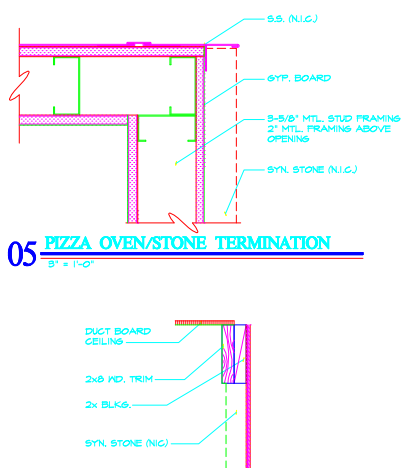
**16 TILE WAINSCOT CAP**  
1" = 1'-0"



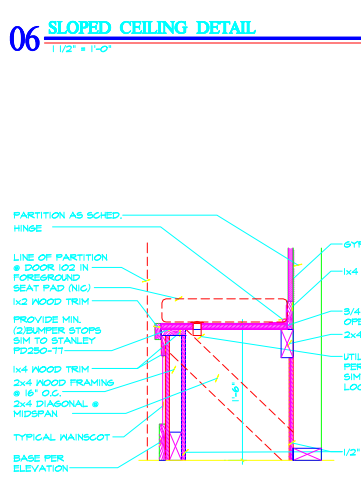
**15 MIRROR TRIM**  
3\"/>

28 JAN. 2004  
OWNER COMMENTS

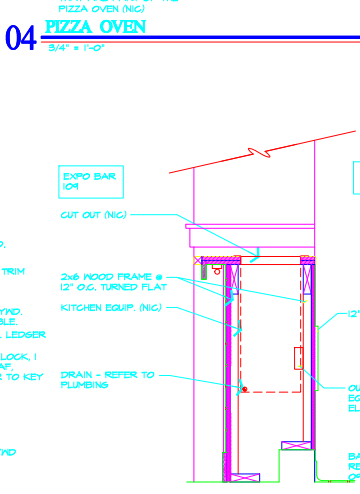
**05 PIZZA OVEN/STONE TERMINATION**  
3'0" x 1'0"



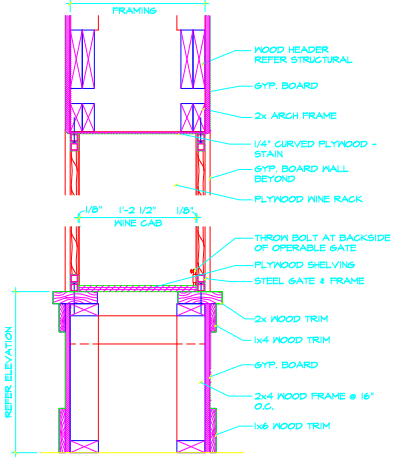
**06 SLOPED CEILING DETAIL**  
1'1/2" x 1'0"



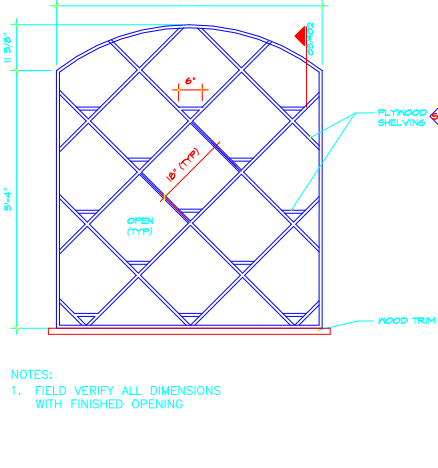
**04 PIZZA OVEN**  
3'4" x 1'0"



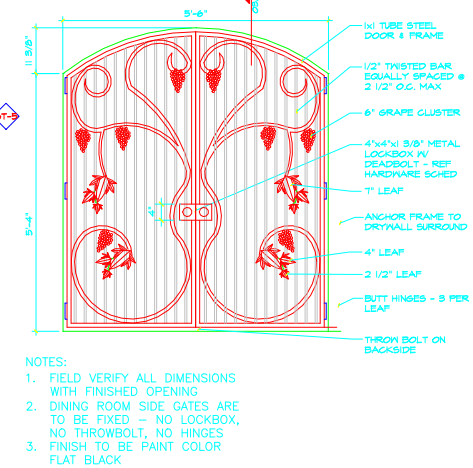
**03 WINE RACK**  
1'1/2" x 1'0"



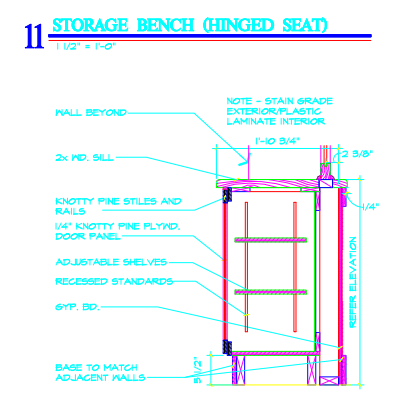
**02 WINE RACK**  
3'4" x 1'0"



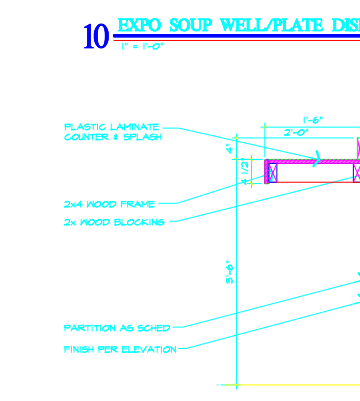
**01 WINE RACK GATE**  
3'4" x 1'0"



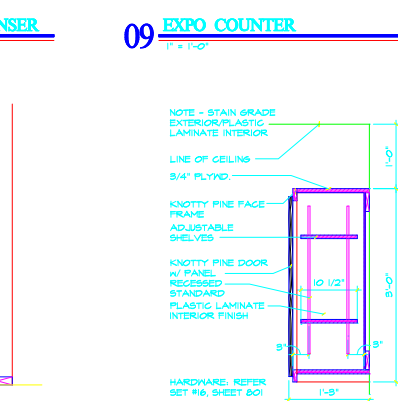
**11 STORAGE BENCH (HINGED SEAT)**  
1'1/2" x 1'0"



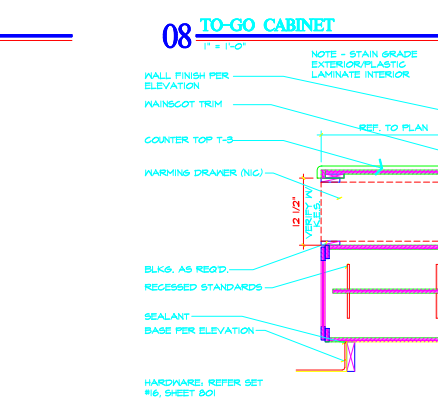
**10 EXPO SOUP WELL/PLATE DISPENSER**  
1'1" x 1'0"



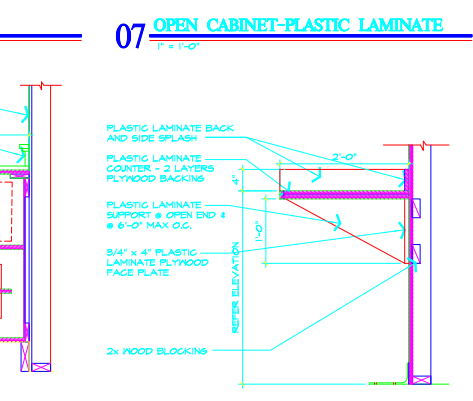
**09 EXPO COUNTER**  
1'1" x 1'0"



**08 TO-GO CABINET**  
1'1" x 1'0"



**07 OPEN CABINET-PLASTIC LAMINATE**  
1'1" x 1'0"



**16 HOSTESS COUNTER**  
1'1" x 1'0"



**15 COUNTER**  
1'1" x 1'0"



**14 POS UPPER CABINET**  
1'1" x 1'0"

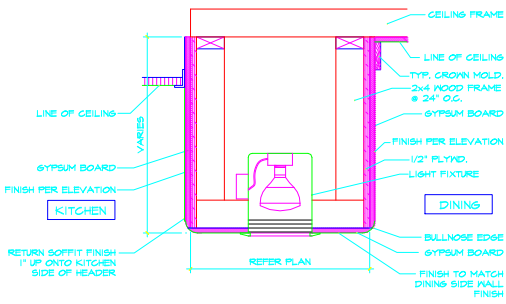


**13 TO-GO CABINET**  
1'1" x 1'0"

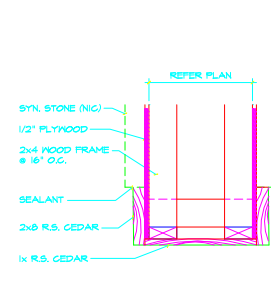


**12 OFFICE COUNTER-PLASTIC LAMINATE**  
1'1" x 1'0"

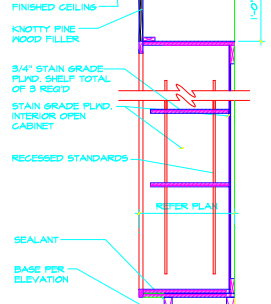




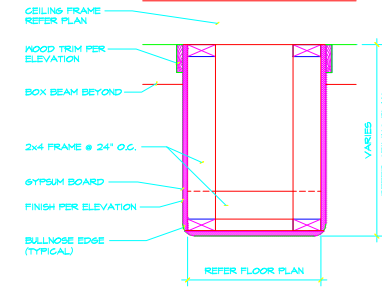
**05 ARCH AT PASS-OUT LINE**  
1 1/2" x 1'-0"



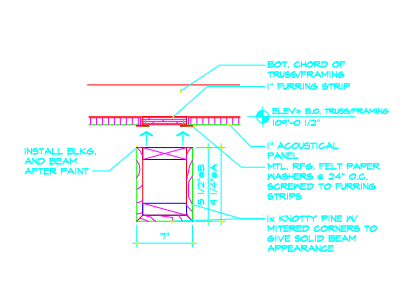
**04 WOOD/STONE HEADER**  
1 1/2" x 1'-0"



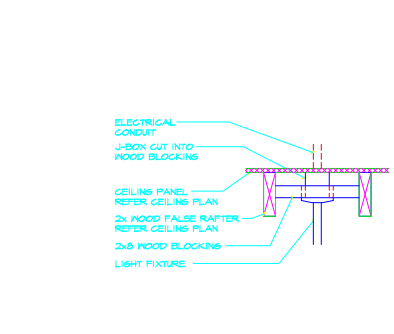
**03 TO-GO STAGING CAB.**  
1' x 1'-0"



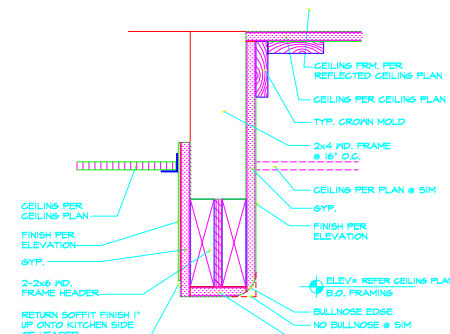
**02 GYP. BOARD HEADER**  
1 1/2" x 1'-0"



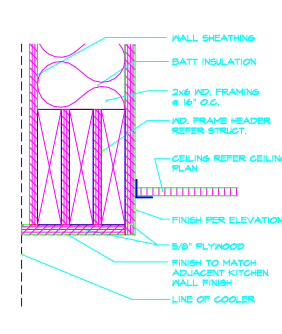
**01 BOX BEAM DETAIL**  
1 1/2" x 1'-0"



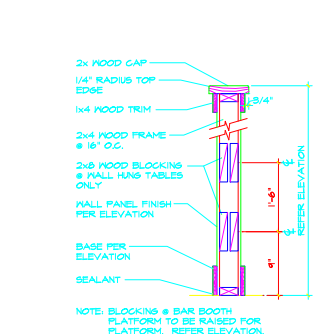
**10 LIGHT FIXTURE MOUNT**  
1 1/2" x 1'-0"



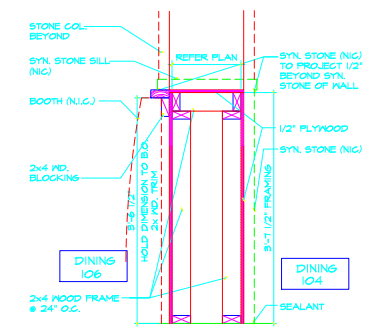
**09 HEADER AT KITCHEN/DINING CORR.**  
1' x 1'-0"



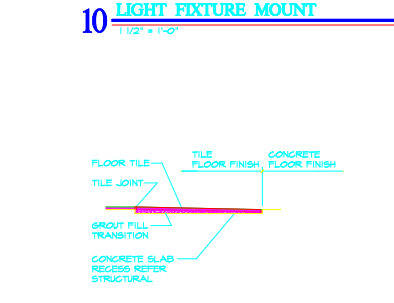
**08 HEADER AT COOLER DOOR**  
8\"/>



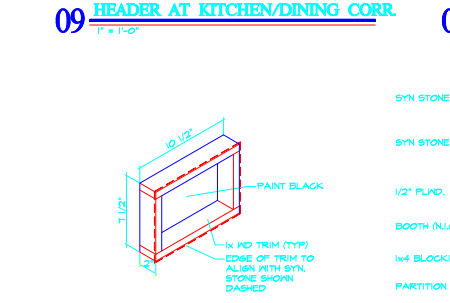
**07 TYPICAL LOW PARTITION**  
1' x 1'-0"



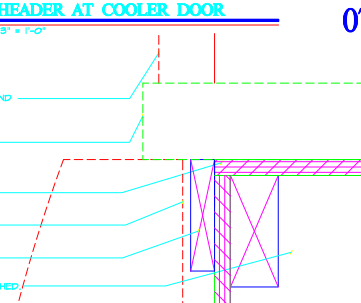
**06 SYN. STONE LOW PARTITION**  
1' x 1'-0"



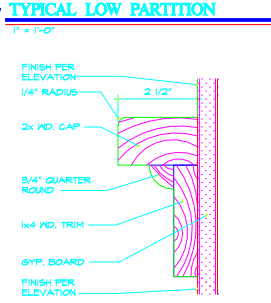
**15 FLOOR TILE TRANSITION**  
1' x 1'-0"



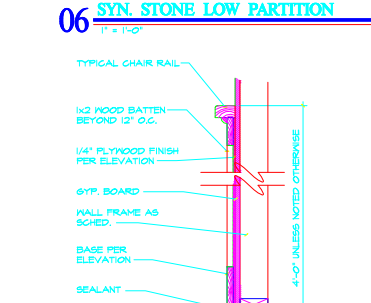
**14 WD. TRIM AT SYN. STONE FOR T-STAY**  
1 1/2" x 1'-0"



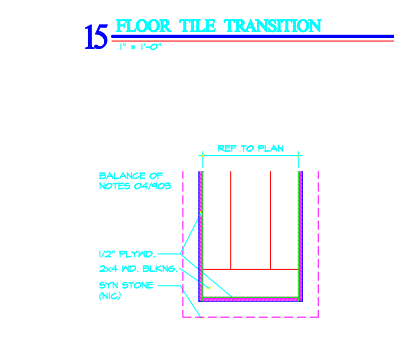
**13 WD. TRIM AT STONE MEETING BOOTH**  
8\"/>



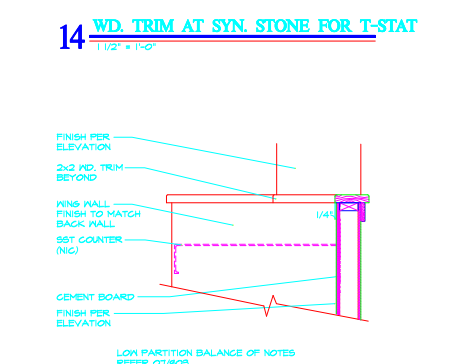
**12 TYPICAL CHAIR RAIL**  
8\"/>



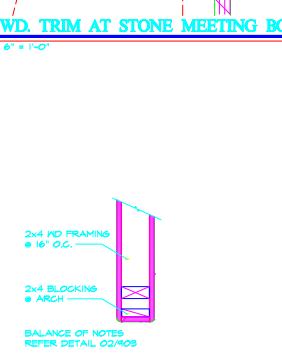
**11 TYPICAL WOOD WAINSCOT**  
1 1/2" x 1'-0"



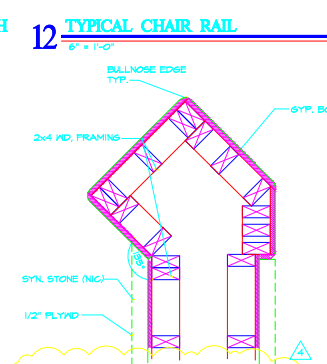
**20 SYN. STONE HEADER**  
1 1/2" x 1'-0"



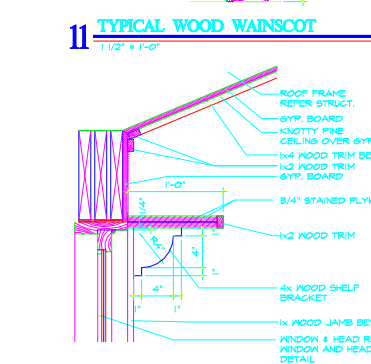
**19 DRINK STATION**  
1' x 1'-0"



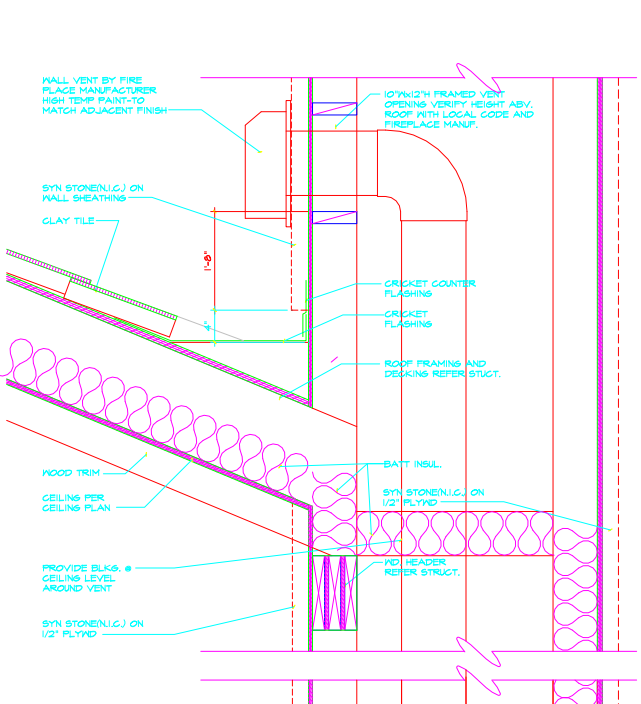
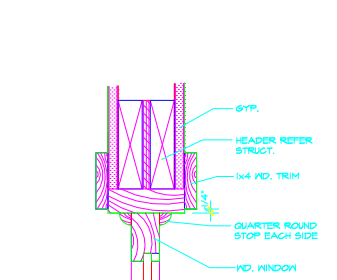
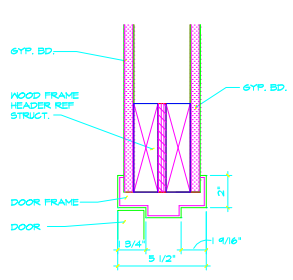
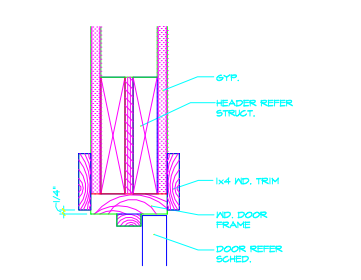
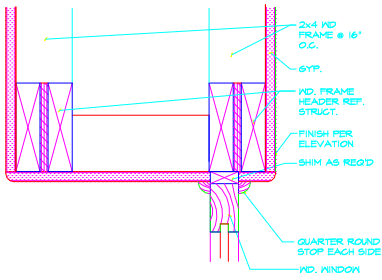
**18 GYP. BOARD HEADER**  
1 1/2" x 1'-0"



**17 GYP./STONE COLUMN**  
1 1/2" x 1'-0"



**16 SHELF**  
1 1/2" x 1'-0"

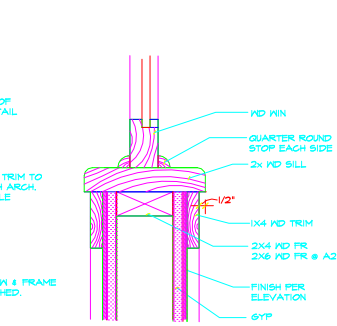
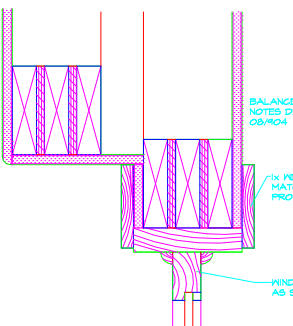
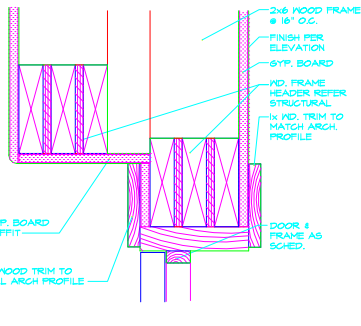


**05 HEAD WIN "K"**  
3" x 1'-0"

**04 TYP. INTERIOR DOOR FRAME**  
3" x 1'-0"

**03 HEAD DOOR 121**  
3" x 1'-0"

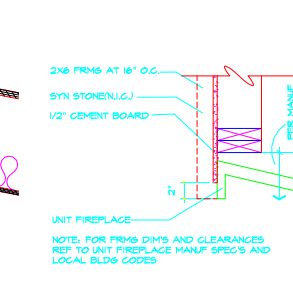
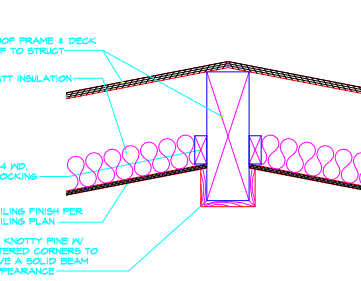
**02 TYP. INTERIOR WIN. HEAD**  
3" x 1'-0"



**08 HEAD DOOR 102**  
3" x 1'-0"

**07 HEAD WIN "A-2"**  
3" x 1'-0"

**06 SILL AT WIN. H, A-2**  
3" x 1'-0"

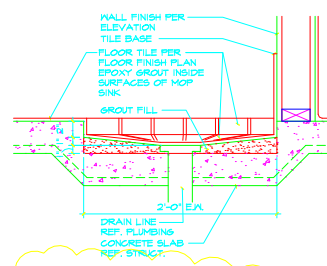
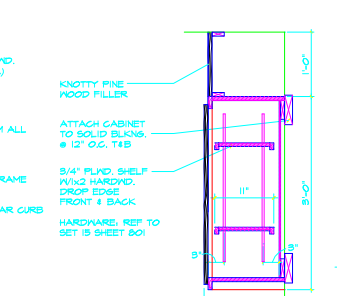
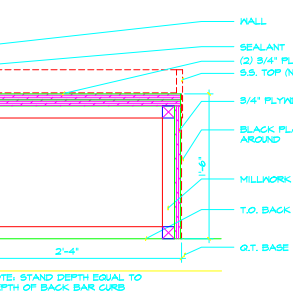
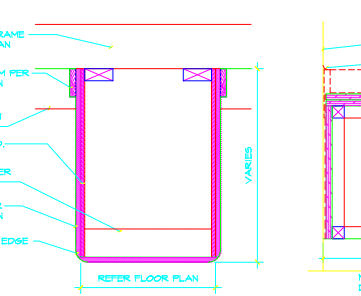


**09 NOT USED**

**12 EXPOSED BEAM TRIM**  
1 1/2" x 1'-0"

**11 FIREPLACE DETAIL**  
1 1/2" x 1'-0"

**10 NOT USED**  
3" x 1'-0"

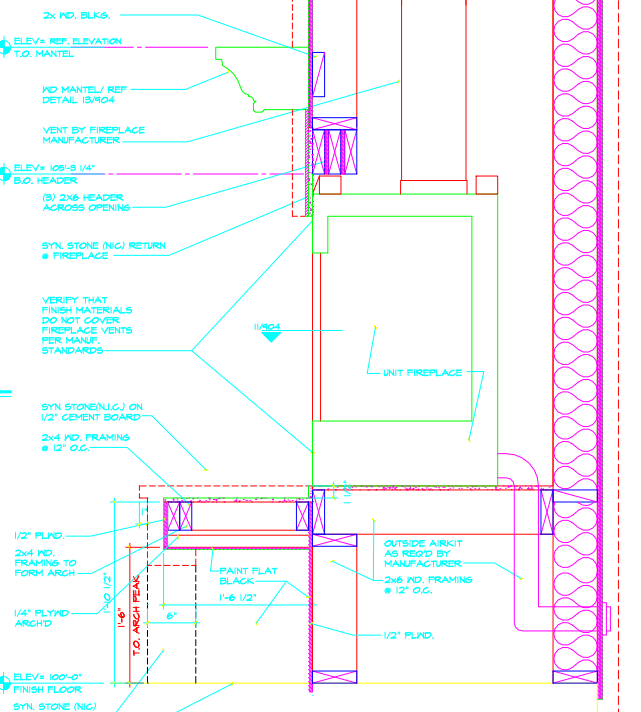


**13 MOP SINK**  
1 1/2" x 1'-0"

**16 ARCH HDR. DTL.**  
1 1/2" x 1'-0"

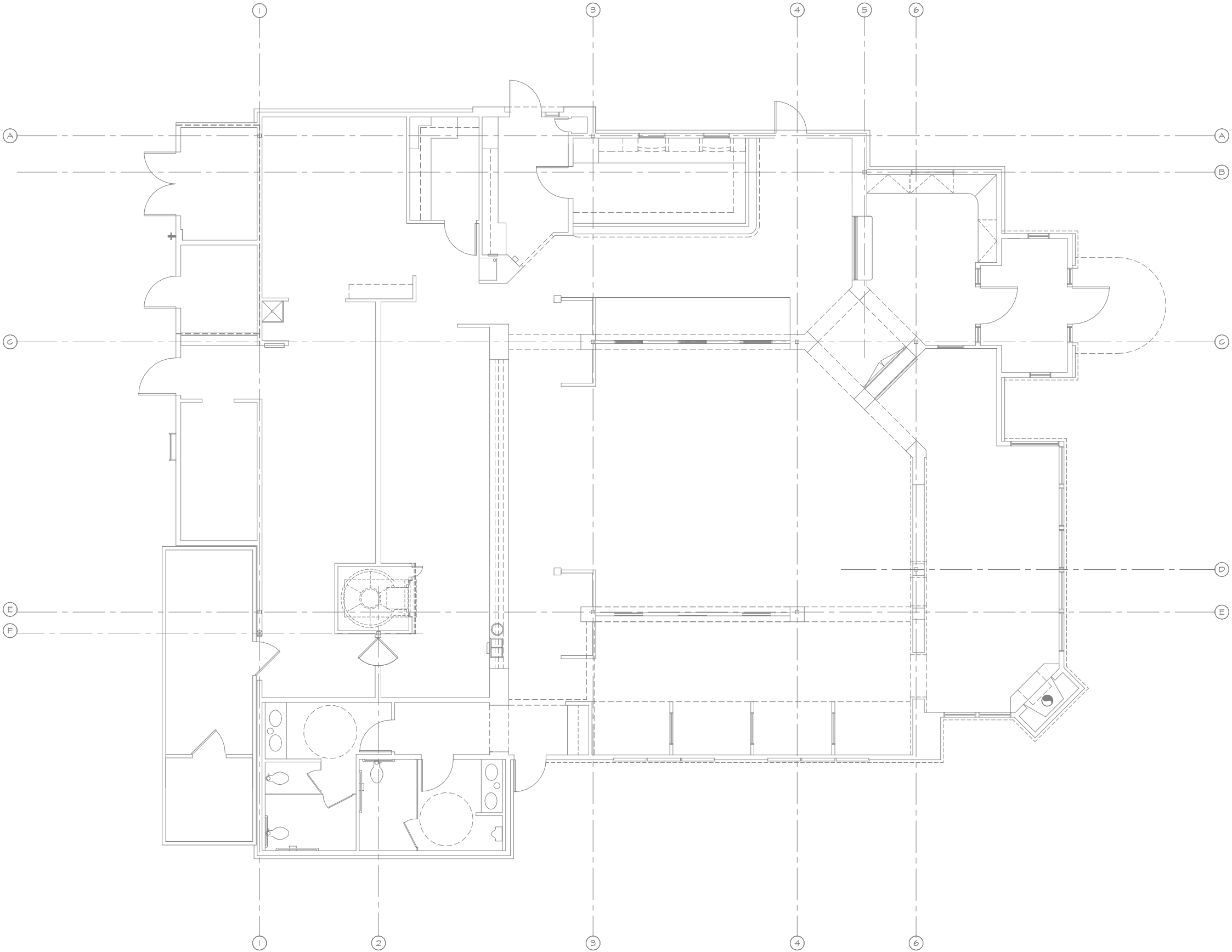
**15 DRINK MACHINE STAND**  
1 1/2" x 1'-0"

**14 TO-GO UPPER CABINET**  
1 1/2" x 1'-0"

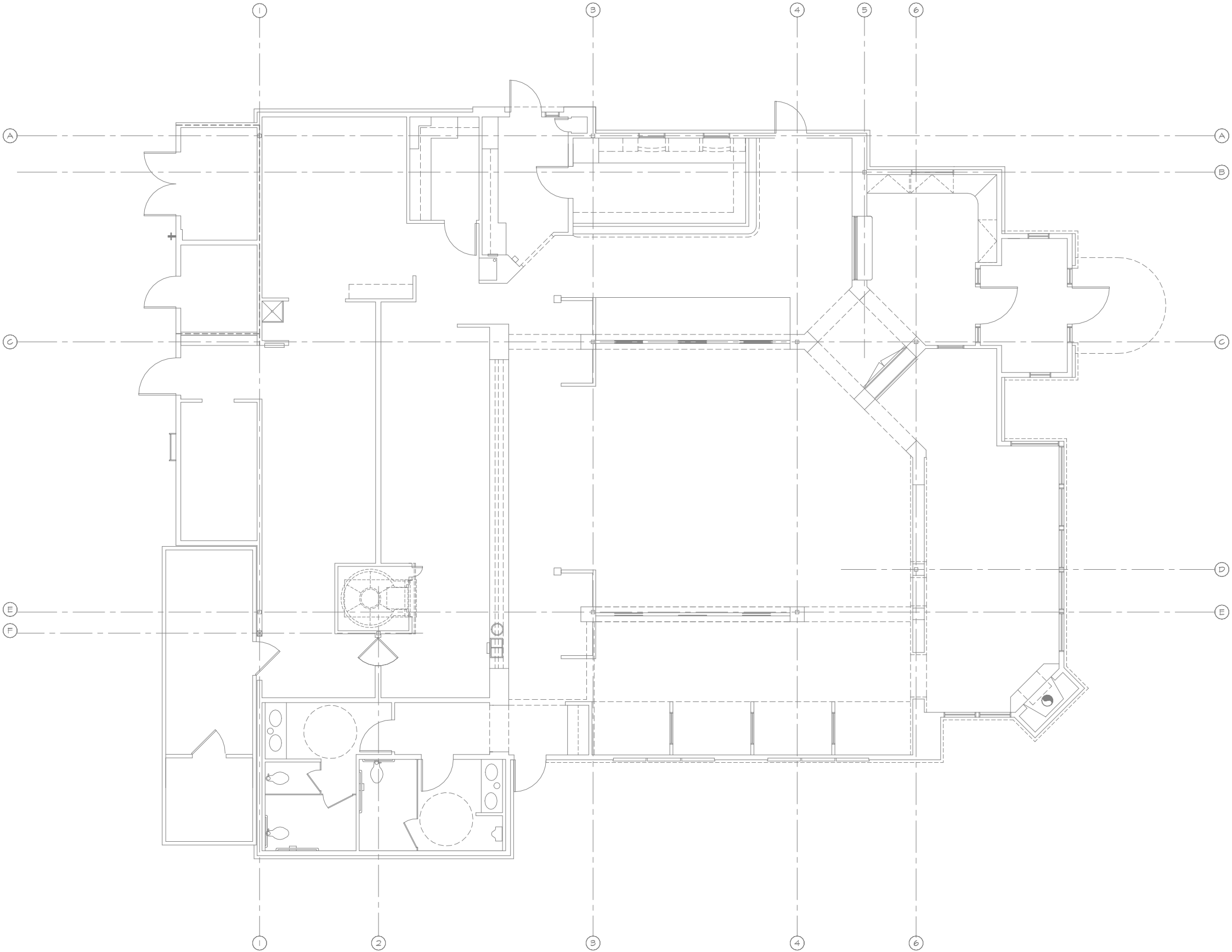


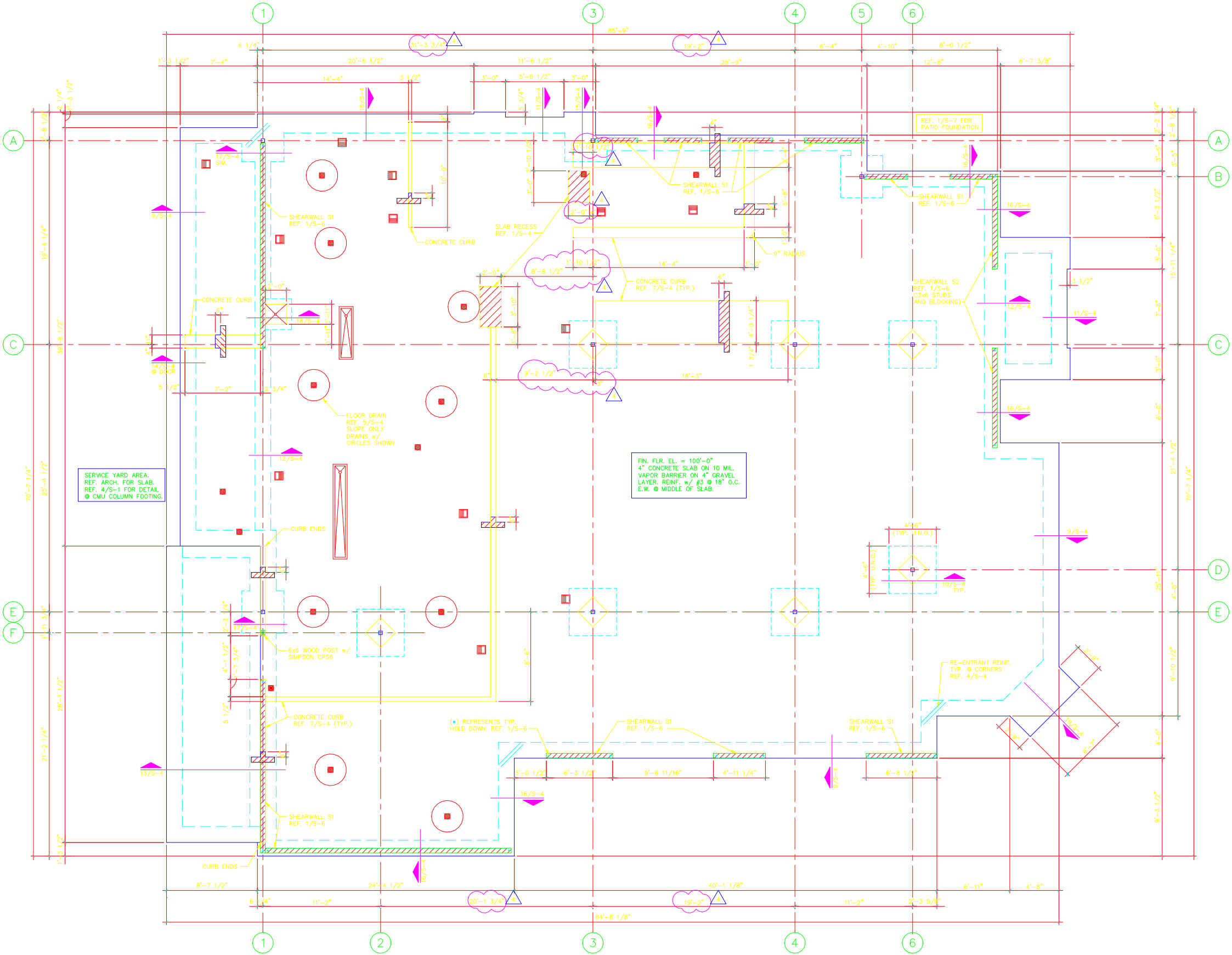
**01 NOT USED**  
1 1/2" x 1'-0"

**17 END WALL DINING 104**  
1 1/2" x 1'-0"









SERVICE YARD AREA.  
REF. ARCH. FOR SLAB.  
REF. 4/S-1 FOR DETAIL  
⊙ CMU COLUMN FOOTING.

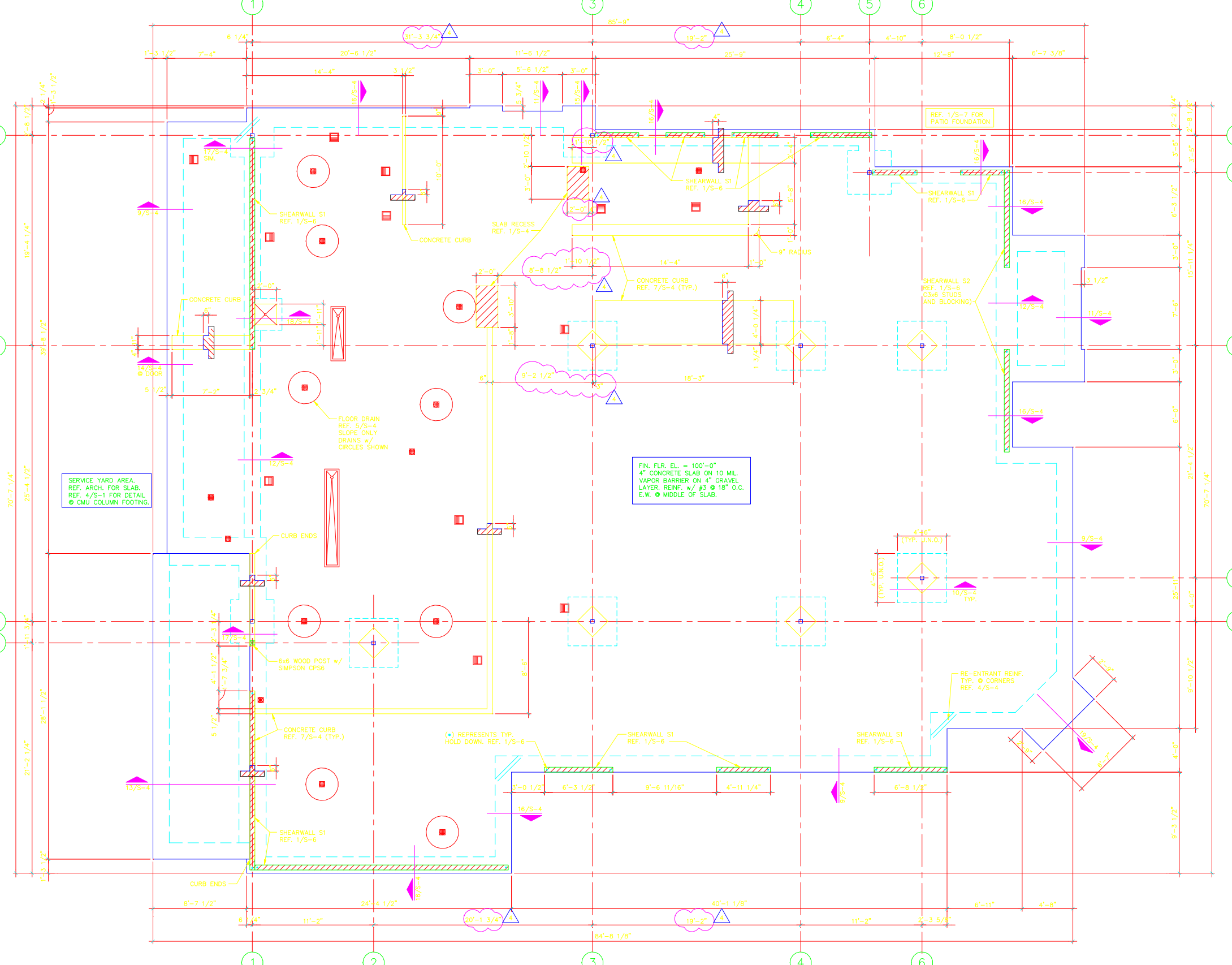
FIN. FLR. EL. = 100'-0"  
4" CONCRETE SLAB ON 10 MIL.  
VAPOR BARRIER ON 4" GRAVEL  
LAYER, REINF. w/ #3 @ 18" O.C.  
E.W. @ MIDDLE OF SLAB.

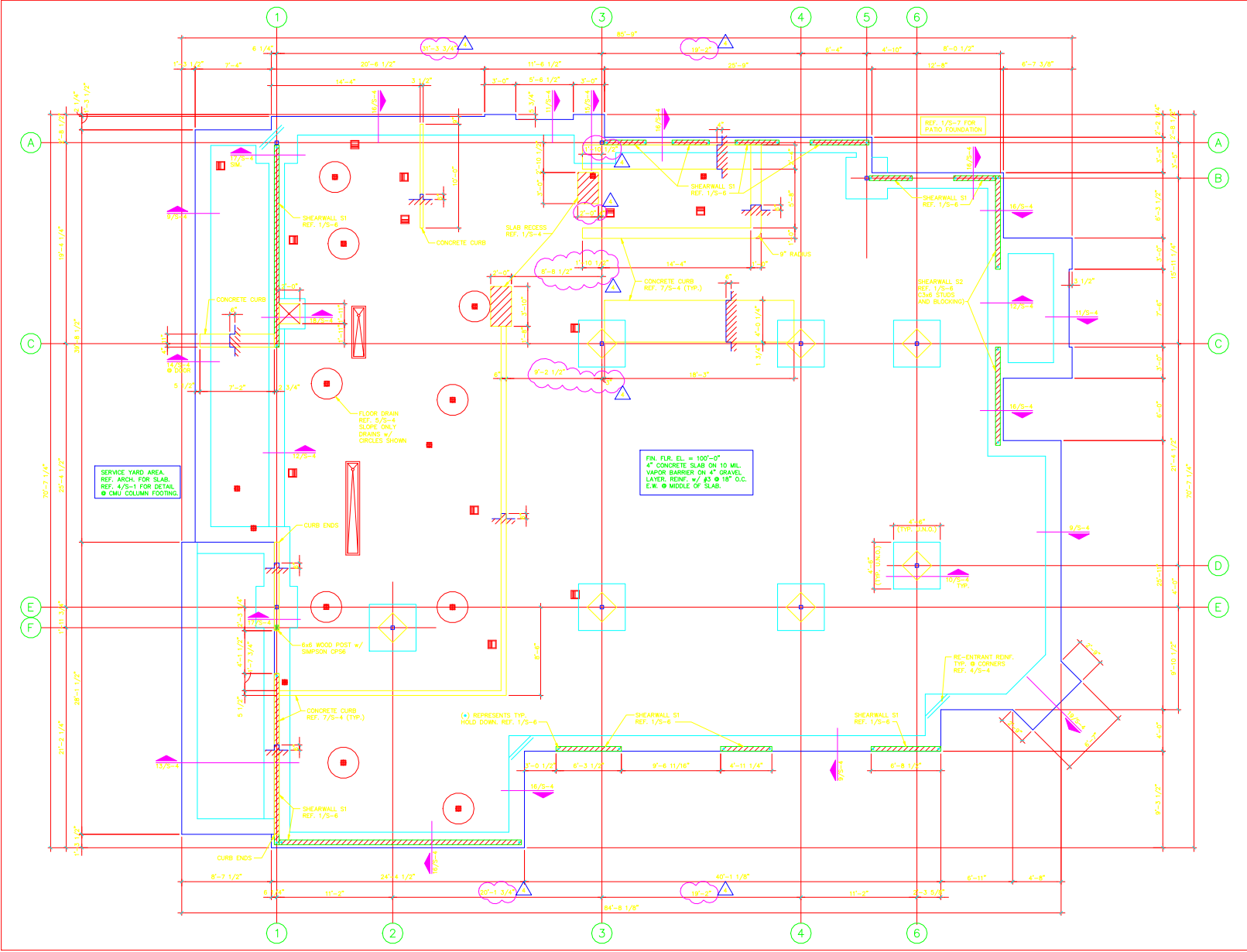
⊙ REPRESENTS TYP.  
HOLD DOWN. REF. 1/S-6

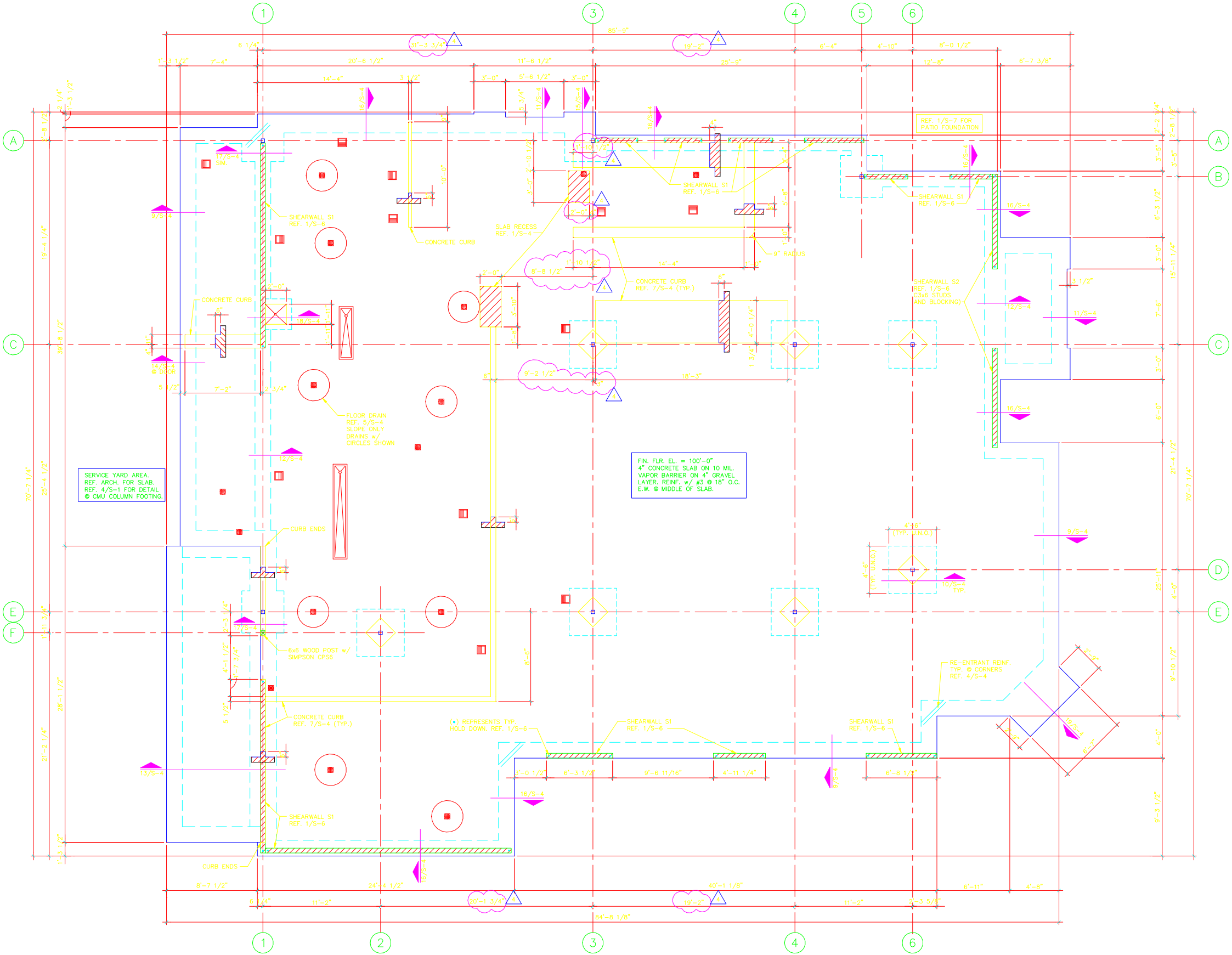
REF. 1/S-7 FOR PATIO FOUNDATION

SHEARWALL S2  
REF. 1/S-6  
C3x6 STUDS  
AND BLOCKING

RE-ENTRANT REINF.  
TYP. @ CORNERS  
REF. 4/S-4







SERVICE YARD AREA.  
REF. ARCH. FOR SLAB.  
REF. 4/S-1 FOR DETAIL  
⊙ CMU COLUMN FOOTING.

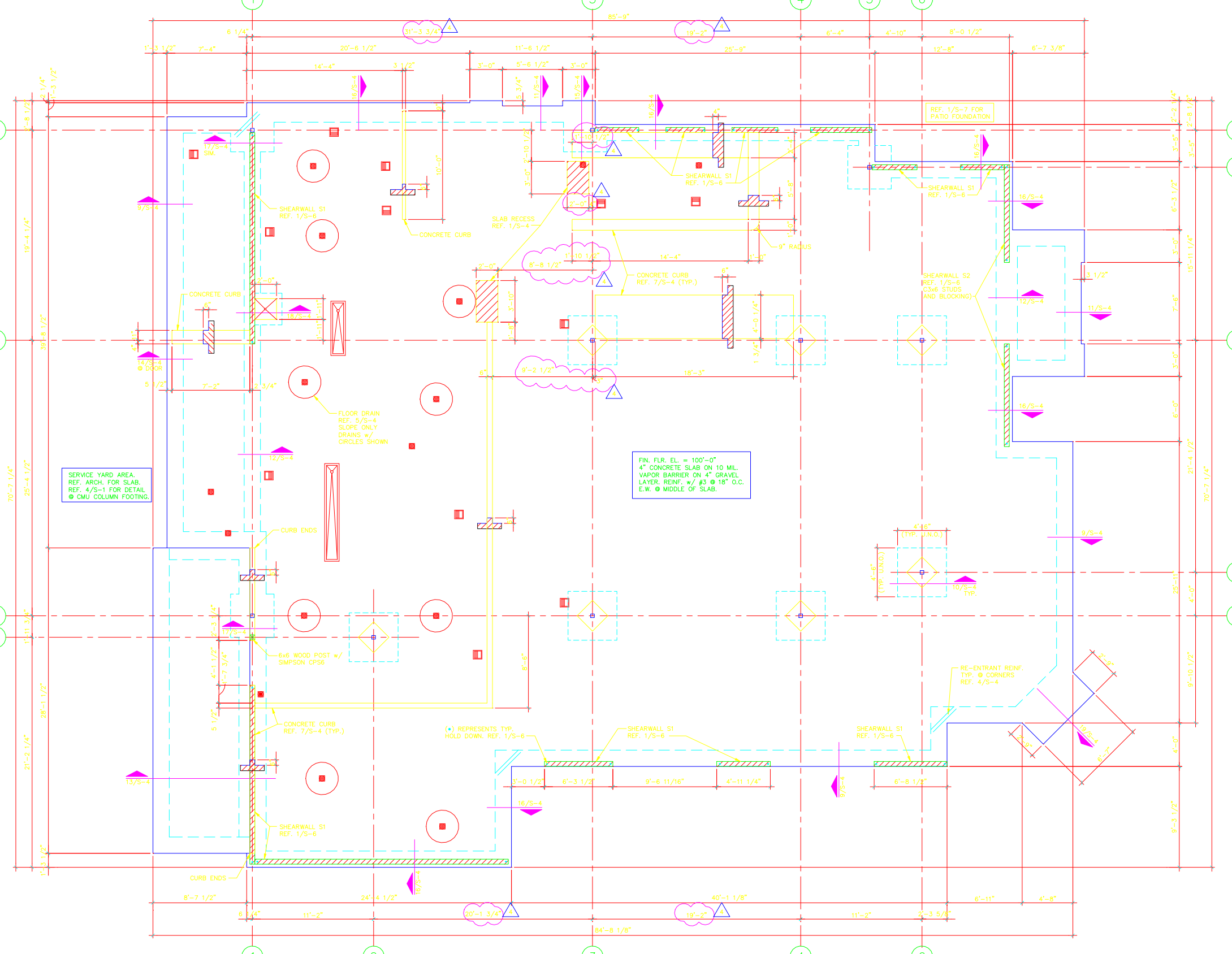
FIN. FLR. EL. = 100'-0"  
4" CONCRETE SLAB ON 10 MIL.  
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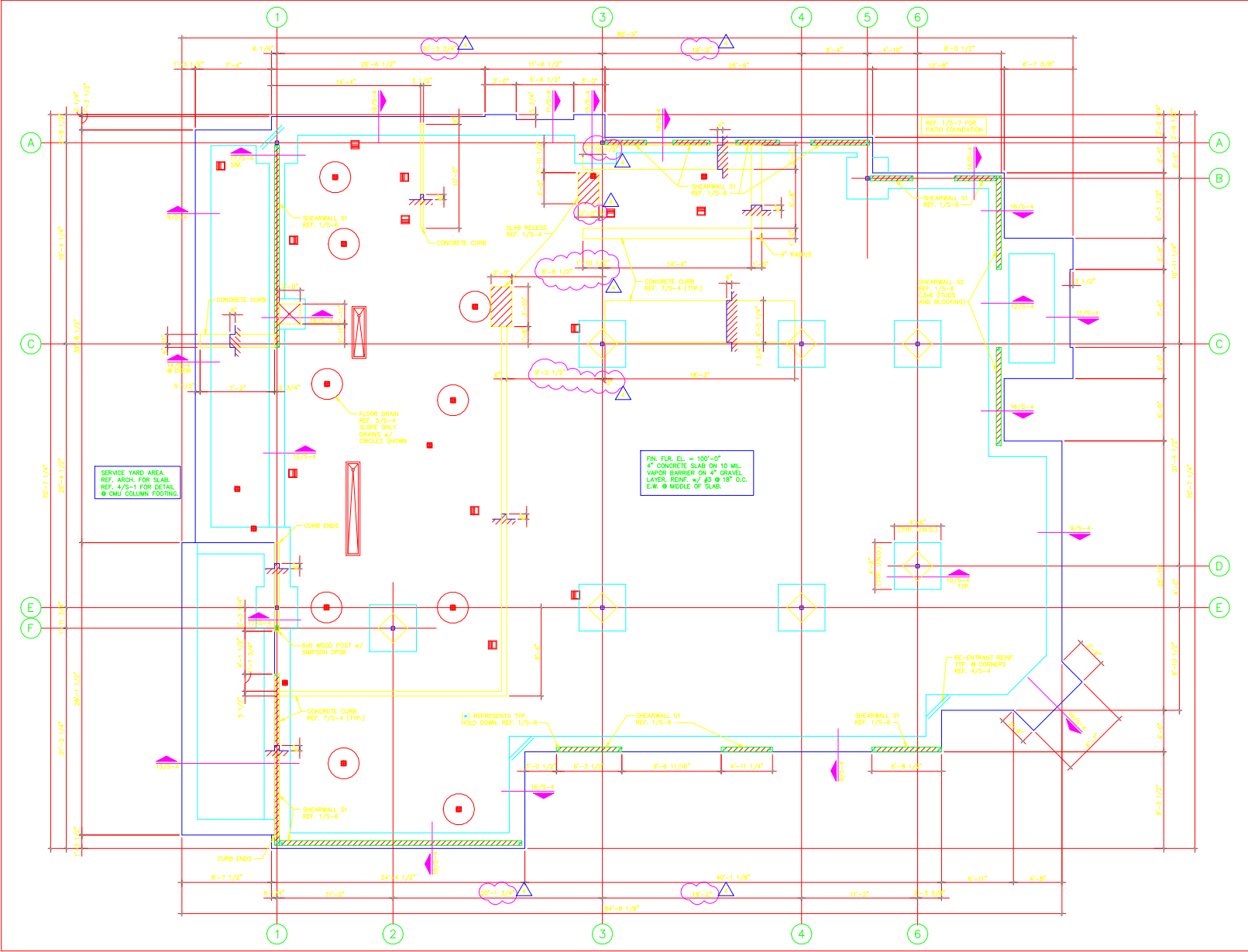
⊙ REPRESENTS TYP.  
HOLD DOWN. REF. 1/S-6

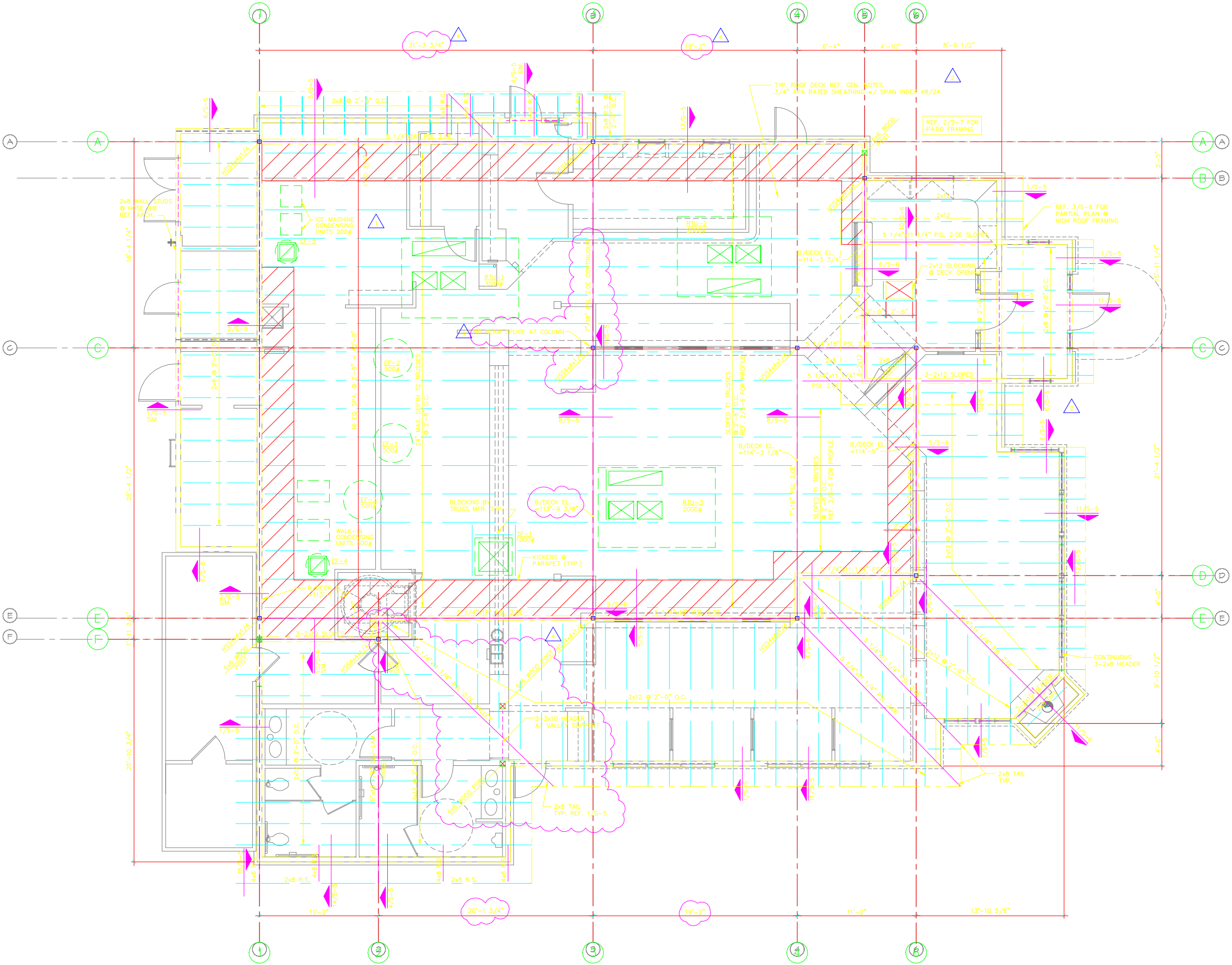
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SHEARWALL S2  
REF. 1/S-6  
C3x6 STUDS  
AND BLOCKING

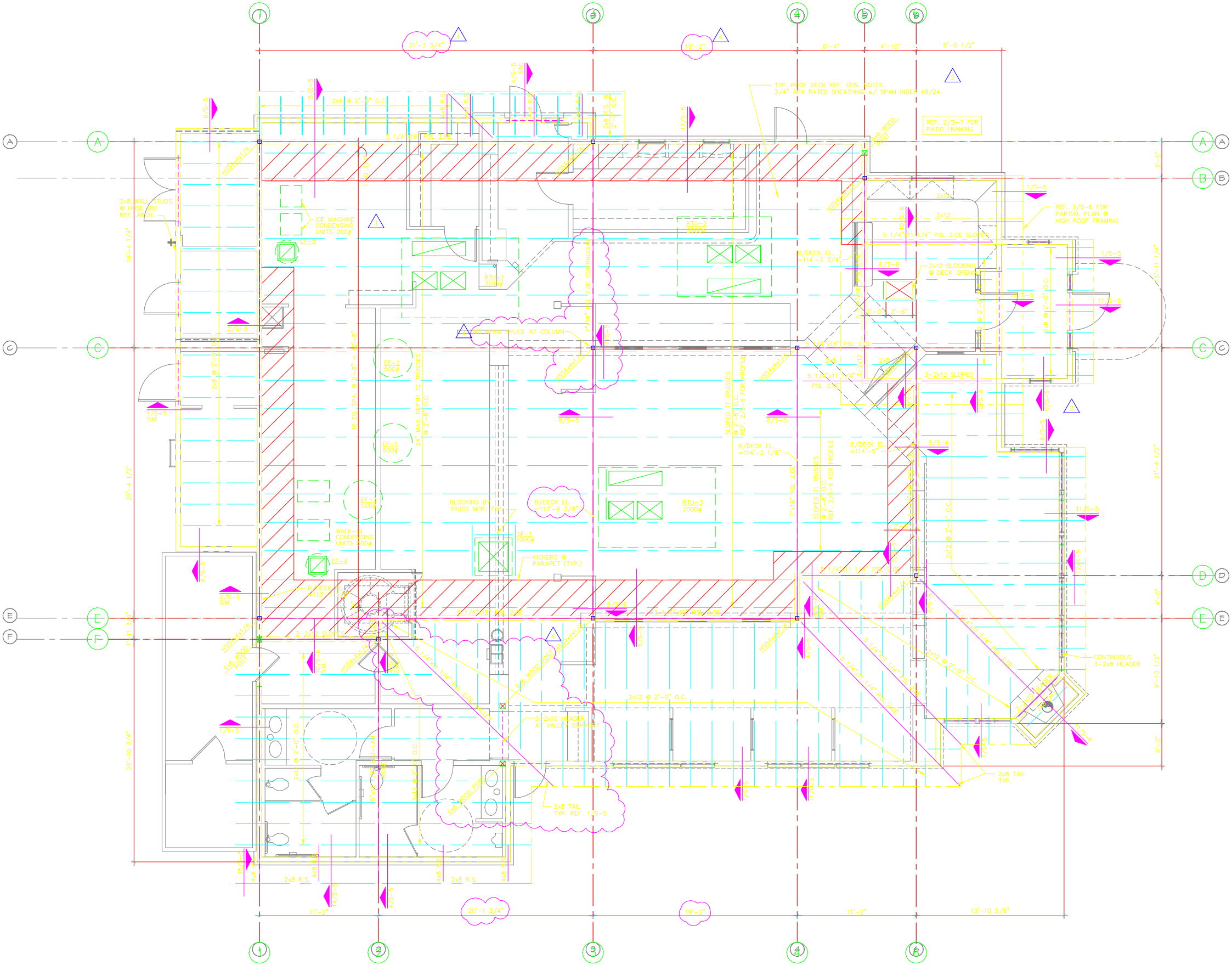
RE-ENTRANT REINF.  
TYP. @ CORNERS  
REF. 4/S-4















# GENERAL NOTES

## GENERAL

1. THE STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE UNIFORM BUILDING CODE, 1994 EDITION, AND ALL APPLICABLE REGULATIONS OF THE CITY OF CASA GRANDE, ARIZONA.

2. THE LOADS THAT HAVE BEEN USED IN THE STRUCTURAL DESIGN INCLUDE THE FOLLOWING:

DEAD LOADS AT ROOF:  
 ROOF JOISTS 4 PSF  
 ROOF DECK AND INSULATION 4 PSF  
 ROOFING 9 PSF TYP. (10 PSF @ CLAY TILE ROOF)  
 CEILING 2 PSF  
 MEP (INCLUDING SPRINKLERS) 6 PSF  
 TOTAL = 20 PSF @ CLAY TILE ROOF

LIVE LOAD AT ROOF 20 PSF

WIND LOADS:  
 BASIC WIND SPEED 75 MPH  
 EXPOSURE C  
 NET UPLIFT FIELD = 10 PSF PERIMETER = 10 PSF

SEISMIC LOADS:  
 ZONE 1  
 SOIL TYPE SD

ALL LIVE LOADS HAVE BEEN REDUCED BASED ON TRIBUTARY AREAS IN ACCORDANCE WITH CODE PROVISIONS. UNLESS NOTED OTHERWISE, BOTH ALTERNATE AND ADJACENT SPACES WHERE APPROPRIATE TO DERIVE GOVERNING CONDITIONS.

ALLOWABLE STRESSES HAVE BEEN INCREASED BY 1/3 FOR LOADING COMBINATIONS INCLUDING WIND AND SEISMIC WHERE ALLOWED BY CODE.

DEAD LOADS HAVE BEEN CALCULATED TO INCLUDE THE ACTUAL WEIGHT OF ALL WORK SHOWN ON THE STRUCTURAL, MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS. NO OTHER EQUIPMENT SHALL BE PLACED ON OR HUNG FROM THE ROOF SYSTEM WITHOUT PRIOR WRITTEN APPROVAL OF THE ENGINEER. ROOF-MOUNTED HVAC UNITS SHALL BE PLACED WITHIN THE DESIGNATED AREAS SHOWN ON THE FRAMING PLANS.

3. COMPLETE SHOP DRAWINGS FOR THE STRUCTURAL WORK SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO COMMENCEMENT OF CONSTRUCTION. IN ACCORDANCE WITH THE SPECIFICATIONS, SUCH REVIEW BY THE ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR CORRECT FABRICATION AND CONSTRUCTION OF THE WORK. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR MATERIALS PURCHASED PRIOR TO REVIEW OF SHOP DRAWINGS.

4. ANY DEVIATION FROM, ADDITION TO, SUBSTITUTION FOR, OR MODIFICATION TO THE STRUCTURE OR ANY PART OF THE STRUCTURE DETAILED ON THESE DRAWINGS SHALL BE SUBMITTED IN WRITING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS THAT ARE SUBMITTED FOR REVIEW DO NOT CONSTITUTE "WRITING" UNLESS IT IS CLEARLY NOTED THAT SPECIFIC CHANGES ARE BEING SUGGESTED.

5. THE CONTRACTOR SHALL REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND FOR EXACT LOCATIONS OF ALL ARCHITECTURAL DETAILS. THE CONTRACTOR SHALL COMPARE THE STRUCTURAL SECTIONS WITH THE ARCHITECTURAL SECTIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO COMPLETION OF THE SHOP DRAWINGS.

6. THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH THE WORK.

7. PRINCIPAL OPENINGS ARE SHOWN ON THE STRUCTURAL DRAWINGS. CONTRACTOR SHALL REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, CURBS, INSERTS AND OTHER OPENINGS NOT SHOWN. THE CONTRACTOR SHALL PROVIDE FOR ALL OPENINGS, WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT, SIZE AND LOCATION OF ALL OPENINGS SHALL BE VERIFIED BY THE CONTRACTOR. ANY DEVIATION FROM OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION FOR APPROVAL PRIOR TO CONSTRUCTION.

8. THE STRUCTURAL DRAWINGS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.

9. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN AND OTHER PERSONS DURING CONSTRUCTION.

10. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL WORK AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.

11. CONSTRUCTION MATERIALS SHALL NOT BE STORED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED ON THE DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENFORCE THIS REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS.

12. FIELD INSPECTION REPORTS SHALL BE SUBMITTED TO THE ENGINEER AS OUTLINED IN THE SPECIFICATIONS. INSPECTIONS SHALL BE PROVIDED BY A QUALIFIED AGENCY HIRED BY THE OWNER.

## EARTHWORK AND FOUNDATIONS

1. THE FOUNDATION DESIGN IS BASED ON A SUBSURFACE EXPLORATION AND REPORT BY FOREE & VANN, INC. (PROJECT NO. 11637) DATED JULY 3, 2001. A GEOTECHNICAL ENGINEER SHALL VERIFY THAT SOILS OF THE DESIGN BEARING CAPACITY HAVEN BEEN ENCOUNTERED AND THAT THE BUILDING PAD IS SUITABLE FOR CONSTRUCTION.

2. THE FOUNDATION DESIGN IS BASED ON INDIVIDUAL SLAB DIFFERENTIAL MOVEMENTS OF ONE (1) INCH. IF THIS VALUE IS NOT ACCEPTABLE TO THE OWNER OR TENANTS, THE FOUNDATION DESIGN MUST BE REVISSED.

3. CONTINUOUS AND ISOLATED FOOTINGS ARE DESIGNED FOR AN ALLOWABLE NET BEARING PRESSURE OF 450 PSF FOR TOTAL LOAD; THESE VALUES ARE APPLICABLE FOR FOOTINGS BEARING ON NATIVE UNDISTURBED SOIL. EXTERIOR FOOTINGS MUST BEAR A MINIMUM OF 30 INCHES BELOW FINISH GRADE.

4. ALL SURFACE SOIL, VEGETATION, AND DEBRIS SHALL BE STRIPPED AND REMOVED FOR THE AREA OF THE BUILDING PAD AND AT LEAST THREE (3) FEET BEYOND THE BUILDING PERIMETER.

5. AFTER STRIPPING IS COMPLETE, THE EXPOSED SUBGRADE SHALL BE SCARIFIED TO A DEPTH OF TWELVE (12) INCHES, MUST BE HEAVILY CONDITIONED AND RECOMPACTED PER THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT.

6. ANY UNSTABLE SOILS DETECTED SHALL BE REMOVED AND REPLACED WITH STRUCTURAL FILL.

7. PLACE STRUCTURAL FILL TO PROPER ELEVATIONS. STRUCTURAL FILL SHALL CONSIST OF SOILS AS RECOMMENDED IN THE GEOTECHNICAL REPORT. PLACE A MINIMUM 12 INCHES OF EXPANSIVE FILL DIRECTLY BELOW THE 4 INCHES AGGREGATE BASE COURSE.

8. A QUALIFIED SOIL TECHNICIAN SHALL PERFORM SUFFICIENT IN-PLACE DENSITY TESTS DURING FILL OPERATIONS TO VERIFY THAT PROPER LEVELS OF COMPACTION ARE ATTAINED AND THAT FOOTINGS ARE BEARING ON THE PROPER MATERIALS.

9. CONCRETE FOR SLABS ON FILL SHALL BE PLACED OVER A CONTINUOUS 10 MIL POLYETHYLENE MOISTURE BARRIER OVER A 4 INCH LAYER OF AGGREGATE BASE COURSE CONSISTING OF FILL AS DESCRIBED IN THE GEOTECHNICAL REPORT. PLACE A MINIMUM OF 12 INCHES OF SELECT STRUCTURAL FILL DIRECTLY BELOW THE BASE COURSE.

10. THE ABOVE REQUIREMENTS ARE A SUMMARY OF THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. CONTRACTOR SHALL NOT BE ABSOLVED FROM THE RESPONSIBILITY OF REVIEWING AND COMPLYING WITH THE ENTIRE GEOTECHNICAL REPORT.

## CAST IN PLACE REINFORCED CONCRETE

1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE MOST RECENT EDITION OF ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE."

2. STEEL REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 60.

3. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185, GRADE 65. ALL WELDED WIRE FABRIC SHALL BE SUPPLIED IN FLAT SHEETS, NOT IN ROLLS.

4. ALL CONCRETE SHALL UTILIZE NORMAL WEIGHT AGGREGATE UNLESS NOTED OTHERWISE

5. MIXES SHALL BE DESIGNED TO PROVIDE CONCRETE WITH A 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.

6. ALL EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED. AIR CONTENT SHALL BE 3%-6%. INTERIOR SLABS SHALL NOT HAVE AIR-ENTRAINMENT GRADE BEAMS, RETAINING WALLS, AND FOOTINGS DO NOT REQUIRE AIR ENTRAINMENT.

7. CONCRETE SLUMPS SHALL BE AS FOLLOWS:  
 RAMPS & SLOPING SURFACES 3" MAX.  
 CONCRETE CONTAINING SUPERPLASTICIZER 8" MAX.  
 ALL OTHER CONCRETE 4"

8. THE TESTING LABORATORY SHALL BE NOTIFIED AFTER THE MILD STEEL REINFORCING AND EMBEDS ARE POSITIONED PRIOR TO EACH CONCRETE PLACEMENT. NO CONCRETE SHALL BE PLACED UNTIL THESE ITEMS ARE CHECKED AND APPROVED BY THE TESTING LABORATORY.

## WOOD CONSTRUCTION:

1. PREPARATION, FABRICATION, AND INSTALLATION OF WOOD MEMBERS AND THEIR FASTENINGS SHALL CONFORM TO ACCEPTED ENGINEERING PRACTICES AND TO THE REQUIREMENTS OF THE BUILDING CODE. ALL WORK SHALL BE PERFORMED BY EXPERIENCED FABRICATORS.

2. WOOD FRAMING MATERIALS:

A. TYP. WALL SHEATHING SHALL BE 1/2" EXPOSURE 1 APA RATED SHEATHING, INDEX 32/16, TYP. ROOF DECK SHALL BE 3/4" APA RATED SHEATHING, INDEX 48/24. AT TYPICAL OVERHANGS, PROVIDE 3/8" APA RATED SHEATHING OVER 3/8" T1-11 GROOVE SIDE DOWN WITH GROOVES PERPENDICULAR TO RAFTERS.

B. SHEAR WALL SHEATHING SHALL BE EXPOSURE 1 APA RATED STRUCTURAL 1 SHEATHING OF THICKNESS SPECIFIED. REFER TO SHEAR WALL DETAIL 1/S-6.

C. PSL AND LVL BEAMS SPECIFIED ON THIS PROJECT SHALL BE MANUFACTURED BY TRUSS JOIST MACMILLAN, AN EQUIVALENT PRODUCT MAY BE SUBSTITUTED UPON APPROVAL BY ENGINEER. MIN. BASIC ALLOWABLE BENDING FB=2900 PSI & MIN. ALLOW. SHEAR FV=290 PSI.

D. ALL WOOD FRAMING SHALL BE NO. 1 DOUG-FIR OR NO. 2 SOUTHERN PINE WITH A MINIMUM ALLOWABLE BENDING Fb = 975 PSI AND A MIN. ALLOWABLE SHEAR FV=90 PSI.

E. ALL GLU-LAM BEAMS SHALL BE NO. 2 SOUTHERN PINE WITH A MINIMUM ALLOWABLE BENDING Fb=2400 PSI, MIN. SHEAR FV=165 PSI & MIN. E=1,800 KSI.

3. ROOF DECKING:

A. PANEL SHALL BE APPLIED WITH LONG DIMENSION PERPENDICULAR TO FRAMING; ADJACENT ROW OF PANELS SHALL BE STAGGERED.

B. PANELS SHALL BE FASTENED TO SUPPORTS USING 8d NAILS SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS AND AT 6" O.C. AT SUPPORTED PANEL EDGES.

C. 1/8" SPACE SHALL BE ALLOWED AT ALL PANEL ENDS AND EDGES

D. PANELS SHALL BE INSTALLED CONTINUOUSLY OVER TWO OR MORE SPANS.

4. CONNECTIONS:

A. THE FRAMING CONTRACTOR SHALL FURNISH ALL HARDWARE REQUIRED TO MAKE CONNECTIONS BETWEEN VARIOUS ELEMENTS. UNLESS NOTED OTHERWISE, REFER TO NAILING SCHEDULE FOR SIZE OF NAILS.

B. SOLID BLOCKING SHALL BE PROVIDED BETWEEN ALL JOISTS AND TRUSSES AT EACH SUPPORT.

C. ALL CONNECTIONS NOT SPECIFICALLY DETAILED SHALL UTILIZE JOIST HANGERS, BEAM HANGERS, CONNECTORS, COLUMN BASES, ETC. THIS HARDWARE SHALL BE MANUFACTURED BY SIMPSON COMPANY AND SHALL BE SELECTED AND INSTALLED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS.

D. ALL BOLTED CONNECTIONS SHALL HAVE WASHERS AT BOLT HEAD AS WELL AS AT THE NUT.

5. TRUSS FABRICATION:

A. TRUSS FABRICATOR SHALL VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

B. LOADS FROM ALL EQUIPMENT SHOWN ON PLAN SHALL BE INCLUDED. REFER TO TRUSS PROFILE ON SHEET S-6

C. TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS PREPARED BY A REGISTERED ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.

D. TRUSS MANUFACTURER SHALL PROVIDE ALL NECESSARY BRIDGING, TRUSSES SHALL BE DESIGNED FOR THE NET WIND UPLIFT LOAD NOTED ABOVE.

E. ALL TRUSSES SHALL BE DESIGNED WITH THE FOLLOWING DEFLECTION CRITERIA:  
 - ALLOWABLE LL DEFLECTION - L/360  
 - ALLOWABLE TL DEFLECTION - L/240

F. ALL TRUSSES SHALL BE FASTENED TO SUPPORTS WITH GALVANIZED METAL CONNECTORS AS INDICATED IN FRAMING DETAILS.

G. ALL TRUSSES SHALL BE DESIGNED AND FABRICATED USING #3 SOUTHERN PINE OR #1 DOUGLAS-FIR PER THE RECOMMENDATIONS OF THE TP1 DESIGN SPECIFICATIONS.

## STEEL

1. STRUCTURAL STEEL SHALL BE NEW STEEL AND SHALL CONFORM TO THE A.I.S.C. "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS."

2. STRUCTURAL STEEL ROLLED SHAPES AND PLATES NOTED "GRADE 50" SHALL CONFORM TO ASTM A-572, GRADE 50. ALL OTHER ROLLED SHAPES AND PLATES SHALL CONFORM TO ASTM A-36.

3. STRUCTURAL STEEL PIPE SHALL CONFORM TO ASTM A-53, TYPES "E" OR "S", GRADE B OR ASTM 501.

4. STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A-500, GRADE B.

5. ALL STRUCTURAL STEEL CONNECTIONS AND DETAILS SHALL CONFORM TO THE A.I.S.C. "CODE OF STANDARD PRACTICE FOR STEEL, BUILDINGS AND BRIDGES."

6. UNFINISHED THREADED FASTENERS SHALL CONFORM TO ASTM A-307, GRADE A BOLTS AND NUTS WITH HEXAGONAL HEADS. UNFINISHED THREADED FASTENERS SHALL BE USED ONLY FOR BOLTED CONNECTIONS OF SECONDARY FRAMING MEMBERS TO PRIMARY MEMBERS, FOR TEMPORARY BRACING TO FACILITATE ERECTION, AND FOR ANCHORAGE TO CONCRETE OR MASONRY CONSTRUCTION.

7. BOLTED CONNECTIONS OF PRIMARY MEMBERS SHALL CONFORM TO THE REQUIREMENTS OF THE A.I.S.C. "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS." UNLESS NOTED OTHERWISE, ALL PRIMARY CONNECTIONS WITH HIGH STRENGTH BOLTS SHALL USE ASTM A-325 BOLTS AND HEAVY HEX NUTS.

8. WELDED CONSTRUCTION SHALL CONFORM TO AWS D-1, 1. "STRUCTURAL WELDING CODE." WELDING PROCESSES AND OPERATORS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURES." ELECTRODES FOR FIELD AND SHOP WELDS SHALL BE E70XX, UNLESS NOTED OTHERWISE.

9. SHEAR STUDS SHALL CONFORM TO ASTM A- 108, HEADED ANCHORS.

10. STEEL MEMBERS SHALL NOT BE SPLICED EXCEPT AS SHOWN ON THE DRAWINGS.

11. ALL EXTERIOR STEEL EXPOSED TO WEATHER, INCLUDING LINTELS, SHALL BE GALVANIZED.

## REINFORCED MASONRY

1. ALL REINFORCED MASONRY SHALL CONFORM TO THE PROVISIONS OF ACI 530.1/ASCE 6 (WITH EXCEPTIONS NOTED IN THE SPECIFICATIONS).

2. CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90, GRADE N, TYPE 1, LIGHTWEIGHT UNITS WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI.

3. MORTAR SHALL CONFORM TO ASTM C-270, TYPE M OR S. AGGREGATES FOR MORTAR SHALL CONFORM TO ASTM C-144. MORTAR SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS.

4. GROUT SHALL CONFORM TO ASTM C-476. AGGREGATES FOR GROUT SHALL CONFORM TO ASTM C-404. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. CONSOLIDATE ALL GROUT MASONRY WITH MECHANICAL VIBRATION.

5. MASONRY CONSTRUCTION SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (F'm) OF 1500 PSI AT 28 DAYS.

6. SPECIAL INSPECTION SHALL BE PROVIDED BY THE TESTING LAB AND REPORTED TO THE ENGINEER. THE TESTING LAB SHALL PERFORM PERIODIC INSPECTIONS DURING PREPARATION AND TAKING OF PRISMS OR TEST SPECIMENS, PLACING OF REINFORCING AND GROUT, AND LAYING UP OF MASONRY.

7. BOND BEAMS SHALL BE CONSTRUCTED WITH PORTLAND CEMENT GROUT HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS, AND A MAXIMUM AGGREGATE SIZE OF 3/4".

8. REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60.

9. HORIZONTAL JOINT REINFORCEMENT SHALL BE FACTORY FABRICATED, TRUSS TYPE, 9 GAGE WIRE CONFORMING TO ASTM A-82. PROVIDE 1"-#4 MINIMUM ABOVE AND BELOW ALL WALL OPENINGS AND EXTENDING 24" MINIMUM INTO LANTO JAMBS.

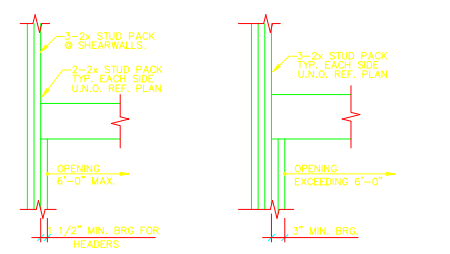
10. ALL CELLS CONTAINING REINFORCEMENT, BOLTS, OR OTHER METAL ANCHORS SHALL BE GROUNDED TO THE GROUND EITHER AT OR BELOW GRADE SHALL BE GROUNDED SOLID, WHETHER REINFORCED OR NOT.

## NAILING SCHEDULE

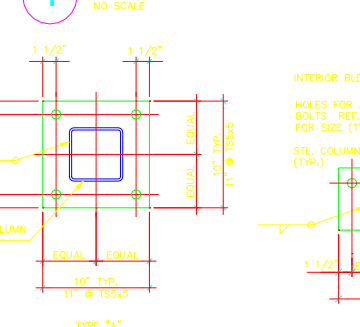
CONNECTION	NAILING
(1) JOIST TO SILL OR GIRDER, TOENAIL	3-16d
(2) BRIDGING TO JOIST, TOENAIL EACH END.	2-8d
(3) 1" x 6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	2-8d
(4) WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST, FACE NAIL	3-8d
(5) 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND END	2-16d
(6) FACE PLATE TO JOIST OR BLOCKING, FACE NAIL	16d @ 16" O.C.
(7) TOP PLATE TO STUD, END NAIL	2-16d
(8) STUD TO SOLE PLATE,	4-8d TOENAIL OR 2-16d END NAIL
(9) DOUBLED STUDS, FACE NAIL	16d @ 24" O.C.
(10) DOUBLED TOP PLATES, FACE NAIL	16d @ 16" O.C.
(11) TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	2-16d
(12) CONTINUOUS HEADER, TWO PIECES.	16d @ 16" O.C. ALONG EACH EDGE
(13) CEILING JOISTS TO PLATE, TOENAIL	3-8d
(14) CONTINUOUS HEADER TO STUD, TOENAIL.	4-8d
(15) CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	3-16d
(16) CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	3-16d
(17) RAFTER TO HIP	3-10d TOENAIL OR 2-16d FACE NAIL
(18) 1" BRACE TO EACH STUD AND PLATE, FACE NAIL	2-8d
(19) 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL	2-8d
(20) WIDER THAN 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL	2-8d
(21) BUILT-UP CORNER STUDS.	3-8d 16d @ 24" O.C. 20d @ 32" O.C. @ TOP & BOT; STAGGERED
(22) BUILT-UP GIRDER AND BEAMS.	2-20d @ EACH SPLICE

HEADER SIZE MATERIAL AT EDGE	SUPPORTING ONLY CEILING AND ROOF
3-2x6	4'-0"
3-2x8	6'-0"
3-2x10	8'-0"
3-2x12	10'-0"

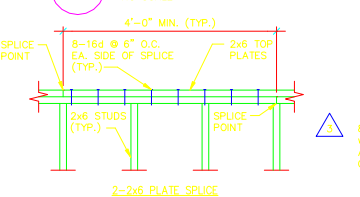
- 1.) HEADER SIZES MAY BE USED IF NO SIZES ARE CALLED OUT ON THE PLANS OR DETAILS.
- 2.) FOR WALL OPENING SIZES REFERENCE ARCHITECTURAL DRAWINGS.
- 3.) USE 1/2" FLITCH BETWEEN 3-2x HEADERS.



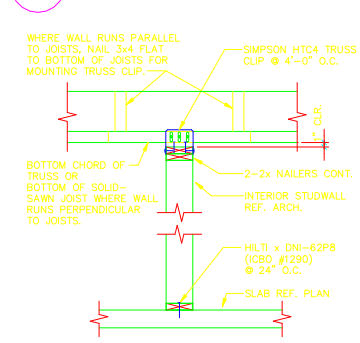
## 1. HEADER BEARINGS



## 2. TYPICAL BASE PLATE



## 3. TYPICAL SECTION

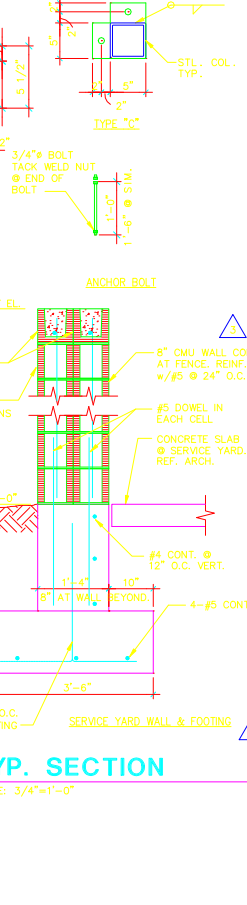


## 5. TYP. DETAIL

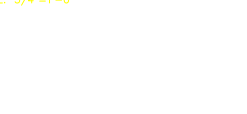


JOISTS	HANGER
2x8	U26
2x10	U28
2-2x12	U210-2
3-2x12	U210-3
5 1/4"x11 1/4" PSL	HQLTV5.511
5 1/4"x18" PSL	GLTV5.518

1. PROVIDE THESE HANGERS U.N.O. ON THE PLANS OR SECTIONS.
2. ALL HANGERS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE.
3. HANGERS FOR WOOD I-JOISTS SHALL BE SPECIFIED BY I-JOIST MANUFACTURER (U.N.O.).
4. SLOPE AND SKEW HANGERS AS REQ'D. REF. PLANS AND ARCH. ELEVATIONS.



## 4. TYP. SECTION



# GENERAL NOTES

## GENERAL

- THE STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE UNIFORM BUILDING CODE, 1994 EDITION, AND ALL APPLICABLE REGULATIONS OF THE CITY OF CASA GRANDE, ARIZONA.
- THE LOADS THAT HAVE BEEN USED IN THE STRUCTURAL DESIGN INCLUDE THE FOLLOWING:
 

DEAD LOADS AT ROOF:	
ROOF JOISTS	3 PSF
ROOF DECK AND INSULATION	4 PSF
ROOFING	5 PSF TYP. (10 PSF @ CLAY TILE ROOF)
CEILING	2 PSF
M.E.P. (INCLUDING SPRINKLERS)	2 PSF
TOTAL = 20 PSF TYP. (25 PSF @ CLAY TILE ROOF)	
- LIVE LOAD AT ROOF: 20 PSF
- WIND LOADS:
 

BASIC WIND SPEED	75 MPH
EXPOSURE C	
NET UPLIFT	FIELD = 10 PSF
	PERIMETER = 10 PSF
- SEISMIC LOADS:
 

ZONE 1	
SOIL TYPE SD	
- ALLOWABLE STRESSES HAVE BEEN INCREASED BY 1/3 FOR LOADING COMBINATIONS INCLUDING WIND AND SEISMIC WHERE ALLOWED BY CODE.
- DEAD LOADS HAVE BEEN CALCULATED TO INCLUDE THE ACTUAL WEIGHT OF ALL WORK SHOWN ON THE STRUCTURAL, MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS. NO OTHER EQUIPMENT SHALL BE PLACED ON OR HUNG FROM THE ROOF SYSTEM WITHOUT PRIOR WRITTEN APPROVAL OF THE ENGINEER. ROOF-MOUNTED HVAC UNITS SHALL BE PLACED WITHIN THE DESIGNATED AREAS SHOWN ON THE FRAMING PLANS.
- COMPLETE SHOP DRAWINGS FOR THE STRUCTURAL WORK SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO COMMENCEMENT OF CONSTRUCTION. IN ACCORDANCE WITH THE SPECIFICATIONS, SUCH REVIEW BY THE ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR CORRECT FABRICATION AND CONSTRUCTION OF THE WORK. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR MATERIALS PURCHASED PRIOR TO REVIEW OF SHOP DRAWINGS.
- ANY DEVIATION FROM, ADDITION TO, SUBSTITUTION FOR, OR MODIFICATION TO THE STRUCTURE OR ANY PART OF THE STRUCTURE DETAILED ON THESE DRAWINGS SHALL BE SUBMITTED IN WRITING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS THAT ARE SUBMITTED FOR REVIEW DO NOT CONSTITUTE "IN-WRITING" UNLESS IT IS CLEARLY NOTED THAT SPECIFIC CHANGES ARE BEING SUGGESTED.
- THE CONTRACTOR SHALL REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS NOT SHOWN AND FOR EXACT LOCATIONS OF ALL ARCHITECTURAL DETAILS. THE CONTRACTOR SHALL COMPARE THE STRUCTURAL SECTIONS WITH THE ARCHITECTURAL SECTIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO COMPLETION OF THE SHOP DRAWINGS.
- THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH THE WORK.
- PRINCIPAL OPENINGS ARE SHOWN ON THE STRUCTURAL DRAWINGS. CONTRACTOR SHALL REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, CURBS, INSERTS AND OTHER OPENINGS NOT SHOWN. THE CONTRACTOR SHALL PROVIDE FOR ALL OPENINGS, WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT, SIZE AND LOCATION OF ALL OPENINGS SHALL BE VERIFIED BY THE CONTRACTOR. ANY DEVIATION FROM OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION FOR APPROVAL PRIOR TO CONSTRUCTION.
- THE STRUCTURAL DRAWINGS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.
- THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHODS OF CONSTRUCTION UNLESS SO NOTED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN AND OTHER PERSONS DURING CONSTRUCTION.
- THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL WORK AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.
- CONSTRUCTION MATERIALS SHALL NOT BE STORED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED ON THE DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENFORCE THIS REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS.
- FIELD INSPECTION REPORTS SHALL BE SUBMITTED TO THE ENGINEER AS OUTLINED IN THE SPECIFICATIONS. INSPECTIONS SHALL BE PROVIDED BY A QUALIFIED AGENCY HIRED BY THE OWNER.

## EARTHWORK AND FOUNDATIONS

- THE FOUNDATION DESIGN IS BASED ON A SUBSURFACE EXPLORATION AND REPORT BY FOREE & VANN, INC. (PROJECT NO. 11637) DATED JULY 3, 2001. A GEOTECHNICAL ENGINEER SHALL VERIFY THAT SOILS OF THE DESIGN BEARING CAPACITY HAVE BEEN ENCOUNTERED AND THAT THE BUILDING PAD IS SUITABLE FOR CONSTRUCTION.
- THE FOUNDATION DESIGN IS BASED ON POTENTIAL SLAB DIFFERENTIAL MOVEMENTS OF ONE (1) INCH. IF THIS VALUE IS NOT ACCEPTABLE TO THE OWNER OR TENANTS, THE FOUNDATION DESIGN MUST BE REVISED.
- CONTINUOUS AND ISOLATED FOOTINGS ARE DESIGNED FOR AN ALLOWABLE NET BEARING PRESSURE OF 1250 PSF FOR TOTAL LOAD. THESE VALUES ARE APPLICABLE FOR FOOTINGS BEARING ON NATIVE UNDISTURBED SOIL. EXTERIOR FOOTINGS MUST BEAR A MINIMUM OF 30 INCHES BELOW FINISH GRADE.
- ALL SURFACE SOIL, VEGETATION, AND DEBRIS SHALL BE STRIPPED AND REMOVED FOR THE AREA OF THE BUILDING PAD AND AT LEAST THREE (3) FEET BEYOND THE BUILDING PERIMETER.
- AFTER STRIPPING IS COMPLETE, THE EXPOSED SUBGRADE SHALL BE SCARIFIED TO A DEPTH OF TWELVE (12) INCHES, MOISTURE CONDITIONED AND RECOMPACTED PER THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT.
- ANY UNSTABLE SOILS DETECTED SHALL BE REMOVED AND REPLACED WITH STRUCTURAL FILL.
- PLACE STRUCTURAL FILL TO PROPER ELEVATIONS. STRUCTURAL FILL SHALL CONSIST OF SOILS AS RECOMMENDED IN THE GEOTECHNICAL REPORT. PLACE A MINIMUM 12 INCHES OF NON-EXPANSIVE FILL DIRECTLY BELOW THE 4 INCHES AGGREGATE BASE.

- A QUALIFIED SOIL TECHNICIAN SHALL PERFORM SUFFICIENT IN-PLACE DENSITY TESTS DURING FILL OPERATIONS TO VERIFY THAT PROPER LEVELS OF COMPACTION ARE ATTAINED AND THAT FOOTINGS ARE BEARING ON THE PROPER MATERIAL.
- CONCRETE FOR SLABS ON FILL SHALL BE PLACED OVER A CONTINUOUS 10 MIL POLYETHYLENE MOISTURE BARRIER OVER A 4 INCH LAYER OF AGGREGATE BASE COURSE CONSISTING OF FILL AS DESCRIBED IN THE GEOTECHNICAL REPORT. PLACE A MINIMUM OF 12 INCHES OF SELECT STRUCTURAL FILL DIRECTLY BELOW THE BASE COURSE.
- THE ABOVE REQUIREMENTS ARE A SUMMARY OF THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. CONTRACTOR SHALL NOT BE ABSOLVED FROM THE RESPONSIBILITY OF REVIEWING AND COMPLYING WITH THE ENTIRE GEOTECHNICAL REPORT.

## CAST-IN PLACE REINFORCED CONCRETE

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE MOST RECENT EDITION OF ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE".
- STEEL REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 60.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185, GRADE 65. ALL WELDED WIRE FABRIC SHALL BE SUPPLIED IN FLAT SHEETS, NOT IN ROLLS.
- ALL CONCRETE SHALL UTILIZE NORMAL WEIGHT AGGREGATE UNLESS NOTED OTHERWISE.
- MIXES SHALL BE DESIGNED TO PROVIDE CONCRETE WITH A 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.
- ALL EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED. AIR CONTENT SHALL BE 3%-6%. INTERIOR SLABS SHALL NOT HAVE AIR-ENTRAINMENT. GRADE BEAMS, RETAINING WALLS, AND FOOTINGS DO NOT REQUIRE AIR ENTRAINMENT.
- CONCRETE SLUMPS SHALL BE AS FOLLOWS:
 

RAMPS & SLOPING SURFACES	3" MAX.
CONCRETE CONTAINING SUPERPLASTICIZER	8" MAX.
ALL OTHER CONCRETE	4"
- THE TESTING LABORATORY SHALL BE NOTIFIED AFTER THE MILD STEEL REINFORCEMENT AND EMBEDS ARE POSITIONED PRIOR TO EACH CONCRETE PLACEMENT. NO CONCRETE SHALL BE PLACED UNTIL THESE ITEMS ARE CHECKED AND APPROVED BY THE TESTING LABORATORY.

## WOOD CONSTRUCTION

- PREPARATION, FABRICATION, AND INSTALLATION OF WOOD MEMBERS AND THEIR FASTENING SHALL CONFORM TO ACCEPTED ENGINEERING PRACTICES AND TO THE REQUIREMENTS OF THE BUILDING CODE. WORK SHALL BE PERFORMED BY EXPERIENCED FABRICATORS.
- WOOD FRAMING MATERIALS:
  - TYP. WALL SHEATHING SHALL BE 1/2" EXPOSURE 1 APA RATED SHEATHING, INDEX 32/16. TYP. ROOF DECK SHALL BE 3/4" APA RATED SHEATHING, INDEX 48/24. AT TYPICAL OVERHANGS, PROVIDE 5/8" APA RATED SHEATHING OVER 3/8" T1-11 GROOVE SIDE DOWN WITH GROOVES PERPENDICULAR TO RAFTERS.
  - SHEAR WALL SHEATHING SHALL BE EXPOSURE 1 APA RATED STRUCTURAL 1 SHEATHING OF THICKNESS SPECIFIED. REFER TO SHEAR WALL DETAIL 1/S-6.
  - PSL AND LVL BEAMS SPECIFIED ON THIS PROJECT SHALL BE MANUFACTURED BY TRUSS JOIST MACMILLAN. AN EQUIVALENT PRODUCT MAY BE SUBSTITUTED UPON APPROVAL BY ENGINEER. MIN. BASIC ALLOWABLE BENDING F<sub>b</sub>=2900 PSI AND MIN. ALLOW. SHEAR F<sub>v</sub>=290 PSI.
  - ALL WOOD FRAMING SHALL BE NO. 1 DOUG-FIR OR NO. 2 SOUTHERN PINE WITH A MINIMUM ALLOWABLE BENDING F<sub>b</sub> = 975 PSI AND A MIN. ALLOWABLE SHEAR F<sub>v</sub>=90 PSI.
  - ALL GLU-LAM BEAMS SHALL BE NO. 2 SOUTHERN PINE WITH A MINIMUM ALLOWABLE BENDING F<sub>b</sub>=2400 PSI, MIN. SHEAR F<sub>v</sub>=165 PSI & MIN. E=1,800 KSI.
- ROOF DECKING:
  - PANEL SHALL BE APPLIED WITH LONG DIMENSION PERPENDICULAR TO FRAMING. ADJACENT ROW OF PANELS SHALL BE STAGGERED.
  - PANELS SHALL BE FASTENED TO SUPPORTS USING 8d NAILS SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS AND AT 6" O.C. AT SUPPORTED PANEL EDGES.
  - 1/8" SPACE SHALL BE ALLOWED AT ALL PANEL ENDS AND EDGES
  - PANELS SHALL BE INSTALLED CONTINUOUSLY OVER TWO OR MORE SPANS.
- CONNECTIONS:
  - THE FRAMING CONTRACTOR SHALL FURNISH ALL HARDWARE REQUIRED TO MAKE CONNECTIONS BETWEEN VARIOUS ELEMENTS. UNLESS NOTED OTHERWISE, REFER TO NAILING SCHEDULE FOR SIZE OF NAILS.
  - SOLID BLOCKING SHALL BE PROVIDED BETWEEN ALL JOISTS AND TRUSSES AT EACH SUPPORT.
  - ALL CONNECTIONS NOT SPECIFICALLY DETAILED SHALL UTILIZE JOIST HANGERS, BEAM HANGERS, CONNECTORS, COLUMN BASES, ETC. THIS HARDWARE SHALL BE MANUFACTURED BY SIMPSON COMPANY AND SHALL BE SELECTED AND INSTALLED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS.
  - ALL BOLTED CONNECTIONS SHALL HAVE WASHERS AT BOLT HEAD AS WELL AS AT THE NUT.
- TRUSS FABRICATION:
  - TRUSS FABRICATOR SHALL VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
  - LOADS FROM ALL EQUIPMENT SHOWN ON PLAN SHALL BE INCLUDED. REFER TO TRUSS PROFILE ON SHEET S-6
  - TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS STAMPED BY A REGISTERED ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.
  - TRUSS MANUFACTURER SHALL PROVIDE ALL NECESSARY BRIDGING. TRUSSES SHALL BE DESIGNED FOR THE NET WIND UPLIFT LOAD NOTED ABOVE.
  - ALL TRUSSES SHALL BE DESIGNED WITH THE FOLLOWING DEFLECTION CRITERIA:
    - ALLOWABLE LL DEFLECTION = L/360
    - ALLOWABLE TL DEFLECTION = L/240
  - ALL TRUSSES SHALL BE FASTENED TO SUPPORTS WITH GALVANIZED METAL CONNECTORS AS INDICATED IN FRAMING DETAILS.
  - ALL TRUSSES SHALL BE DESIGNED AND FABRICATED USING #3 SOUTHERN PINE OR #1 DOUGLAS-FIR PER THE RECOMMENDATIONS OF THE TP1 DESIGN SPECIFICATIONS.

## STEEL

- STRUCTURAL STEEL SHALL BE NEW STEEL AND SHALL CONFORM TO THE A. I. S. C. "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS."
- STRUCTURAL STEEL ROLLED SHAPES AND PLATES NOTED "GRADE 50" SHALL CONFORM TO ASTM A-572, GRADE 50. ALL OTHER ROLLED SHAPES AND PLATES SHALL CONFORM TO ASTM A-36.
- STRUCTURAL STEEL PIPE SHALL CONFORM TO ASTM A-53, TYPES "E" OR "S", GRADE B OR ASTM 501.
- STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A-500, GRADE B.
- ALL STRUCTURAL STEEL CONNECTIONS AND DETAILS SHALL CONFORM TO THE A. I. S. C. "CODE OF STANDARD PRACTICE FOR STEEL, BUILDINGS AND BRIDGES."
- UNFINISHED THREADED FASTENERS SHALL CONFORM TO ASTM A-307, GRADE A BOLTS AND NUTS WITH HEXAGONAL HEADS. UNFINISHED THREADED FASTENERS SHALL BE USED ONLY FOR BOLTED CONNECTIONS OF SECONDARY FRAMING MEMBERS TO PRIMARY MEMBERS, FOR TEMPORARY BRACING TO FACILITATE ERECTION, AND FOR ANCHORAGE TO CONCRETE OR MASONRY CONSTRUCTION.
- BOLTED CONNECTIONS OF PRIMARY MEMBERS SHALL CONFORM TO THE REQUIREMENTS OF THE A. I. S. C. "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS. UNLESS NOTED OTHERWISE, ALL PRIMARY CONNECTIONS WITH HIGH STRENGTH BOLTS SHALL USE ASTM A-325 BOLTS AND HEAVY HEX NUTS.
- WELDED CONSTRUCTION SHALL CONFORM TO AWS D-1.1. "STRUCTURAL WELDING CODE - WELDING PROCESSES AND OPERATORS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATIONS PROCEDURE". ELECTRODES FOR FIELD AND SHOP WELDS SHALL BE E70XX, UNLESS NOTED OTHERWISE.
- SHEAR STUDS SHALL CONFORM TO ASTM A-108, HEADED ANCHORS.
- STEEL MEMBERS SHALL NOT BE SPLICED EXCEPT AS SHOWN ON THE DRAWINGS.
- ALL EXTERIOR STEEL EXPOSED TO WEATHER, INCLUDING LINTELS, SHALL BE GALVANIZED.

## REINFORCED MASONRY

- ALL REINFORCED MASONRY SHALL CONFORM TO THE PROVISIONS OF ACI 530.1/ASCE 5 (WITH EXCEPTIONS NOTED IN THE SPECIFICATIONS).
- CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90, GRADE N, TYPE 1, LIGHTWEIGHT UNITS WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI.
- MORTAR SHALL CONFORM TO ASTM C-270, TYPE M OR S. AGGREGATES FOR MORTAR SHALL CONFORM TO ASTM C-144. MORTAR SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS.
- GROUT SHALL CONFORM TO ASTM C-476. AGGREGATES FOR GROUT SHALL CONFORM TO ASTM C-404. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. CONSOLIDATE ALL GROUTED MASONRY WITH MECHANICAL VIBRATION.
- MASONRY CONSTRUCTION SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f<sub>m</sub>) OF 1500 PSI AT 28 DAYS.
- SPECIAL INSPECTION SHALL BE PROVIDED BY THE TESTING LAB AND REPORTED TO THE ENGINEER. THE TESTING LAB SHALL PERFORM PERIODIC INSPECTIONS DURING PREPARATION AND TAKING OF PRISMS OR TEST SPECIMENS, PLACING OF REINFORCING AND GROUT, AND LAYING UP OF MASONRY.
- BOND BEAMS SHALL BE CONSTRUCTED WITH PORTLAND CEMENT GROUT HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS, AND A MAXIMUM AGGREGATE SIZE OF 3/4".
- REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60.
- HORIZONTAL JOINT REINFORCEMENT SHALL BE FACTORY FABRICATED, TRUSS TYPE, 9 GAGE WIRE CONFORMING TO ASTM A-82. PROVIDE 1-#4 MINIMUM ABOVE AND BELOW ALL WALL OPENINGS AND EXTENDING 24" MINIMUM INTO JAMBS.
- ALL CELLS CONTAINING REINFORCEMENT, BOLTS, OR OTHER METAL ANCHORS SHALL BE GROUTED SOLID. ANY CELLS AT OR BELOW GRADE SHALL BE GROUTED SOLID, WHETHER REINFORCED OR NOT.

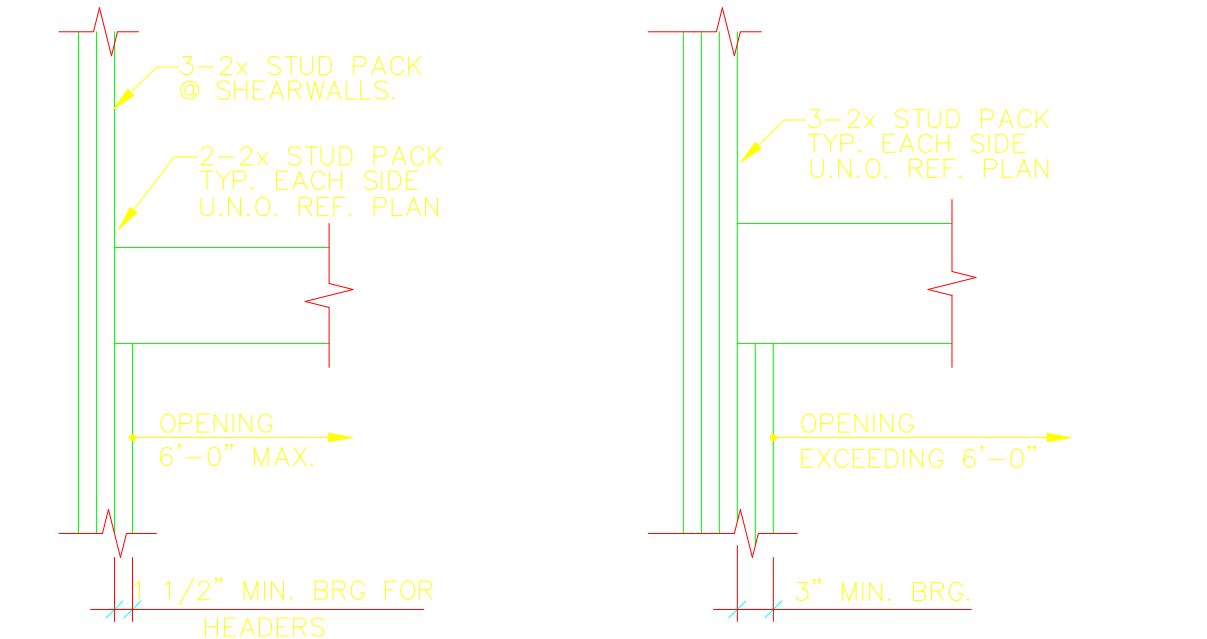
## NAILING SCHEDULE

CONNECTION	NAILING
(1) JOIST TO SILL OR GIRDER, TOENAIL	3-16d
(2) BRIDGING TO JOIST, TOENAIL EACH END	2-8d
(3) 1" x 6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	2-8d
(4) WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST, FACE NAIL	3-8d
(5) 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	2-16d
(6) SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	16d @ 16" O.C.
(7) TOP PLATE TO STUD, END NAIL	2-16d
(8) STUD TO SOLE PLATE, TOENAIL OR 2-16d O.C. ALONG EACH EDGE	4-8d
(9) DOUBLE STUDS, FACE NAIL	16d @ 24" O.C.
(10) DOUBLED TOP PLATES, FACE NAIL	16d @ 16" O.C.
(11) TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	2-16d
(12) CONTINUOUS HEADER, TWO PIECES	16d @ 16" O.C. ALONG EACH EDGE
(13) CEILING JOISTS TO PLATE, TOENAIL	3-8d
(14) CONTINUOUS HEADER TO STUD, TOENAIL	4-8d
(15) CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	3-16d
(16) CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	3-16d
(17) RAFTER TO HIP	3-10d TOENAIL OR 2-16d FACE NAIL
(18) 1" BRACE TO EACH STUD AND PLATE, FACE NAIL	2-8d
(19) 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL	2-8d
(20) WIDER THAN 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL	3-8d
(21) BUILT-UP CORNER STUDS, O.C.	16d @ 24" O.C.
(22) BUILT-UP GIRDER AND BEAMS, 2-#4 @ 32" O.C. @ TOP & BOT. & STAGGERED 2-20d @ ENDS & @ EACH SPLICE	2-20d @ 32" O.C. @ TOP & BOT. & STAGGERED 2-20d @ ENDS & @ EACH SPLICE

## MAXIMUM CLEAR SPAN FOR HEADERS

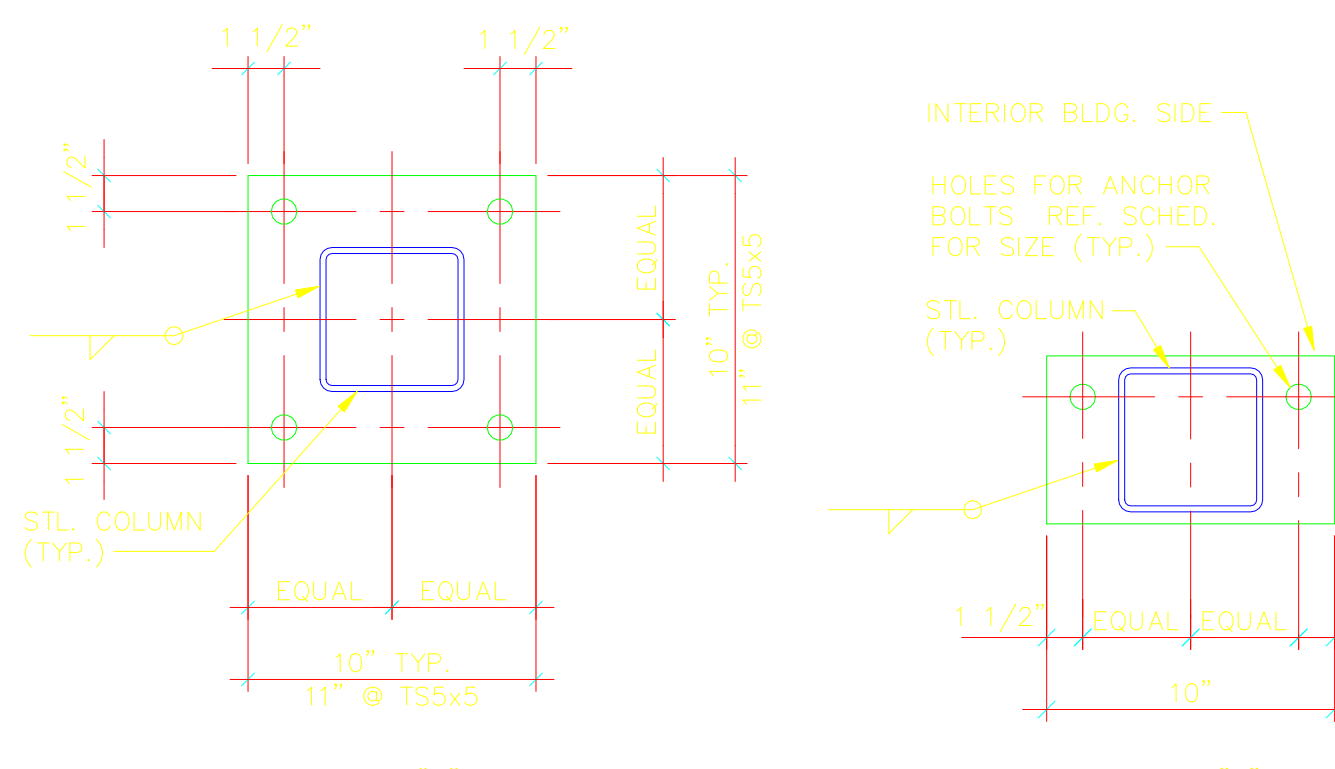
HEADER SIZE MATERIAL AT EDGE	SUPPORTING ONLY CEILING AND ROOF
3-2x6	4'-0"
3-2x8	6'-0"
3-2x10	8'-0"
3-2x12	10'-0"

- HEADER SIZES MAY BE USED IF NO SIZES ARE CALLED OUT ON THE PLANS OR DETAILS.
- FOR WALL OPENING SIZES REFERENCE ARCHITECTURAL DRAWINGS.
- USE 1/2" FLITCH BETWEEN 3-2x HEADERS.



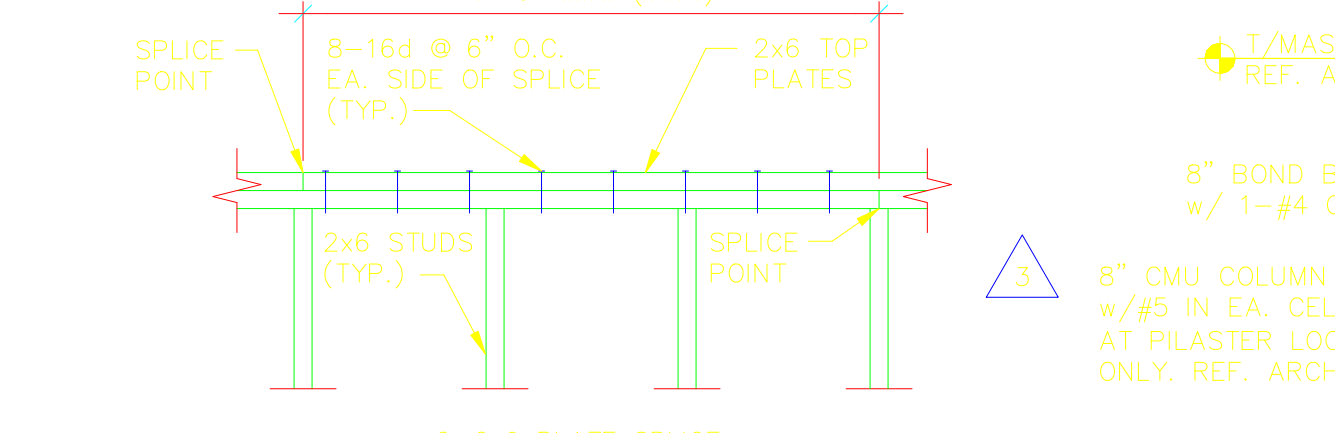
## 1 HEADER BEARINGS

NO SCALE



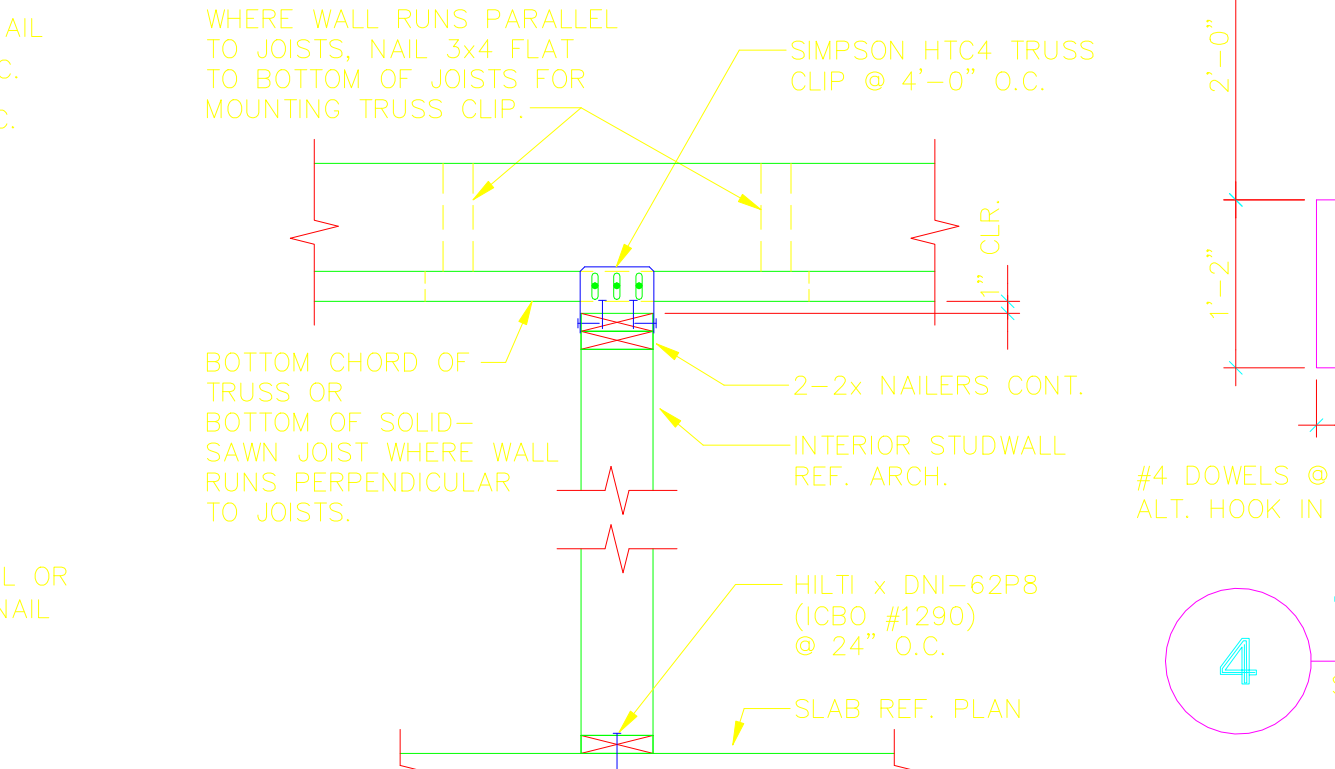
## 2 TYPICAL BASE PLATE

NO SCALE



## 3 TYPICAL SECTION

SCALE: 3/4"=1'-0"



## 5 TYP. DETAIL

SCALE: 3/4"=1'-0"

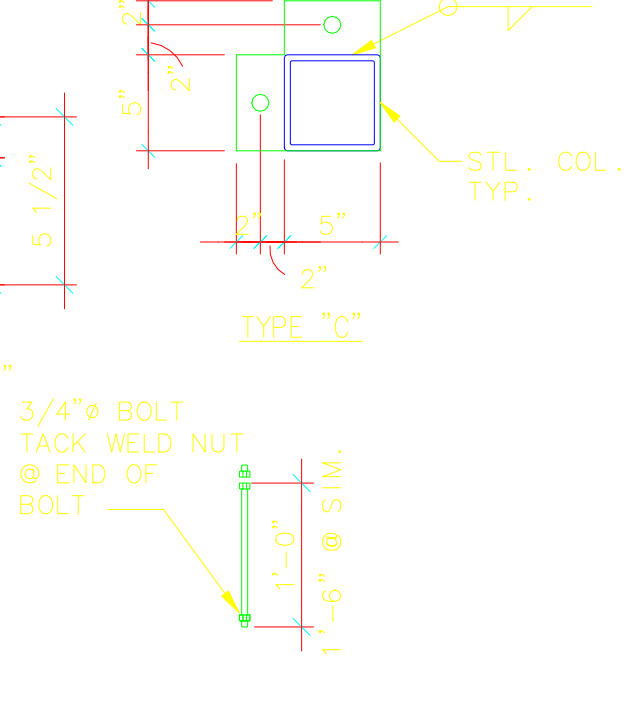
## LAP SPLICE SCHED.

BAR SIZE	LAP	BAR SIZE	LAP
3	1'-10"	8	4'-9"
4	2'-3"	9	6'-1"
5	2'-8"	10	7'-3"
6	3'-2"		
7	3'-9"		

## HANGER SCHEDULE

JOISTS	HANGER
2x8	U26
2x10	U28
2x12	U210
2-2x12	U210-2
3-2x12	U210-3
5 1/4"x11 1/4" PSL	HCLTV5.511
5 1/4"x18" PSL	CLTV5.518

- PROVIDE THESE HANGERS U.N.O. ON THE PLANS OR SECTIONS.
- ALL HANGERS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE.
- HANGERS FOR WOOD I-JOISTS SHALL BE SPECIFIED BY I-JOIST MANUFACTURER (U.N.O.).
- SLOPE AND SKEW HANGERS AS REQ'D. REF. PLANS AND ARCH. ELEVATIONS.



## 4 TYP. SECTION

SCALE: 3/4"=1'-0"

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$\Delta$ 12 NOV. 2003 CITY COMMENTS
$\Delta$ 18 DEC. 2003 CITY COMMENTS
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# GENERAL NOTES

## GENERAL

- THE STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE UNIFORM BUILDING CODE, 1994 EDITION, AND ALL APPLICABLE REGULATIONS OF THE CITY OF CASA GRANDE, ARIZONA.
- THE LOADS THAT HAVE BEEN USED IN THE STRUCTURAL DESIGN INCLUDE THE FOLLOWING:  
 DEAD LOADS AT ROOF:  
 ROOF JOISTS 4 PSF  
 ROOF DECK AND INSULATION 4 PSF  
 ROOFING 5 PSF TYP. (10 PSF @ CLAY TILE ROOF)  
 CEILING 2 PSF  
 MEP (INCLUDING SPRINKLERS) 6 PSF  
 TOTAL = 20 PSF TYP. (25 PSF @ CLAY TILE ROOF)  
 LIVE LOAD AT ROOF 20 PSF  
 WIND LOADS:  
 BASIC WIND SPEED 75 MPH  
 EXPOSURE C  
 NET UPLIFT FIELD = 10 PSF PERIMETER = 10 PSF  
 SEISMIC LOADS:  
 ZONE 1  
 SOIL TYPE SD  
 ALL LIVE LOADS HAVE BEEN REDUCED BASED ON TRIBUTARY AREAS IN ACCORDANCE WITH CODE PROVISIONS UNLESS OTHERWISE NOTED. TO BOTH ALTERNATE AND ADJACENT SPACES WHERE APPROPRIATE TO DERIVE GOVERNING CONDITIONS.  
 ALLOWABLE STRESSES HAVE BEEN INCREASED BY 1/3 FOR LOADING COMBINATIONS INCLUDING WIND AND SEISMIC WHERE ALLOWED BY CODE.  
 DEAD LOADS HAVE BEEN CALCULATED TO INCLUDE THE ACTUAL WEIGHT OF ALL WORK SHOWN ON THE STRUCTURAL, MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS. NO OTHER EQUIPMENT SHALL BE PLACED ON OR HUNG FROM THE ROOF SYSTEM WITHOUT PRIOR WRITTEN APPROVAL OF THE ENGINEER. ROOF-MOUNTED HVAC UNITS SHALL BE PLACED WITHIN THE DESIGNATED AREAS SHOWN ON THE FRAMING PLANS.  
 COMPLETE SHOP DRAWINGS FOR THE STRUCTURAL WORK SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO COMMENCEMENT OF CONSTRUCTION. IN ACCORDANCE WITH THE SPECIFICATIONS, SUCH REVIEW BY THE ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR CORRECT FABRICATION AND CONSTRUCTION OF THE WORK. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR MATERIALS PURCHASED PRIOR TO REVIEW OF SHOP DRAWINGS.  
 ANY DEVIATION FROM, ADDITION TO, SUBSTITUTION FOR, OR MODIFICATION TO THE STRUCTURE OR ANY PART OF THE STRUCTURE DETAILED ON THESE DRAWINGS SHALL BE SUBMITTED IN WRITING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS THAT ARE SUBMITTED FOR REVIEW DO NOT CONSTITUTE "WRITING" UNLESS IT IS CLEARLY NOTED THAT SPECIFIC CHANGES ARE BEING SUGGESTED.  
 THE CONTRACTOR SHALL REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS AND FOR EXACT LOCATIONS OF ALL ARCHITECTURAL DETAILS. THE CONTRACTOR SHALL COMPARE THE STRUCTURAL SECTIONS WITH THE ARCHITECTURAL SECTIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO COMPLETION OF THE SHOP DRAWINGS.  
 THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH THE WORK.  
 THE PRINCIPAL OPENINGS ARE SHOWN ON THE STRUCTURAL DRAWINGS. CONTRACTOR SHALL REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, CURBS, INSERTS AND OTHER OPENINGS NOT SHOWN. THE CONTRACTOR SHALL PROVIDE FOR ALL OPENINGS, WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT, SIZE AND LOCATION OF ALL OPENINGS SHALL BE VERIFIED BY THE CONTRACTOR. ANY DEVIATION FROM OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION FOR APPROVAL PRIOR TO CONSTRUCTION.  
 THE STRUCTURAL DRAWINGS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.  
 THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN AND OTHER PERSONS DURING CONSTRUCTION.  
 THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL WORK AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY CONDITION WHICH IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.  
 CONSTRUCTION MATERIALS SHALL NOT BE STORED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED ON THE DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENFORCE THIS REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS.  
 FIELD INSPECTION REPORTS SHALL BE SUBMITTED TO THE ENGINEER AS OUTLINED IN THE SPECIFICATIONS. INSPECTIONS SHALL BE PROVIDED BY A QUALIFIED AGENCY HIRED BY THE OWNER.

## EARTHWORK AND FOUNDATIONS

- THE FOUNDATION DESIGN IS BASED ON A SUBSURFACE EXPLORATION AND REPORT BY FORRE & VANN, INC. (PROJECT NO. 11637) DATED JULY 3, 2001. A GEOTECHNICAL ENGINEER SHALL VERIFY THAT SOILS OF THE DESIGN BEARING CAPACITY HAVE BEEN ENCOUNTERED AND THAT THE BUILDING PAD IS SUITABLE FOR CONSTRUCTION.
- THE FOUNDATION DESIGN IS BASED ON POTENTIAL SLAB DIFFERENTIAL MOVEMENTS OF ONE (1) INCH. IF THIS VALUE IS NOT ACCEPTABLE TO THE OWNER OR TENANTS, THE FOUNDATION DESIGN MUST BE REVISID.
- CONTINUOUS AND ISOLATED FOOTINGS ARE DESIGNED FOR AN ALLOWABLE NET BEARING PRESSURE OF 450 PSF FOR TOTAL LOAD. THESE VALUES ARE APPLICABLE FOR FOOTINGS BEARING ON NATIVE UNDISTURBED SOIL. EXTERIOR FOOTINGS MUST BEAR A MINIMUM OF 30 INCHES BELOW FINISH GRADE.
- ALL SURFACE SOIL, VEGETATION, AND DEBRIS SHALL BE STRIPPED AND REMOVED FOR THE AREA OF THE BUILDING PAD AND AT LEAST THREE (3) FEET BEYOND THE BUILDING PERIMETER.
- AFTER STRIPPING IS COMPLETE, THE EXPOSED SUBGRADE SHALL BE SCARIFIED TO A DEPTH OF TWELVE (12) INCHES, IMMEDIATELY CONDITIONED AND RECOMPACTED PER THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT.
- ANY UNSTABLE SOILS DETECTED SHALL BE REMOVED AND REPLACED WITH STRUCTURAL FILL.
- PLACE STRUCTURAL FILL TO PROPER ELEVATIONS. STRUCTURAL FILL SHALL CONSIST OF SOILS AS RECOMMENDED IN THE GEOTECHNICAL REPORT. PLACE A MINIMUM 12 INCHES OF EXPANSIVE FILL DIRECTLY BELOW THE 4 INCHES AGGREGATE BASE COURSE.

- A QUALIFIED SOIL TECHNICIAN SHALL PERFORM SUFFICIENT IN-PLACE DENSITY TESTS DURING FILL OPERATIONS TO VERIFY THAT PROPER LEVELS OF COMPACTION ARE ATTAINED AND THAT FOOTINGS ARE BEARING ON THE PROPER MATERIAL.
- CONCRETE FOR SLABS ON FILL SHALL BE PLACED OVER A CONTINUOUS 10 MIL POLYETHYLENE MOISTURE BARRIER OVER A 4 INCH LAYER OF AGGREGATE BASE COURSE CONSISTING OF FILL AS DESCRIBED IN THE GEOTECHNICAL REPORT. PLACE A MINIMUM OF 12 INCHES OF SELECT STRUCTURAL FILL DIRECTLY BELOW THE BASE COURSE.
- THE ABOVE REQUIREMENTS ARE A SUMMARY OF THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. CONTRACTOR SHALL NOT BE ABSOLVED FROM THE RESPONSIBILITY OF REVIEWING AND COMPLYING WITH THE ENTIRE GEOTECHNICAL REPORT.

## CAST IN PLACE REINFORCED CONCRETE

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE MOST RECENT EDITION OF ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE."
- STEEL REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 60.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185, GRADE 65. ALL WELDED WIRE FABRIC SHALL BE SUPPLIED IN FLAT SHEETS, NOT IN ROLLS.
- ALL CONCRETE SHALL UTILIZE NORMAL WEIGHT AGGREGATE UNLESS NOTED OTHERWISE
- MIXES SHALL BE DESIGNED TO PROVIDE CONCRETE WITH A 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.
- ALL EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED. AIR CONTENT SHALL BE 3%-6%. INTERIOR SLABS SHALL NOT HAVE AIR-ENTRAINMENT GRADE BEAMS, RETAINING WALLS, AND FOOTINGS DO NOT REQUIRE AIR ENTRAINMENT.
- CONCRETE SLUMPS SHALL BE AS FOLLOWS:  
 RAMPS & SLOPING SURFACES 3" MAX.  
 CONCRETE CONTAINING SUPERPLASTICIZER 8" MAX.  
 ALL OTHER CONCRETE 4"
- THE TESTING LABORATORY SHALL BE NOTIFIED AFTER THE WILD STEEL REINFORCING AND EMBEDS ARE POSITIONED PRIOR TO EACH CONCRETE PLACEMENT. NO CONCRETE SHALL BE PLACED UNTIL THESE ITEMS ARE CHECKED AND APPROVED BY THE TESTING LABORATORY.

## WOOD CONSTRUCTION:

- PREPARATION, FABRICATION, AND INSTALLATION OF WOOD MEMBERS AND THEIR FASTENERS SHALL CONFORM TO ACCEPTED ENGINEERING PRACTICES AND TO THE REQUIREMENTS OF THE BUILDING CODE.  
 WORK SHALL BE PERFORMED BY EXPERIENCED FABRICATORS.
- WOOD FRAMING MATERIALS:  
 A. TYP. WALL SHEATHING SHALL BE 1/2" EXPOSURE 1 APA-RATED SHEATHING, INDEX 32/16. TYP. ROOF DECK SHALL BE 3/4" APA RATED SHEATHING, INDEX 48/24. AT TYPICAL OVERHANGS, PROVIDE 3/8" APA RATED SHEATHING OVER 3/8" T1-11 GROOVE SIDE DOWN WITH GROOVES PERPENDICULAR TO RAFTERS.  
 B. SHEAR WALL SHEATHING SHALL BE EXPOSURE 1 APA RATED STRUCTURAL 1 SHEATHING OF THICKNESS SPECIFIED. REFER TO SHEAR WALL DETAIL 1/S-6.  
 C. PSL AND LVL BEAMS SPECIFIED ON THIS PROJECT SHALL BE MANUFACTURED BY TRUSS JOIST MACMILLAN, AN EQUIVALENT PRODUCT MAY BE SUBSTITUTED UPON APPROVAL BY ENGINEER. MIN. BASIC ALLOWABLE BENDING Fb=2900 PSI & MIN. ALLOW. SHEAR Fv=290 PSI.
- WOOD FRAMING SHALL BE NO. 1 DOUG-FIR OR NO. 2 SOUTHERN PINE WITH A MINIMUM ALLOWABLE BENDING Fb = 975 PSI AND A MIN. ALLOWABLE SHEAR Fv=90 PSI.
- ALL GLU-LAM BEAMS SHALL BE NO. 2 SOUTHERN PINE WITH A MINIMUM ALLOWABLE BENDING Fb=2400 PSI, MIN. SHEAR Fv=165 PSI & MIN. E=1,800 KSI.
- ROOF DECKING:  
 A. PANEL SHALL BE APPLIED WITH LONG DIMENSION PERPENDICULAR TO FRAMING; ADJACENT ROW OF PANELS SHALL BE STAGGERED.  
 B. PANELS SHALL BE FASTENED TO SUPPORTS USING 8d NAILS SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS AND AT 6" O.C. AT SUPPORTED PANEL EDGES.  
 C. 1/8" SPACE SHALL BE ALLOWED AT ALL PANEL ENDS AND EDGES  
 D. PANELS SHALL BE INSTALLED CONTINUOUSLY OVER TWO OR MORE SPANS.
- CONNECTIONS:  
 A. THE FRAMING CONTRACTOR SHALL FURNISH ALL HARDWARE REQUIRED TO MAKE CONNECTIONS BETWEEN VARIOUS ELEMENTS. UNLESS NOTED OTHERWISE, REFER TO NAILING SCHEDULE FOR SIZE OF NAILS.  
 B. SOLID BLOCKING SHALL BE PROVIDED BETWEEN ALL JOISTS AND TRUSSES AT EACH SUPPORT.  
 C. ALL CONNECTIONS NOT SPECIFICALLY DETAILED SHALL UTILIZE JOIST HANGERS, BEAM HANGERS, CONNECTORS, COLUMN BASES, ETC. THIS HARDWARE SHALL BE MANUFACTURED BY SIMPSON COMPANY AND SHALL BE SELECTED AND INSTALLED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS.  
 D. ALL BOLTED CONNECTIONS SHALL HAVE WASHERS AT BOLT HEAD AS WELL AS AT THE NUT.
- TRUSS FABRICATION:  
 A. TRUSS FABRICATOR SHALL VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.  
 B. LOADS FROM ALL EQUIPMENT SHOWN ON PLAN SHALL BE INCLUDED. REFER TO TRUSS PROFILE ON SHEET S-6  
 C. TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS TO BE REVIEWED BY A REGISTERED ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.  
 D. TRUSS MANUFACTURER SHALL PROVIDE ALL NECESSARY BRIDGING. TRUSSES SHALL BE DESIGNED FOR THE NET WIND UPLIFT LOAD NOTED ABOVE.  
 E. ALL TRUSSES SHALL BE DESIGNED WITH THE FOLLOWING DEFLECTION CRITERIA:  
 - ALLOWABLE LL DEFLECTION - L/360  
 - ALLOWABLE TL DEFLECTION - L/240  
 F. ALL TRUSSES SHALL BE FASTENED TO SUPPORTS WITH GALVANIZED METAL CONNECTORS AS INDICATED IN FRAMING DETAILS.
- ALL TRUSSES SHALL BE DESIGNED AND FABRICATED USING #3 SOUTHERN PINE OR #1 DOUGLAS-FIR PER THE RECOMMENDATIONS OF THE TP1 DESIGN SPECIFICATIONS.

## STEEL

- STRUCTURAL STEEL SHALL BE NEW STEEL AND SHALL CONFORM TO THE A.I.S.C. "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS."
- STRUCTURAL STEEL ROLLED SHAPES AND PLATES NOTED "GRADE 50" SHALL CONFORM TO ASTM A-572, GRADE 50. ALL OTHER ROLLED SHAPES AND PLATES SHALL CONFORM TO ASTM A-36.
- STRUCTURAL STEEL PIPE SHALL CONFORM TO ASTM A-53, TYPES "E" OR "S", GRADE B OR ASTM 501.
- STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A-500, GRADE B.
- ALL STRUCTURAL STEEL CONNECTIONS AND DETAILS SHALL CONFORM TO THE A.I.S.C. "CODE OF STANDARD PRACTICE FOR STEEL, BUILDINGS AND BRIDGES."
- UNFINISHED THREADED FASTENERS SHALL CONFORM TO ASTM A-307, GRADE A BOLTS AND NUTS WITH HEXAGONAL HEADS. UNFINISHED THREADED FASTENERS SHALL BE USED ONLY FOR BOLTED CONNECTIONS OF SECONDARY FRAMING MEMBERS TO PRIMARY MEMBERS, FOR TEMPORARY BRACING TO FACILITATE ERECTION, AND FOR ANCHORAGE TO CONCRETE OR MASONRY CONSTRUCTION.
- BOLTED CONNECTIONS OF PRIMARY MEMBERS SHALL CONFORM TO THE REQUIREMENTS OF THE A.I.S.C. "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS." UNLESS NOTED OTHERWISE, ALL PRIMARY CONNECTIONS WITH HIGH STRENGTH BOLTS SHALL USE ASTM A-325 BOLTS AND HEAVY HEX NUTS.
- WELDED CONSTRUCTION SHALL CONFORM TO AWS D-1.1, "STRUCTURAL WELDING CODE." WELDING PROCESSES AND OPERATORS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURES." ELECTRODES FOR FIELD AND SHOP WELDS SHALL BE E70XX, UNLESS NOTED OTHERWISE.
- SHEAR STUDS SHALL CONFORM TO ASTM A-108, HEADED ANCHORS.
- STEEL MEMBERS SHALL NOT BE SPLICED EXCEPT AS SHOWN ON THE DRAWINGS.
- ALL EXTERIOR STEEL EXPOSED TO WEATHER, INCLUDING LINTELS, SHALL BE GALVANIZED.

## REINFORCED MASONRY

- ALL REINFORCED MASONRY SHALL CONFORM TO THE PROVISIONS OF ACI 530.1/ASCE 6 (WITH EXCEPTIONS NOTED IN THE SPECIFICATIONS).
- CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90, GRADE N, TYPE 1, LIGHTWEIGHT UNITS WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI.
- MORTAR SHALL CONFORM TO ASTM C-270, TYPE M OR S. AGGREGATES FOR MORTAR SHALL CONFORM TO ASTM C-144. MORTAR SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS.
- GROUT SHALL CONFORM TO ASTM C-476. AGGREGATES FOR GROUT SHALL CONFORM TO ASTM C-404. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. CONSOLIDATE ALL GROUT MASONRY WITH MECHANICAL VIBRATION.
- MASONRY CONSTRUCTION SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (1"m) OF 1500 PSI AT 28 DAYS.
- SPECIAL INSPECTION SHALL BE PROVIDED BY THE TESTING LAB AND REPORTED TO THE ENGINEER. THE TESTING LAB SHALL PERFORM PERIODIC INSPECTIONS DURING PREPARATION AND TAKING OF PRISMS OR TEST SPECIMENS, PLACING OF REINFORCING AND GROUT, AND LAYING UP OF MASONRY.
- BOND BEAMS SHALL BE CONSTRUCTED WITH PORTLAND CEMENT GROUT HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS, AND A MAXIMUM AGGREGATE SIZE OF 3/4".
- REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60.
- HORIZONTAL JOINT REINFORCEMENT SHALL BE FACTORY FABRICATED, TRUSS TYPE, 9 GAGE WIRE CONFORMING TO ASTM A-82. PROVIDE 1"-#4 MINIMUM ABOVE AND BELOW ALL WALL OPENINGS AND EXTENDING 24" MINIMUM INTO JOIST JOISTS.
- ALL CELLS CONTAINING REINFORCEMENT, BOLTS, OR OTHER METAL ANCHORS SHALL BE CONCRETE SOLID OR GROUTED CELLS AT OR BELOW GRADE SHALL BE GROUTED SOLID, WHETHER REINFORCED OR NOT.

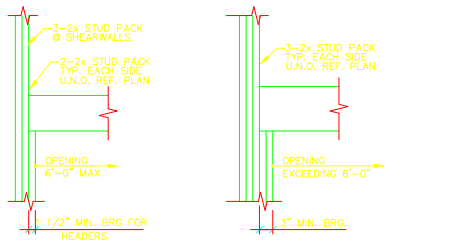
## NAILING SCHEDULE

- | CONNECTION   | NAILING  |
|--|--|
| (1) JOIST TO SILL OR GIRDER, TOENAIL                         | 3-16d  |
| (2) BRIDGING TO JOIST, TOENAIL EACH END.                     | 2-8d   |
| (3) 1" x 6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL        | 2-8d   |
| (4) WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST, FACE NAIL     | 3-8d   |
| (5) 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND END NAIL       | 2-16d  |
| (6) SOLID PLATE TO JOIST OR BLOCKING, FACE NAIL              | 16d @ 16" O.C.   |
| (7) TOP PLATE TO STUD, END NAIL                              | 2-16d  |
| (8) STUD TO SOLE PLATE,                                      | 4-8d   |
|  | TOENAIL OR   |
|  | 2-16d END NAIL   |
| (9) DOUBLED STUDS, FACE NAIL                                 | 16d @ 24" O.C.   |
| (10) DOUBLED TOP PLATES, FACE NAIL                           | 16d @ 16" O.C.   |
| (11) TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL           | 2-16d  |
| (12) CONTINUOUS HEADER, TWO PIECES.                          | 16d @ 16" O.C. ALONG EACH EDGE   |
| (13) CEILING JOISTS TO PLATE, TOENAIL                        | 3-8d   |
| (14) CONTINUOUS HEADER TO STUD, TOENAIL.                     | 4-8d   |
| (15) CEILING JOISTS, LAPS OR PARTITIONS, FACE NAIL           | 3-16d  |
| (16) CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL           | 3-16d  |
| (17) RAFTER TO HIP   | 3-10d TOENAIL OR   |
|  | 2-16d FACE NAIL  |
| (18) 1" BRACE TO EACH STUD AND PLATE, FACE NAIL.             | 2-8d   |
| (19) 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL            | 2-8d   |
| (20) WIDER THAN 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL | 2-8d   |
| (21) BUILT-UP CORNER STUDS.                                  | 3-8d   |
| (22) BUILT-UP GIRDER AND BEAMS.                              | 16d @ 24" O.C. O.C. 20d @ 32" O.C. @ TOP & BOT. & STAGGERED 2-20d ENDS @ EACH SPLICE |

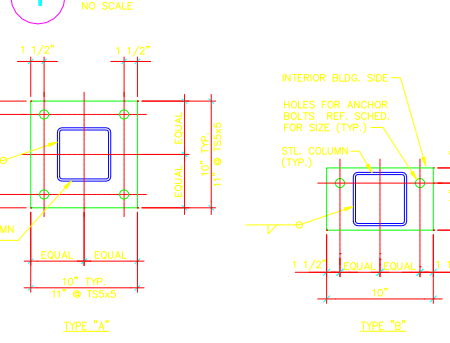
### MAXIMUM CLEAR SPAN FOR HEADERS

HEADER SIZE MATERIAL AT EDGE	SUPPORTING ONLY CEILING AND ROOF
3-2x6	4'-0"
3-2x8	6'-0"
3-2x10	8'-0"
3-2x12	10'-0"

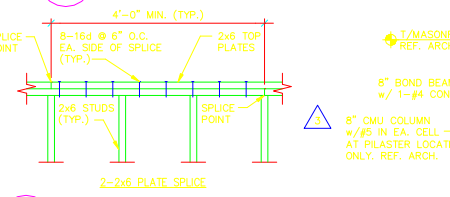
- HEADER SIZES MAY BE USED IF NO SIZES ARE CALLED OUT ON THE PLANS OR DETAILS.
- FOR WALL OPENING SIZES REFERENCE ARCHITECTURAL DRAWINGS.
- USE 1/2" FLITCH BETWEEN 3-2x HEADERS.



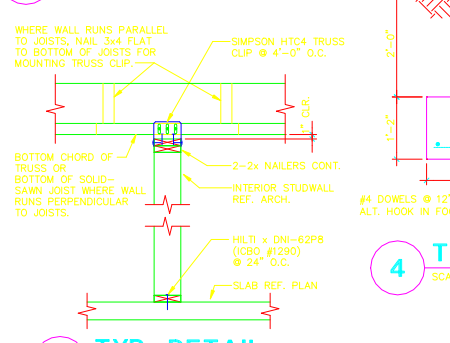
## 1. HEADER BEARINGS



## 2. TYPICAL BASE PLATE



## 3. TYPICAL SECTION



## 5. TYP. DETAIL



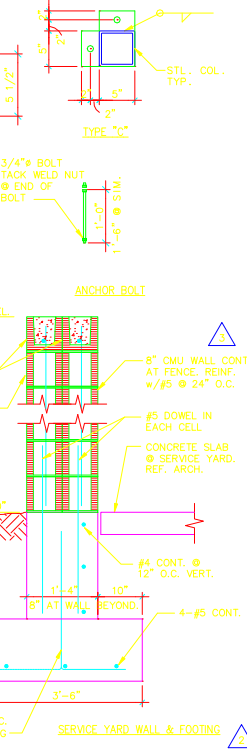
### LAP SPlice SCHED.

BAR SIZE	LAP	BAR SIZE	LAP
3	1'-10"	8	4'-8"
4	2'-3"	9	6'-1"
5	2'-8"	10, 0	7'-3"
6	3'-2"		
7	3'-9"		

### HANGER SCHEDULE

JOISTS	HANGER
2x8	U26
2x10	U28
2-2x12	U210-2
3-2x12	U210-3
5 1/4"x11 1/4" PSL	HGLTV5.511
5 1/4"x18" PSL	GLTV5.518

- PROVIDE THESE HANGERS U.N.O. ON THE PLANS OR SECTIONS.
- ALL HANGERS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE.
- HANGERS FOR WOOD I-JOISTS SHALL BE SPECIFIED BY I-JOIST MANUFACTURER (U.N.O.).
- SLOPE AND SKEW HANGERS AS REQ'D. REF. PLANS AND ARCH. ELEVATIONS.



## 4. TYP. SECTION

SCALE: 3/4"=1'-0"

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- THE STRUCTURAL DRAWINGS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.
- THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN AND OTHER PERSONS DURING CONSTRUCTION.
- THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL WORK AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.
- CONSTRUCTION MATERIALS SHALL NOT BE STORED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED ON THE DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENFORCE THIS REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS.
- FIELD INSPECTION REPORTS SHALL BE SUBMITTED TO THE ENGINEER AS OUTLINED IN THE SPECIFICATIONS. INSPECTIONS SHALL BE PROVIDED BY A QUALIFIED AGENCY HIRED BY THE OWNER.

## EARTHWORK AND FOUNDATIONS

- THE FOUNDATION DESIGN IS BASED ON A SUBSURFACE EXPLORATION AND REPORT BY FOREE & VANN, INC. (PROJECT NO. 11637) DATED JULY 3, 2001. A GEOTECHNICAL ENGINEER SHALL VERIFY THAT SOILS OF THE DESIGN BEARING CAPACITY HAVE BEEN ENCOUNTERED AND THAT THE BUILDING PAD IS SUITABLE FOR CONSTRUCTION.
- THE FOUNDATION DESIGN IS BASED ON POTENTIAL SLAB DIFFERENTIAL MOVEMENTS OF ONE (1) INCH. IF THIS VALUE IS NOT ACCEPTABLE TO THE OWNER OR TENANTS, THE FOUNDATION DESIGN MUST BE REVISED.
- CONTINUOUS AND ISOLATED FOOTINGS ARE DESIGNED FOR AN ALLOWABLE NET BEARING PRESSURE OF 1250 PSF FOR TOTAL LOAD. THESE VALUES ARE APPLICABLE FOR FOOTINGS BEARING ON NATIVE UNDISTURBED SOIL. EXTERIOR FOOTINGS MUST BEAR A MINIMUM OF 30 INCHES BELOW FINISH GRADE.
- ALL SURFACE SOIL, VEGETATION, AND DEBRIS SHALL BE STRIPPED AND REMOVED FOR THE AREA OF THE BUILDING PAD AND AT LEAST THREE (3) FEET BEYOND THE BUILDING PERIMETER.
- AFTER STRIPPING IS COMPLETE, THE EXPOSED SUBGRADE SHALL BE SCARIFIED TO A DEPTH OF TWELVE (12) INCHES, MOISTURE CONDITIONED AND RECOMPACTED PER THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT.
- ANY UNSTABLE SOILS DETECTED SHALL BE REMOVED AND REPLACED WITH STRUCTURAL FILL.
- PLACE STRUCTURAL FILL TO PROPER ELEVATIONS. STRUCTURAL FILL SHALL CONSIST OF SOILS AS RECOMMENDED IN THE GEOTECHNICAL REPORT. PLACE A MINIMUM 12 INCHES OF NON-EXPANSIVE FILL DIRECTLY BELOW THE 4 INCHES AGGREGATE BASE.

- A QUALIFIED SOIL TECHNICIAN SHALL PERFORM SUFFICIENT IN-PLACE DENSITY TESTS DURING FILL OPERATIONS TO VERIFY THAT PROPER LEVELS OF COMPACTION ARE ATTAINED AND THAT FOOTINGS ARE BEARING ON THE PROPER MATERIAL.
- CONCRETE FOR SLABS ON FILL SHALL BE PLACED OVER A CONTINUOUS 10 MIL POLYETHYLENE MOISTURE BARRIER OVER A 4 INCH LAYER OF AGGREGATE BASE COURSE CONSISTING OF FILL AS DESCRIBED IN THE GEOTECHNICAL REPORT. PLACE A MINIMUM OF 12 INCHES OF SELECT STRUCTURAL FILL DIRECTLY BELOW THE BASE COURSE.
- THE ABOVE REQUIREMENTS ARE A SUMMARY OF THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. CONTRACTOR SHALL NOT BE ABSOLVED FROM THE RESPONSIBILITY OF REVIEWING AND COMPLYING WITH THE ENTIRE GEOTECHNICAL REPORT.

## CAST IN PLACE REINFORCED CONCRETE

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE MOST RECENT EDITION OF ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE".
- STEEL REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 60.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185, GRADE 65. ALL WELDED WIRE FABRIC SHALL BE SUPPLIED IN FLAT SHEETS, NOT IN ROLLS.
- ALL CONCRETE SHALL UTILIZE NORMAL WEIGHT AGGREGATE UNLESS NOTED OTHERWISE.
- MIXES SHALL BE DESIGNED TO PROVIDE CONCRETE WITH A 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.
- ALL EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED. AIR CONTENT SHALL BE 3%-6%. INTERIOR SLABS SHALL NOT HAVE AIR-ENTRAINMENT. GRADE BEAMS, RETAINING WALLS, AND FOOTINGS DO NOT REQUIRE AIR ENTRAINMENT.
- CONCRETE SLUMPS SHALL BE AS FOLLOWS:  
RAMPS & SLOPING SURFACES 3" MAX.  
CONCRETE CONTAINING SUPERPLASTICIZER 8" MAX.  
ALL OTHER CONCRETE 4"
- THE TESTING LABORATORY SHALL BE NOTIFIED AFTER THE MILD STEEL REINFORCEMENT AND EMBEDS ARE POSITIONED PRIOR TO EACH CONCRETE PLACEMENT. NO CONCRETE SHALL BE PLACED UNTIL THESE ITEMS ARE CHECKED AND APPROVED BY THE TESTING LABORATORY.

## WOOD CONSTRUCTION:

- PREPARATION, FABRICATION, AND INSTALLATION OF WOOD MEMBERS AND THEIR FASTENING SHALL CONFORM TO ACCEPTED ENGINEERING PRACTICES AND TO THE REQUIREMENTS OF THE BUILDING CODE.  
WOOD FRAMING MATERIALS:  
A. TYP. WALL SHEATHING SHALL BE 15/32" EXPOSURE 1 APA RATED SHEATHING, INDEX 32/16. TYP. ROOF DECK SHALL BE 3/4" APA RATED SHEATHING, INDEX 48/24. AT TYPICAL OVERHANGS, PROVIDE 3/8" APA RATED SHEATHING OVER 3/8" T1-11 GROOVE SIDE DOWN WITH GROOVES PERPENDICULAR TO RAFTERS.  
REF. 1/S-5  
B. SHEAR WALL SHEATHING SHALL BE EXPOSURE 1 APA RATED STRUCTURAL 1 SHEATHING OF THICKNESS SPECIFIED. REFER TO SHEAR WALL DETAIL 1/S-6.  
C. PSL AND LVL BEAMS SPECIFIED ON THIS PROJECT SHALL BE MANUFACTURED BY TRUSS JOIST MACMILLAN. AN EQUIVALENT PRODUCT MAY BE SUBSTITUTED UPON APPROVAL BY ENGINEER. MIN. BASIC ALLOWABLE BENDING Fb=2900 PSI AND MIN. ALLOW. SHEAR Fv=290 PSI.  
D. ALL WOOD FRAMING SHALL BE NO. 1 DOUG-FIR OR NO. 2 SOUTHERN PINE WITH A MINIMUM ALLOWABLE BENDING Fb = 975 PSI AND A MIN. ALLOWABLE SHEAR Fv=90 PSI.  
E. ALL GLU-LAM BEAMS SHALL BE NO. 2 SOUTHERN PINE WITH A MINIMUM ALLOWABLE BENDING Fb=2400 PSI, MIN. SHEAR Fv=165 PSI & MIN. E=1,800 KSI.
- ROOF DECKING:  
A. PANEL SHALL BE APPLIED WITH LONG DIMENSION PERPENDICULAR TO FRAMING. ADJACENT ROW OF PANELS SHALL BE STAGGERED.  
B. PANELS SHALL BE FASTENED TO SUPPORTS USING 8d NAILS SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS AND AT 6" O.C. AT SUPPORTED PANEL EDGES.  
C. 1/8" SPACE SHALL BE ALLOWED AT ALL PANEL ENDS AND EDGES  
D. PANELS SHALL BE INSTALLED CONTINUOUSLY OVER TWO OR MORE SPANS.
- CONNECTIONS:  
A. THE FRAMING CONTRACTOR SHALL FURNISH ALL HARDWARE REQUIRED TO MAKE CONNECTIONS BETWEEN VARIOUS ELEMENTS. UNLESS NOTED OTHERWISE, REFER TO NAILING SCHEDULE FOR SIZE OF NAILS.  
B. SOLID BLOCKING SHALL BE PROVIDED BETWEEN ALL JOISTS AND TRUSSES AT EACH SUPPORT.  
C. ALL CONNECTIONS NOT SPECIFICALLY DETAILED SHALL UTILIZE JOIST HANGERS, BEAM HANGERS, CONNECTORS, COLUMN BASES, ETC. THIS HARDWARE SHALL BE MANUFACTURED BY SIMPSON COMPANY AND SHALL BE SELECTED AND INSTALLED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS.  
D. ALL BOLTED CONNECTIONS SHALL HAVE WASHERS AT BOLT HEAD AS WELL AS AT THE NUT.
- TRUSS FABRICATION:  
A. TRUSS FABRICATOR SHALL VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.  
B. LOADS FROM ALL EQUIPMENT SHOWN ON PLAN SHALL BE INCLUDED. REFER TO TRUSS PROFILE ON SHEET S-6.  
C. TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS STAMPED BY A REGISTERED ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.  
D. TRUSS MANUFACTURER SHALL PROVIDE ALL NECESSARY BRIDGING. TRUSSES SHALL BE DESIGNED FOR THE NET WIND UPLIFT LOAD NOTED ABOVE.  
E. ALL TRUSSES SHALL BE DESIGNED WITH THE FOLLOWING DEFLECTION CRITERIA:  
- ALLOWABLE LL DEFLECTION = L/360  
- ALLOWABLE TL DEFLECTION = L/240  
F. ALL TRUSSES SHALL BE FASTENED TO SUPPORTS WITH GALVANIZED METAL CONNECTORS AS INDICATED IN FRAMING DETAILS.  
G. ALL TRUSSES SHALL BE DESIGNED AND FABRICATED USING #3 SOUTHERN PINE OR #1 DOUGLAS-FIR PER THE RECOMMENDATIONS OF THE TP1 DESIGN SPECIFICATIONS.

## STEEL

- STRUCTURAL STEEL SHALL BE NEW STEEL AND SHALL CONFORM TO THE A. I. S. C. "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS."
- STRUCTURAL STEEL ROLLED SHAPES AND PLATES NOTED "ROLLED 50" SHALL CONFORM TO ASTM A-572, GRADE 50. ALL OTHER ROLLED SHAPES AND PLATES SHALL CONFORM TO ASTM A-36.
- STRUCTURAL STEEL PIPE SHALL CONFORM TO ASTM A-53, TYPES "E" OR "S", GRADE B OR ASTM 501.
- STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A-500, GRADE B.
- ALL STRUCTURAL STEEL CONNECTIONS AND DETAILS SHALL CONFORM TO THE A. I. S. C. "CODE OF STANDARD PRACTICE FOR STEEL, BUILDINGS AND BRIDGES."
- UNFINISHED THREADED FASTENERS SHALL CONFORM TO ASTM A-307, GRADE A BOLTS AND NUTS WITH HEXAGONAL HEADS. UNFINISHED THREADED FASTENERS SHALL BE USED ONLY FOR BOLTED CONNECTIONS OF SECONDARY FRAMING MEMBERS TO PRIMARY MEMBERS, FOR TEMPORARY BRACING TO FACILITATE ERECTION, AND FOR ANCHORAGE TO CONCRETE OR MASONRY CONSTRUCTION.
- BOLTED CONNECTIONS OF PRIMARY MEMBERS SHALL CONFORM TO THE REQUIREMENTS OF THE A. I. S. C. "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS." UNLESS NOTED OTHERWISE, ALL PRIMARY CONNECTIONS WITH HIGH STRENGTH BOLTS SHALL USE ASTM A-325 BOLTS AND HEAVY HEX NUTS.
- WELDED CONSTRUCTION SHALL CONFORM TO AWS D-1.1. "STRUCTURAL WELDING CODE." WELDING PROCESSES AND OPERATORS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATIONS PROCEDURE." ELECTRODES FOR FIELD AND SHOP WELDS SHALL BE E70XX, UNLESS NOTED OTHERWISE.
- SHEAR STUDS SHALL CONFORM TO ASTM A-108, HEADED ANCHORS.
- STEEL MEMBERS SHALL NOT BE SPLICED EXCEPT AS SHOWN ON THE DRAWINGS.
- ALL EXTERIOR STEEL EXPOSED TO WEATHER, INCLUDING LINTELS, SHALL BE GALVANIZED.

## REINFORCED MASONRY

- ALL REINFORCED MASONRY SHALL CONFORM TO THE PROVISIONS OF ACI 530.1/ASCE 6 (WITH EXCEPTIONS NOTED IN THE SPECIFICATIONS).
- CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90, GRADE N, TYPE 1, LIGHTWEIGHT UNITS WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI.
- MORTAR SHALL CONFORM TO ASTM C-270, TYPE M OR S. AGGREGATES FOR MORTAR SHALL CONFORM TO ASTM C-144. MORTAR SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS.
- GROUT SHALL CONFORM TO ASTM C-476. AGGREGATES FOR GROUT SHALL CONFORM TO ASTM C-404. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. CONSOLIDATE ALL GROUTED MASONRY WITH MECHANICAL VIBRATION.
- MASONRY CONSTRUCTION SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'm) OF 1500 PSI AT 28 DAYS.
- SPECIAL INSPECTION SHALL BE PROVIDED BY THE TESTING LAB AND REPORTED TO THE ENGINEER. THE TESTING LAB SHALL PERFORM PERIODIC INSPECTIONS DURING PREPARATION AND TAKING OF PRISMS OR TEST SPECIMENS, PLACING OF REINFORCING AND GROUT, AND LAYING UP OF MASONRY.
- BOND BEAMS SHALL BE CONSTRUCTED WITH PORTLAND CEMENT GROUT HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS, AND A MAXIMUM AGGREGATE SIZE OF 3/4".
- REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60.
- HORIZONTAL JOINT REINFORCEMENT SHALL BE FACTORY FABRICATED, TRUSS TYPE, 9 GAGE WIRE CONFORMING TO ASTM A-82. PROVIDE 1-#4 MINIMUM ABOVE AND BELOW ALL WALL OPENINGS AND EXTENDING 24" MINIMUM INTO JAMBS.
- ALL CELLS CONTAINING REINFORCEMENT, BOLTS, OR OTHER METAL ANCHORS SHALL BE GROUTED SOLID. ANY CELLS AT OR BELOW GRADE SHALL BE GROUTED SOLID, WHETHER REINFORCED OR NOT.

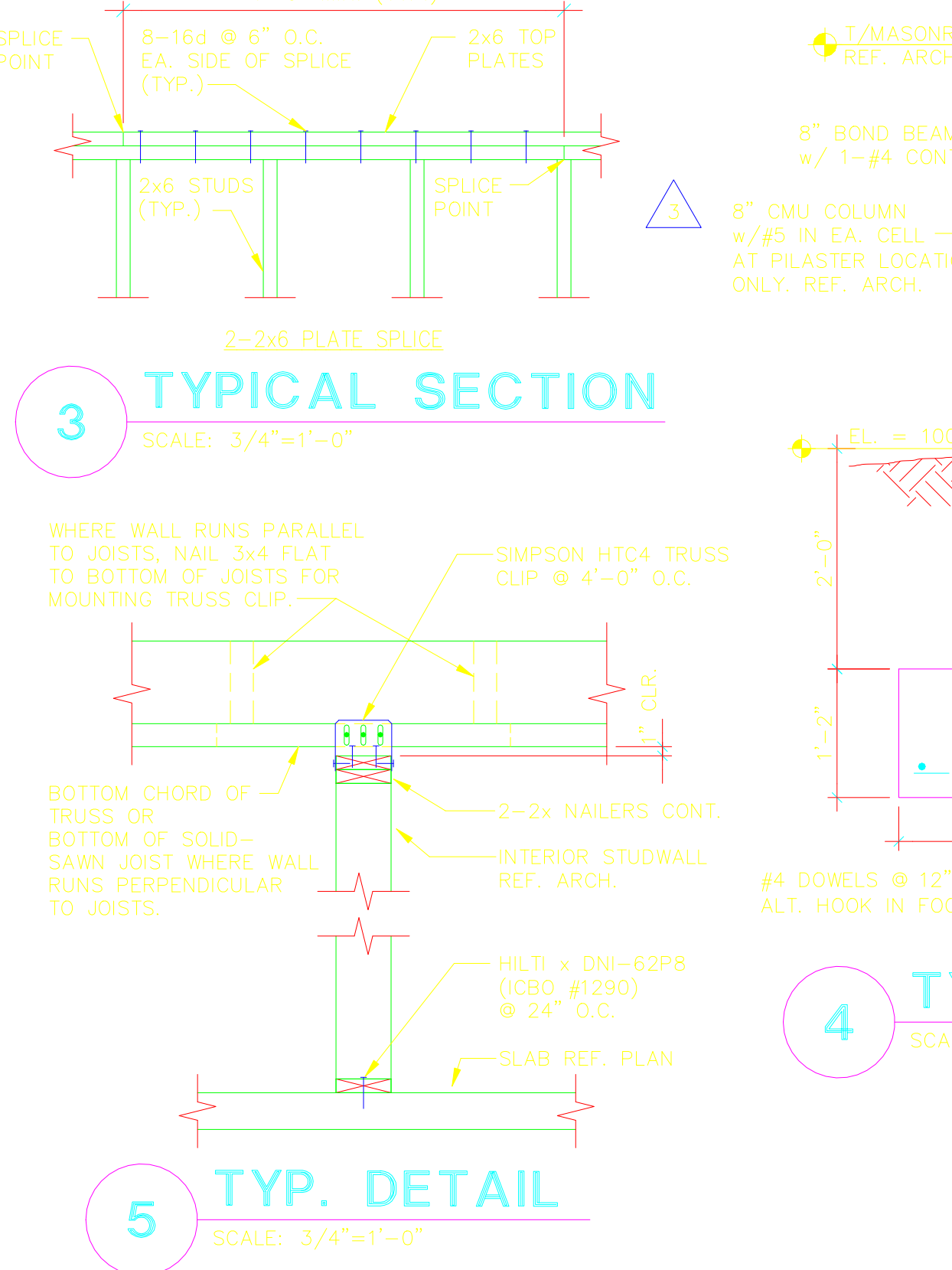
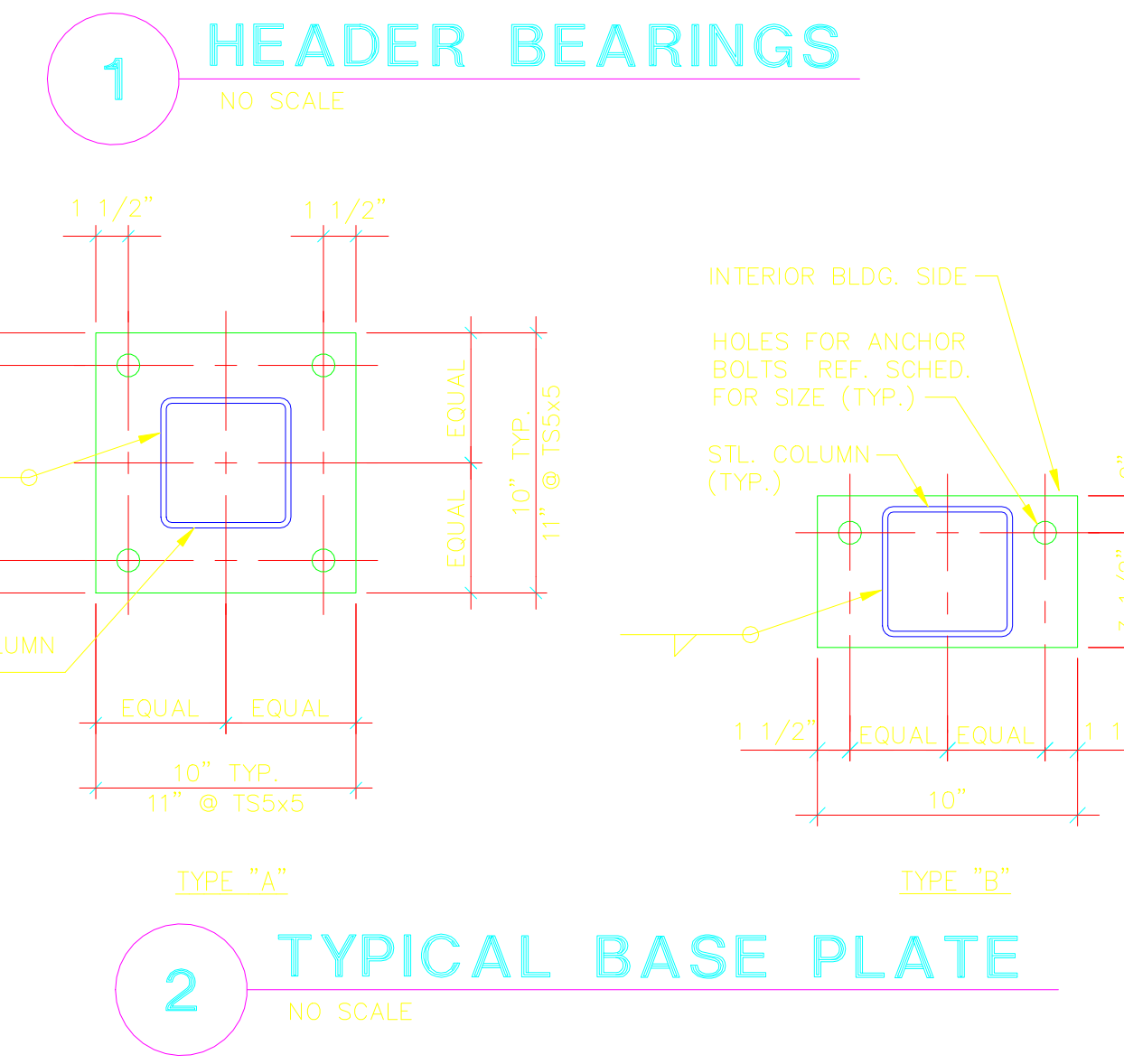
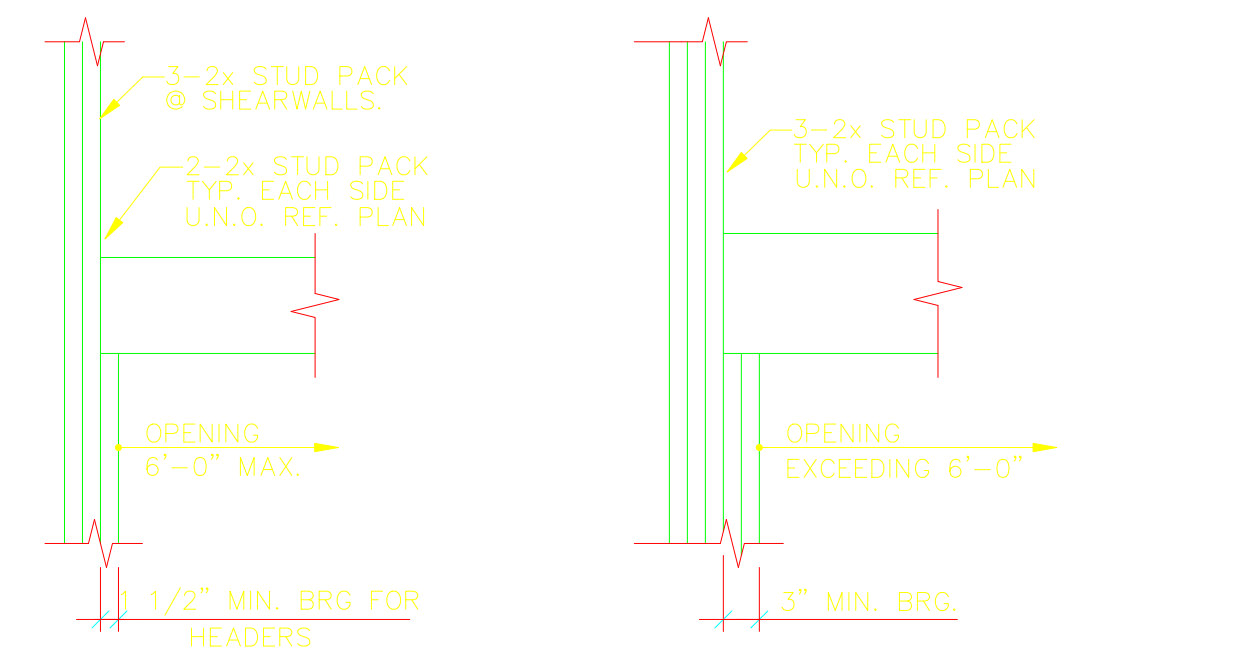
## NAILING SCHEDULE

CONNECTION	NAILING
(1) JOIST TO SILL OR GIRDER, TOENAIL	3-16d
(2) BRIDGING TO JOIST, TOENAIL EACH END	2-8d
(3) 1" x 6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	2-8d
(4) WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST, FACE NAIL	3-8d
(5) 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	2-16d
(6) SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	16d @ 16" O.C.
(7) TOP PLATE TO STUD, END NAIL	2-16d
(8) STUD TO SOLE PLATE	4-8d TOENAIL OR 2-16d O.C. ALONG EACH EDGE
(9) DOUBLE STUDS, FACE NAIL	16d @ 24" O.C.
(10) DOUBLED TOP PLATES, FACE NAIL	16d @ 16" O.C.
(11) TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	2-16d
(12) CONTINUOUS HEADER, TWO PIECES	16d @ 16" O.C. ALONG EACH EDGE
(13) CEILING JOISTS TO PLATE, TOENAIL	3-8d
(14) CONTINUOUS HEADER TO STUD, TOENAIL	4-8d
(15) CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	3-16d
(16) CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	3-16d
(17) RAFTER TO HIP	3-10d TOENAIL OR 2-16d FACE NAIL
(18) 1" BRACE TO EACH STUD AND PLATE, FACE NAIL	2-8d
(19) 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL	2-8d
(20) WIDER THAN 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL	3-8d
(21) BUILT-UP CORNER STUDS	16d @ 24" O.C. O.C.
(22) BUILT-UP GIRDER AND BEAMS	20d @ 32" O.C. @ TOP & BOT. & STAGGERED 2-20d @ ENDS & @ EACH SPlice

### MAXIMUM CLEAR SPAN FOR HEADERS

HEADER SIZE MATERIAL AT EDGE	SUPPORTING ONLY CEILING AND ROOF
3-2x6	4'-0"
3-2x8	6'-0"
3-2x10	8'-0"
3-2x12	10'-0"

- HEADER SIZES MAY BE USED IF NO SIZES ARE CALLED OUT ON THE PLANS OR DETAILS.
- FOR WALL OPENING SIZES REFERENCE ARCHITECTURAL DRAWINGS.
- USE 1/2" FLITCH BETWEEN 3-2x HEADERS.



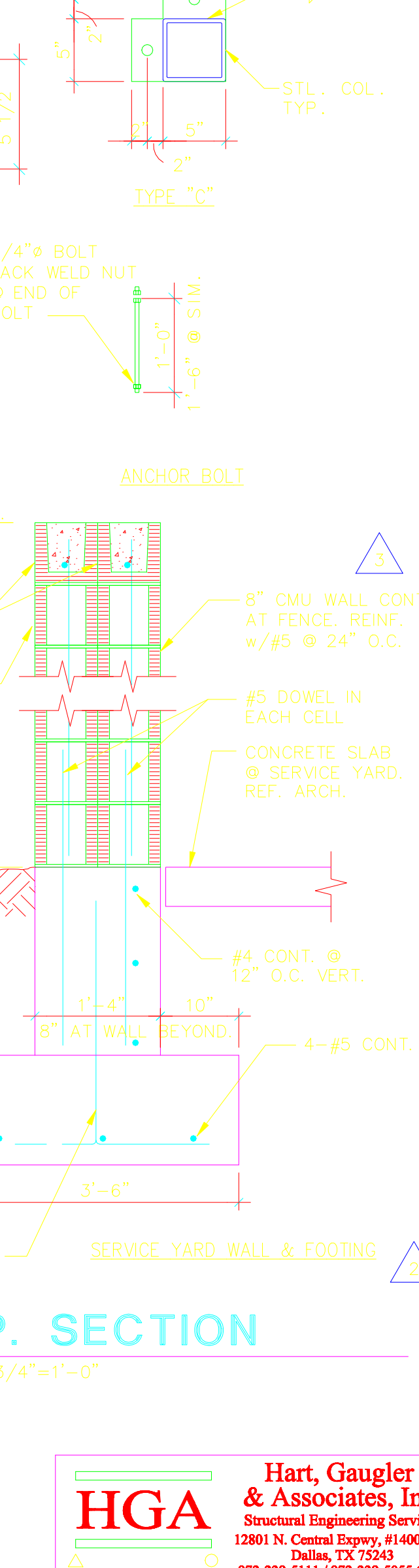
### LAP SPLICE SCHED.

BAR SIZE	LAP	BAR SIZE	LAP
3	1'-10"	8	4'-9"
4	2'-3"	9	6'-1"
5	2'-8"	10	7'-3"
6	3'-2"		
7	3'-9"		

### HANGER SCHEDULE

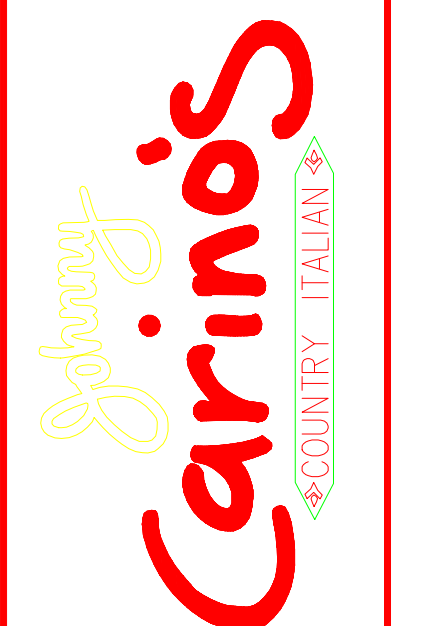
JOISTS	HANGER
2x8	U26
2x10	U28
2x12	U210
2-2x12	U210-2
3-2x12	U210-3
5 1/4"x11 1/4" PSL	HCLTV5.511
5 1/4"x18" PSL	GLTV5.518

- PROVIDE THESE HANGERS U.N.O. ON THE PLANS OR SECTIONS.
- ALL HANGERS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE.
- HANGERS FOR WOOD I-JOISTS SHALL BE SPECIFIED BY I-JOIST MANUFACTURER (U.N.O.).
- SLOPE AND SKEW HANGERS AS REQ'D. REF. PLANS AND ARCH. ELEVATIONS.



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28 OCT. 2003	ADDENDUM #1
12 NOV. 2003	CITY COMMENTS
18 DEC. 2003	CITY COMMENTS
28 JAN. 2004	OWNER COMMENTS

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HWY 287 AT PEARL RD.  
CASA GRANDE, AZ



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FOUNDATION PLAN NOTES:

1. FINISH FLOOR EL. IS 100'-0" REF. CIVIL FOR ACTUAL FINISH FLOOR ELEVATION.
2. COORDINATE ARCH. FOR EXACT LOCATIONS OF ALL DOWELED CONCRETE CURBS.
3. COORDINATE ALL DRAIN LOCATIONS AND PIPE PENETRATIONS w/ PLUMBING AND KITCHEN SHEETS.
4. DENOTES SHEAR WALL AND HOLD-DOWN LOCATIONS.
5. REF. GEN. NOTES FOR SUBGRADE PREPARATIONS.
6. REF. 2/S-1 FOR TYP. BASE PLATE DETAIL.

**1** FOUNDATION PLAN  
SCALE: 1/4"=1'-0"

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# 1 ROOF FRAMING PLAN

SCALE: 1/4"=1'-0"

ROOF FRAMING PLAN NOTES:

1. DATUM EL. = 100'-0". REF. CIVIL FOR ACTUAL F.F. EL.
2. REF. GEN. NOTES FOR TYP. ROOF DECK AND SHEATHING.
3. ROOF OPENINGS SHALL BE FRAMED w/ BLOCKING AND SIMPSON HANGERS PER TJI MFR. U.N.O.
4. COORDINATE SIZE, LOCATION, AND BLOCKING FOR MECH. UNIT CURBS w/ MECH. CONTRACTOR. INCLUDE ALL LOADS FROM MECH. UNITS IN DESIGN OF ALL ROOF JOISTS. LOADS SHOWN INCLUDE 20% INCREASE PER CODE FOR IMPACT LOADING.
5. REFER TO SHEET S-1 FOR TYPICAL HEADER SCHED.
6. ROOF JOISTS NOTED R.S. ARE ROUGH SAWN FIR. REF. ARCH.
7. REF. ARCH. FOR EXHAUST HOOD LOCATIONS. HOODS MAY BE HUNG FROM STRUCTURE ABOVE w/ 2-2x6 AND SIMPSON HANGERS @ EA. END BETWEEN JOISTS. REF. SUPPLIER FOR PICK-UP POINTS.
8. REF. FOUNDATION PLAN FOR SHEARWALL LOCATIONS.

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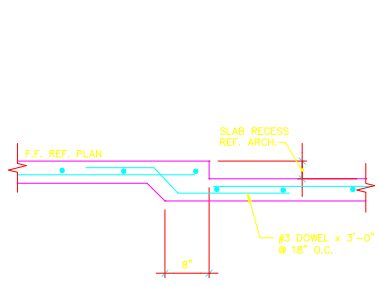
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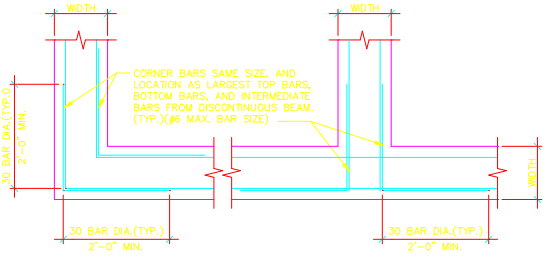
DRAWN: JLL    DESIGN: JEK    HGA JOB # 203180

SHEET NO: S-3

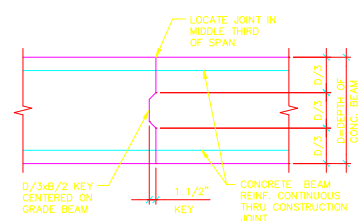
HGA JOB # 203180



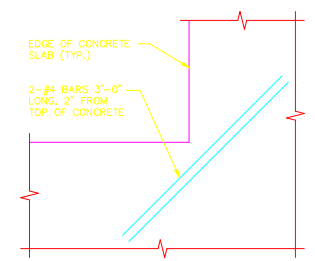
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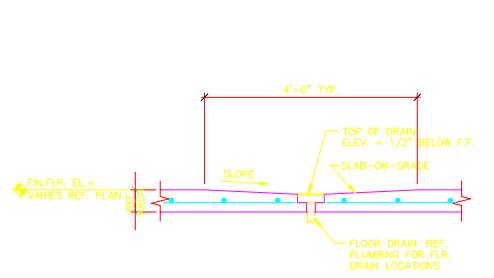
**2 TYPICAL DETAIL**  
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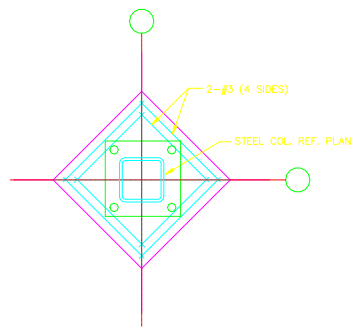
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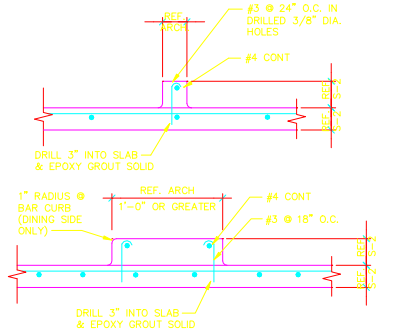
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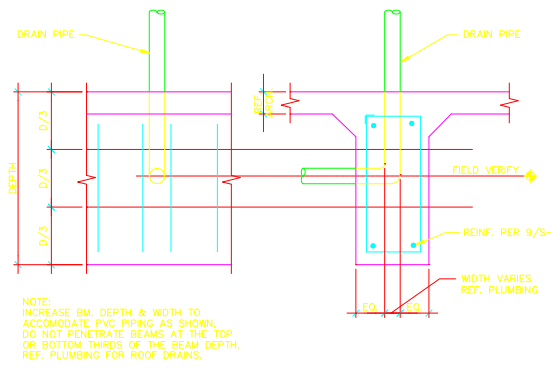
**5 TYPICAL DRAIN DETAIL**  
SCALE: NO SCALE



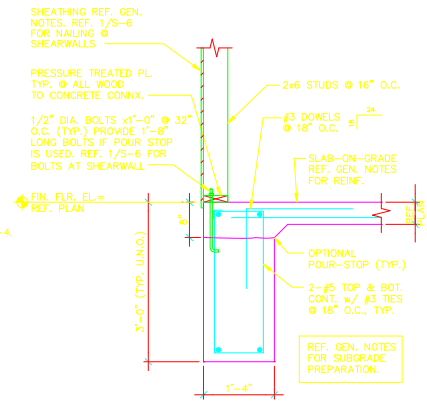
**6 TYP. SLAB LEAVE-OUT AT INTERIOR COLUMN**  
SCALE: NO SCALE



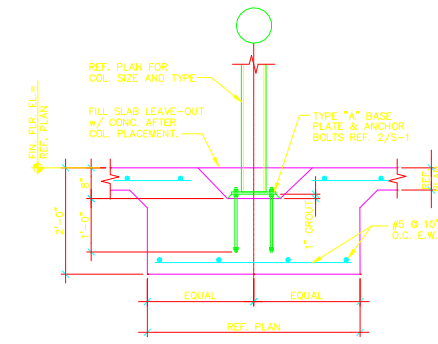
**7 TYPICAL CURB DETAIL**  
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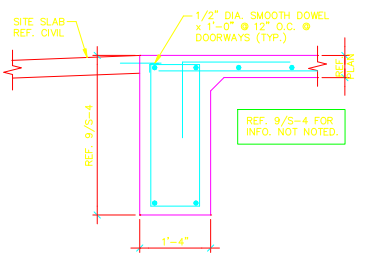
**8 TYP. GRADE BM. PENETRATION**  
SCALE: NO SCALE



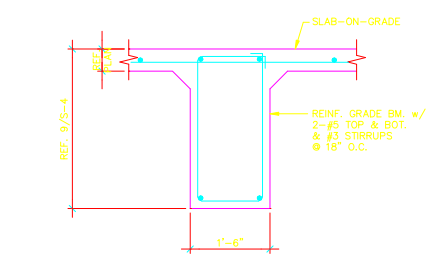
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SCALE: 3/4"=1'-0"



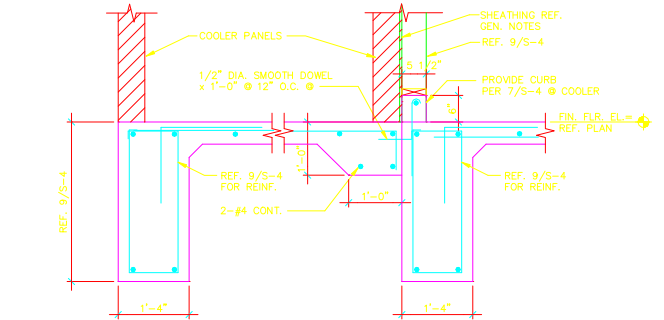
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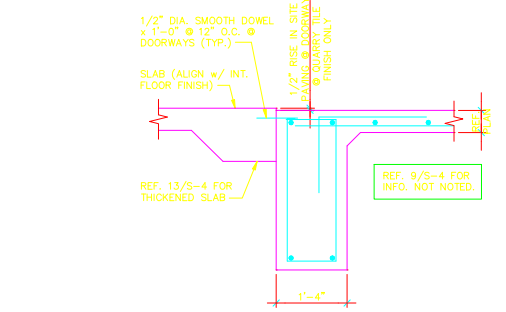
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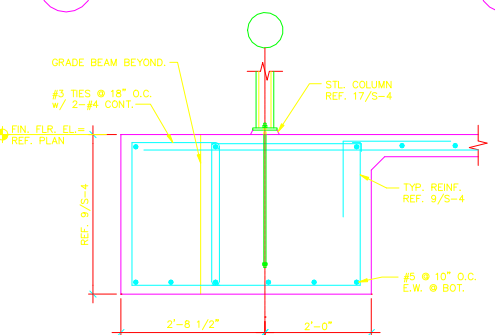
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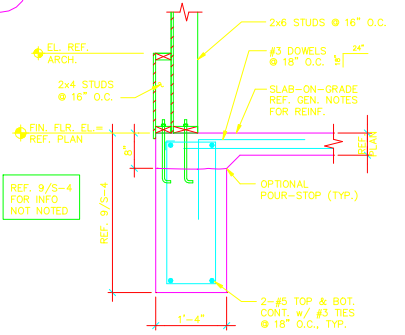
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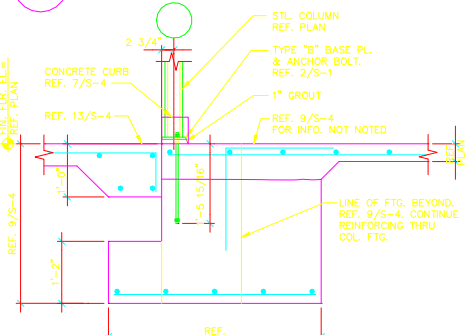
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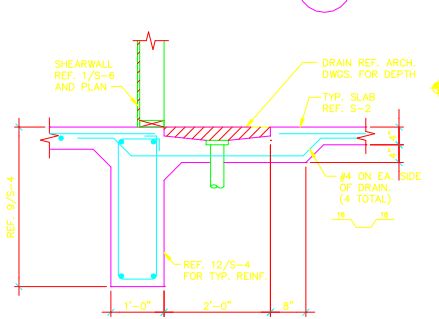
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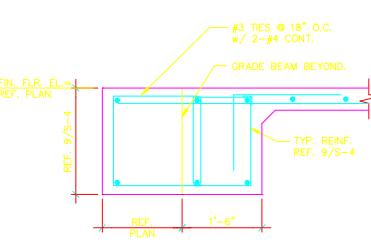
**16 SECTION**  
SCALE: 3/4"=1'-0"



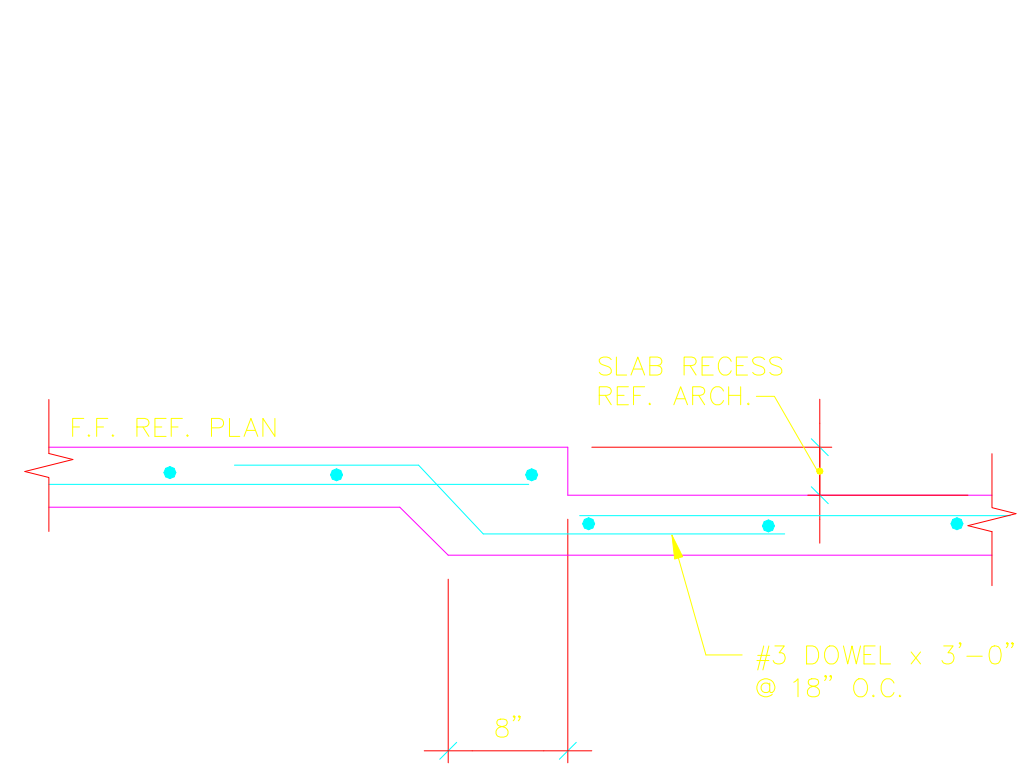
**17 SECTION**  
SCALE: 3/4"=1'-0"



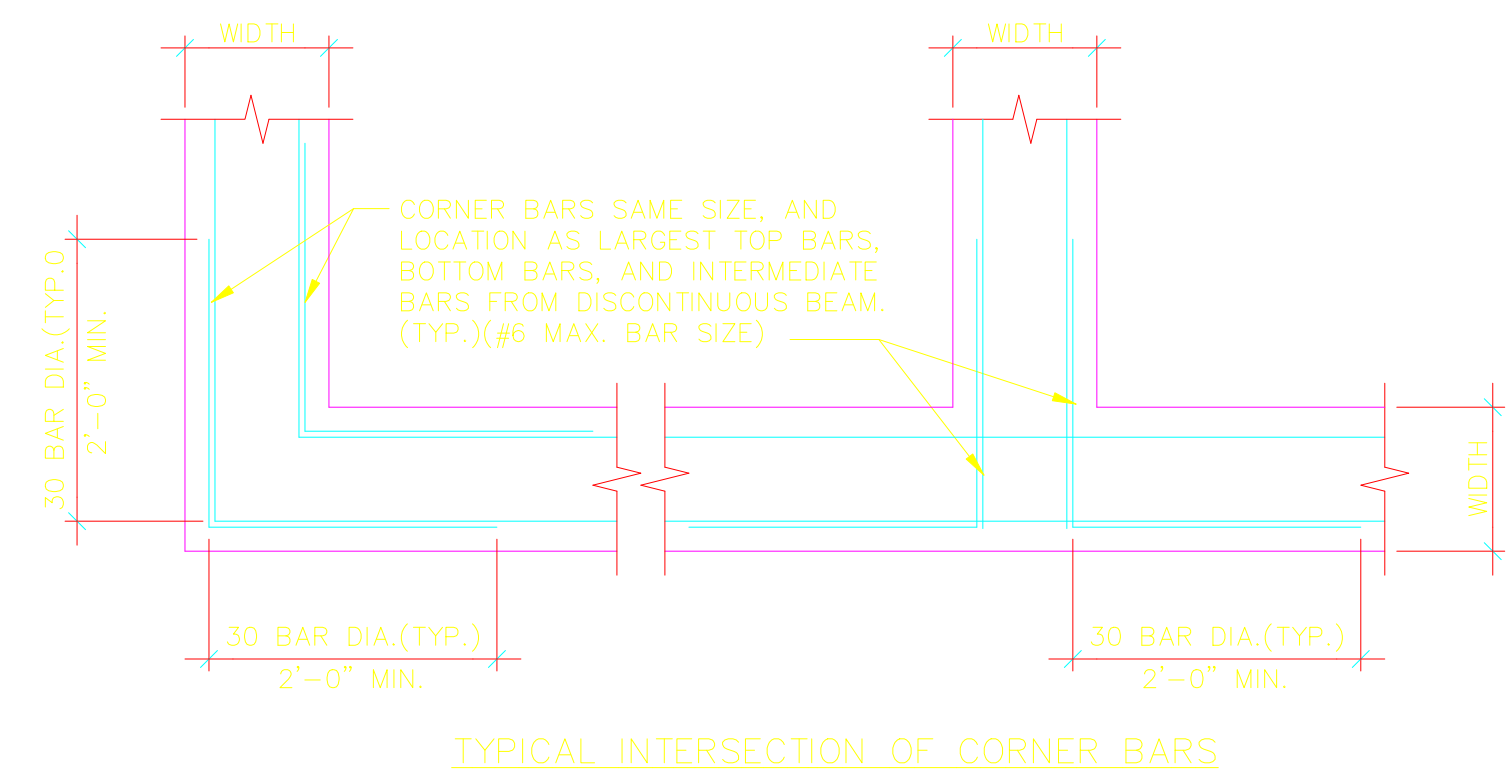
**18 RECESS AT MOP SINK**  
SCALE: 3/4"=1'-0"



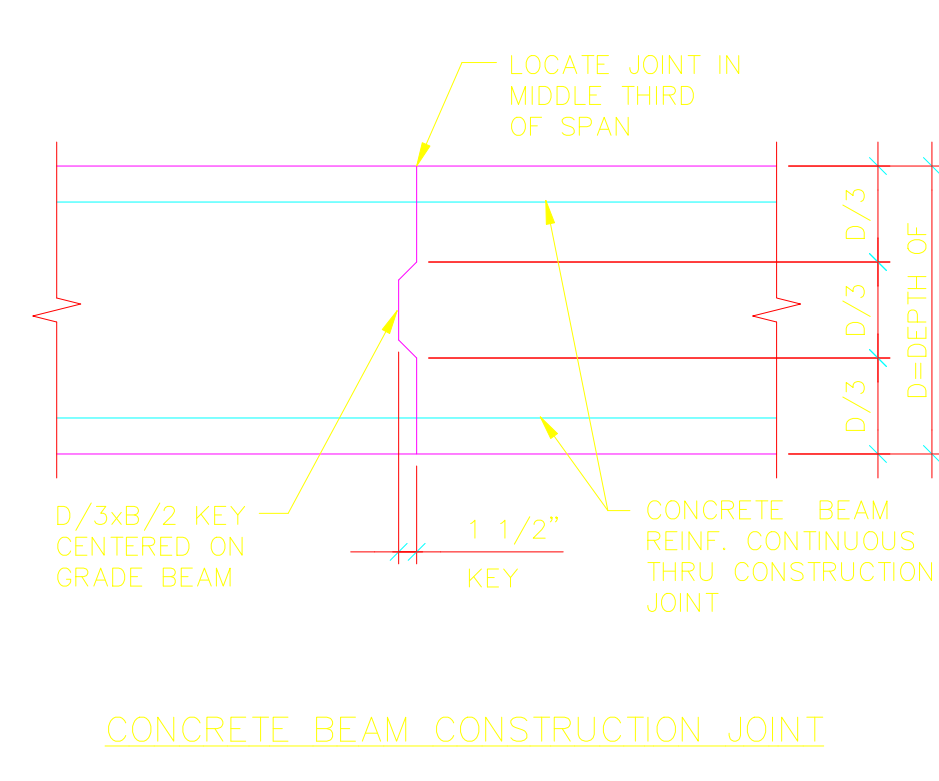
**19 SECTION**  
SCALE: 3/4"=1'-0"



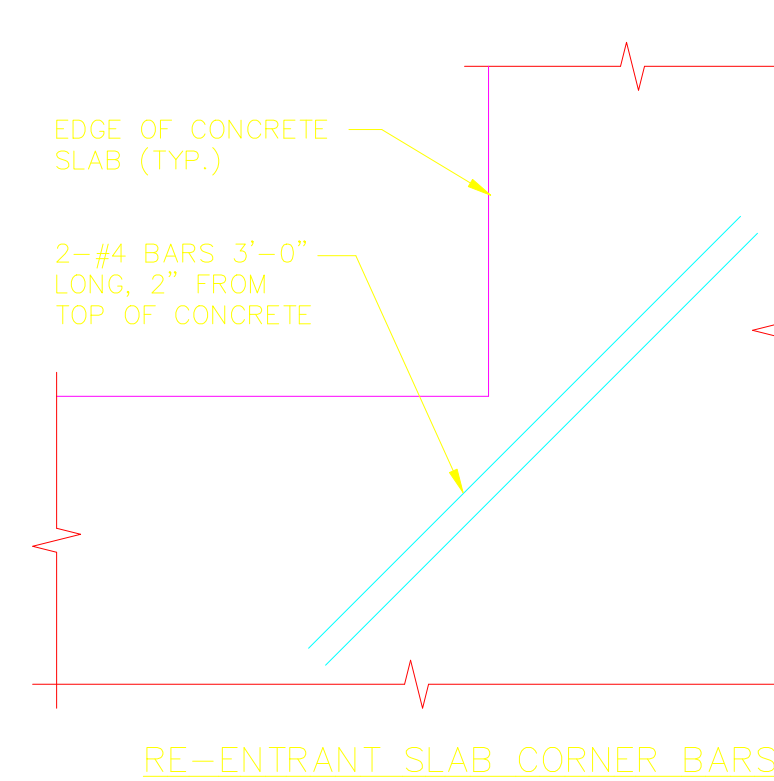
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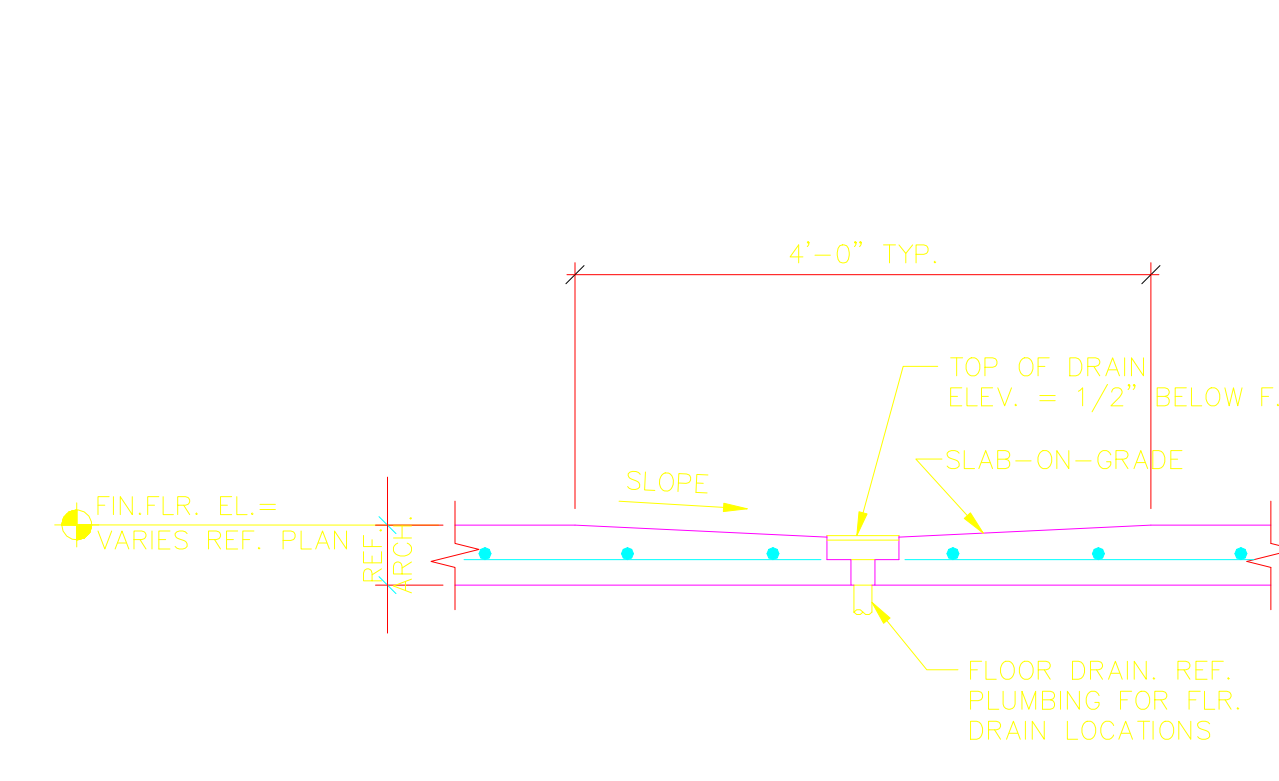
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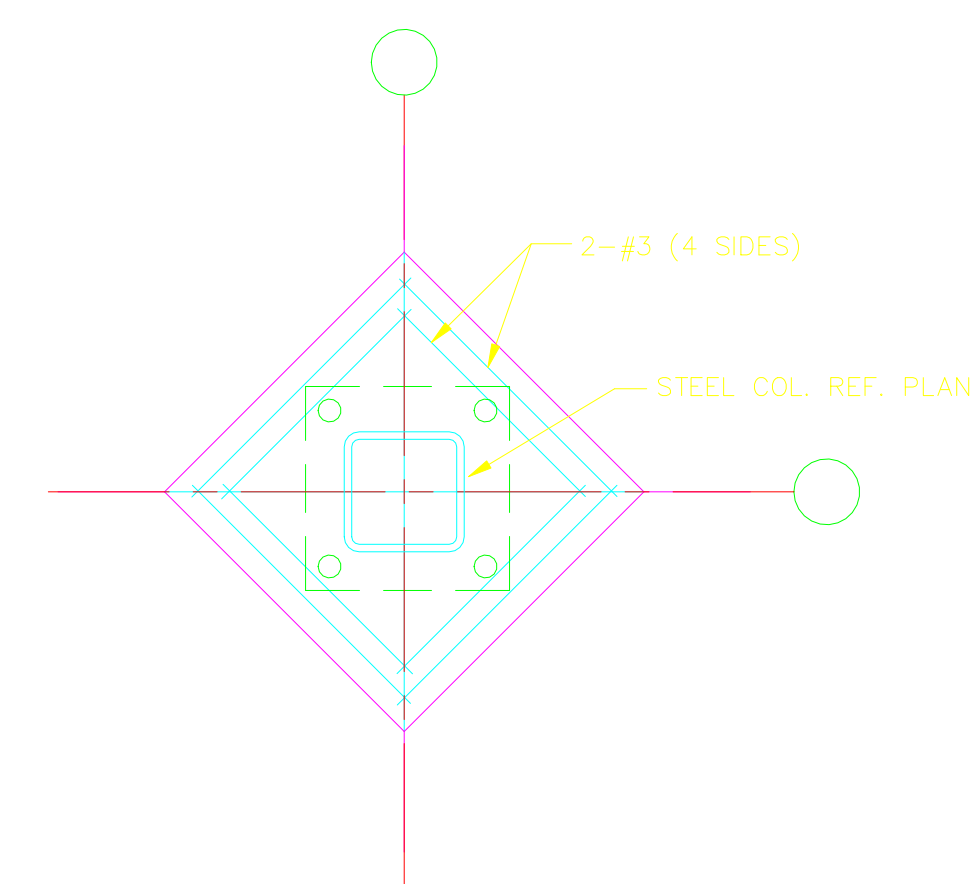
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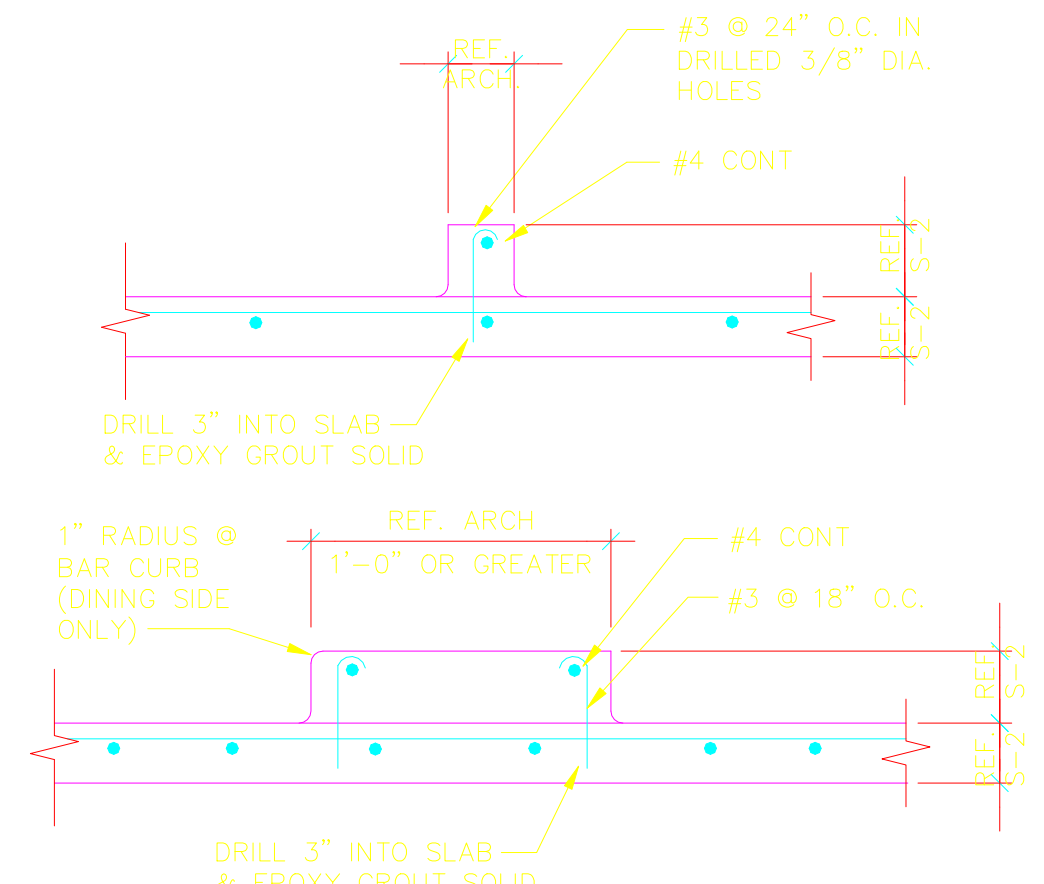
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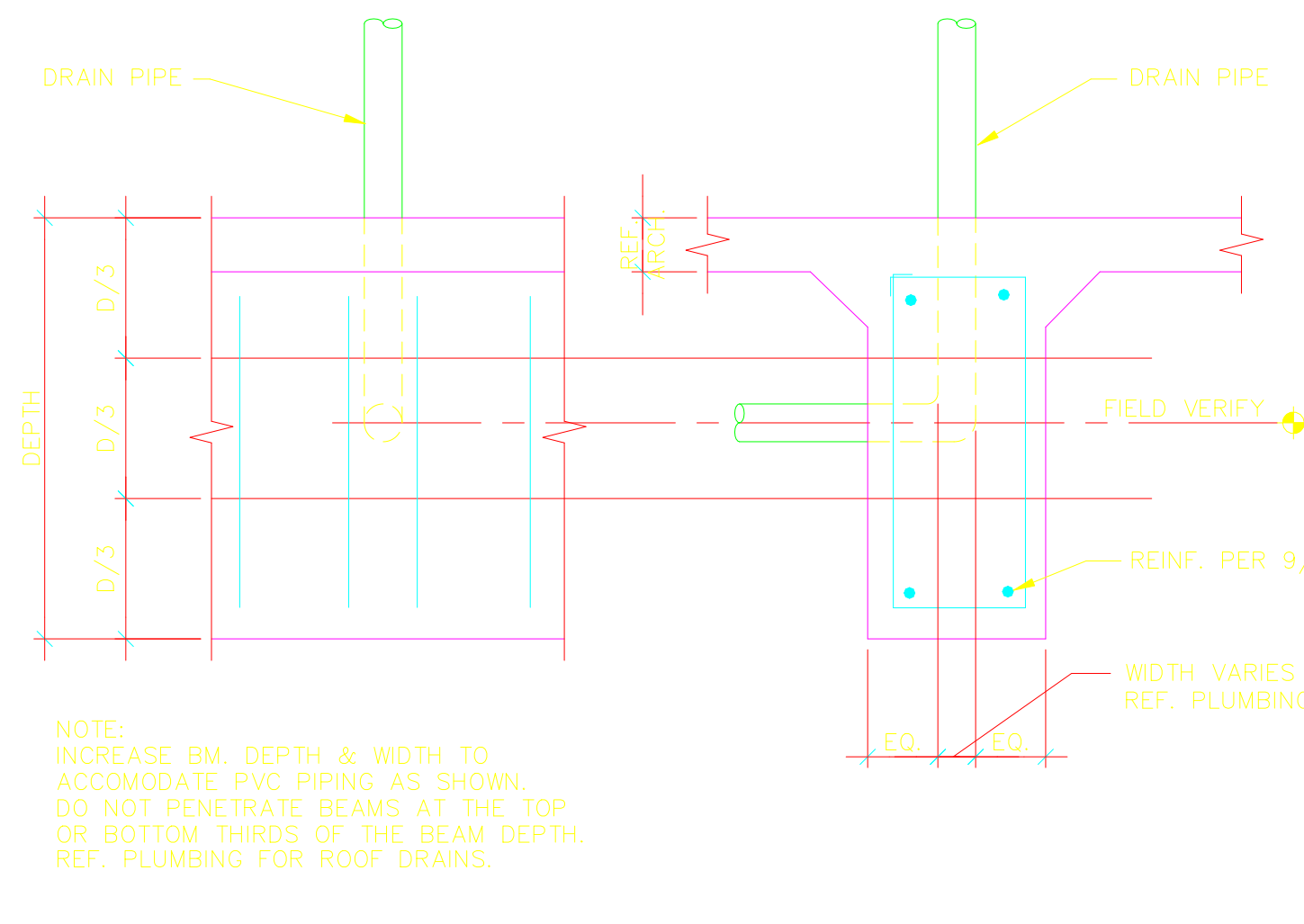
**5 TYPICAL DRAIN DETAIL**  
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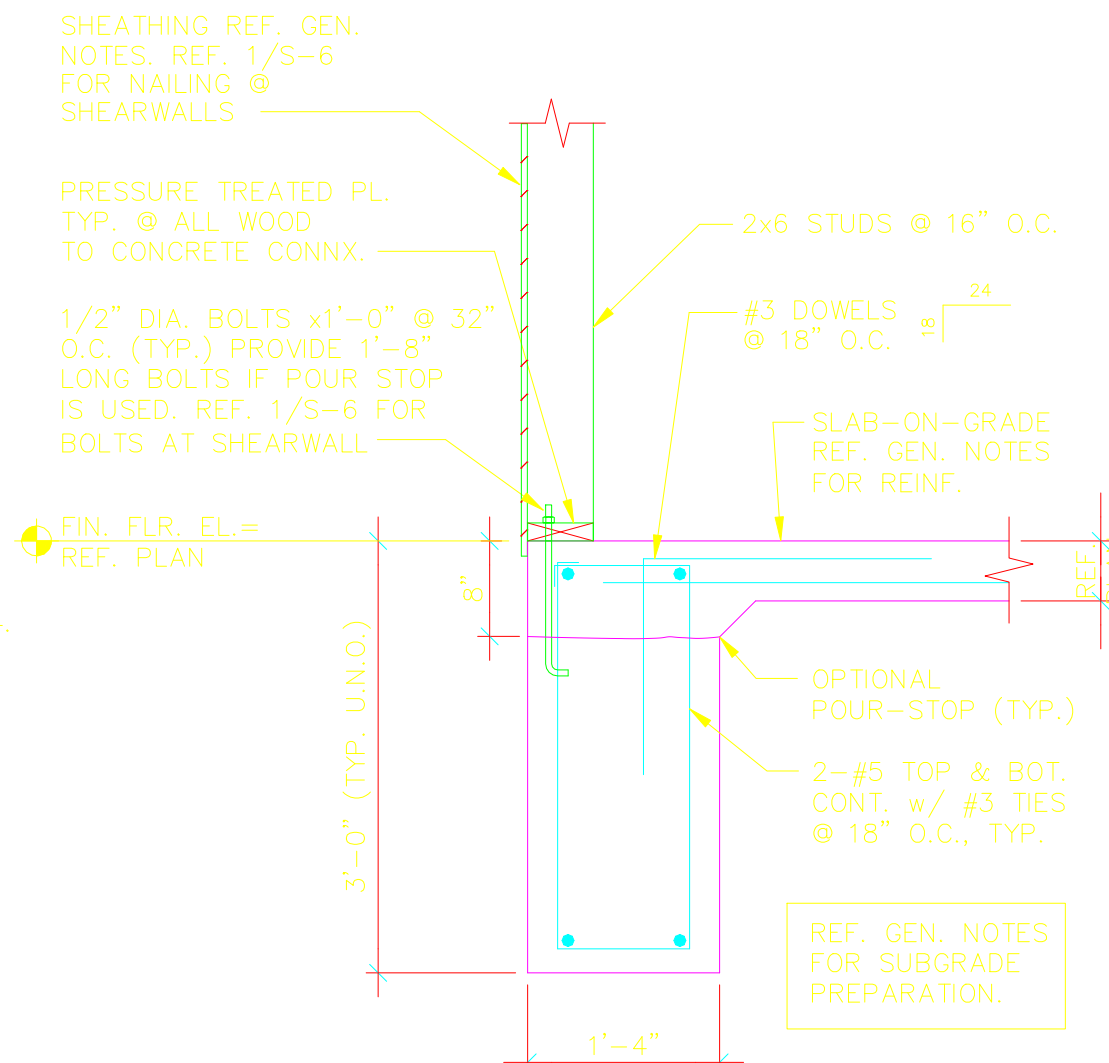
**6 TYP. SLAB LEAVE-OUT AT INTERIOR COLUMN**  
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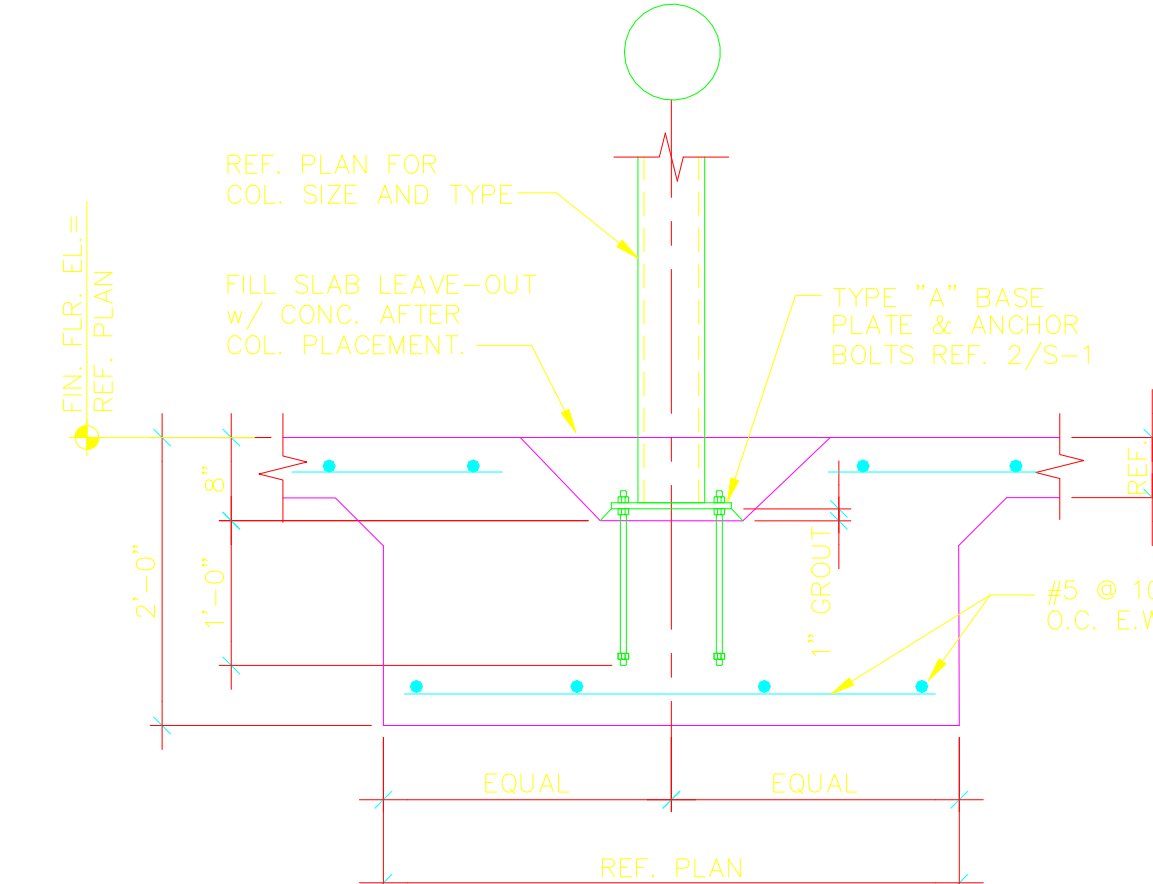
**7 TYPICAL CURB DETAIL**  
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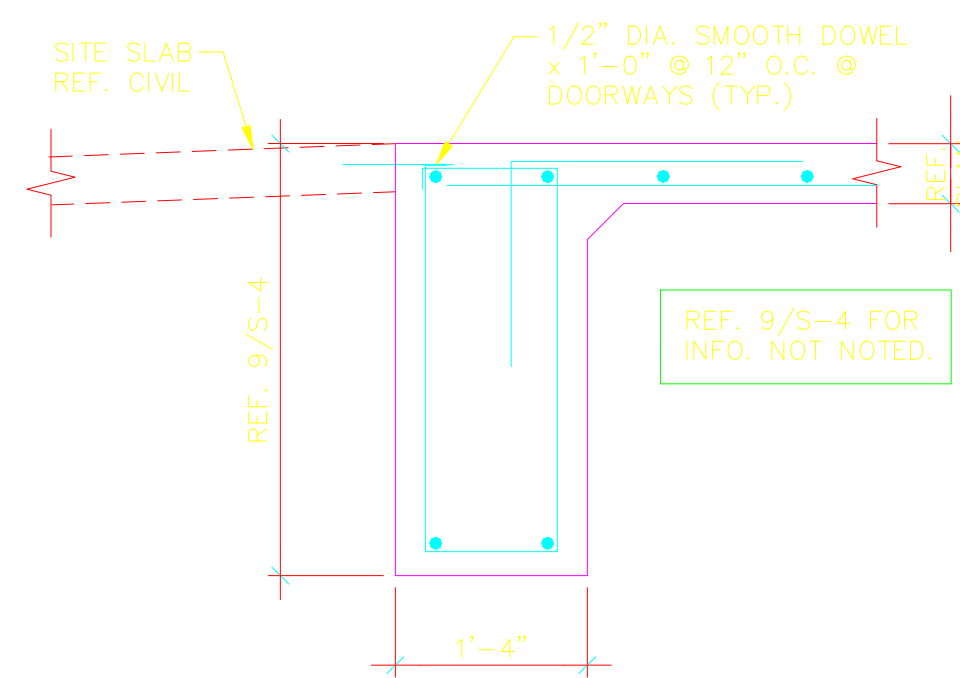
**8 TYP. GRADE B.M. PENETRATION**  
SCALE: NO SCALE



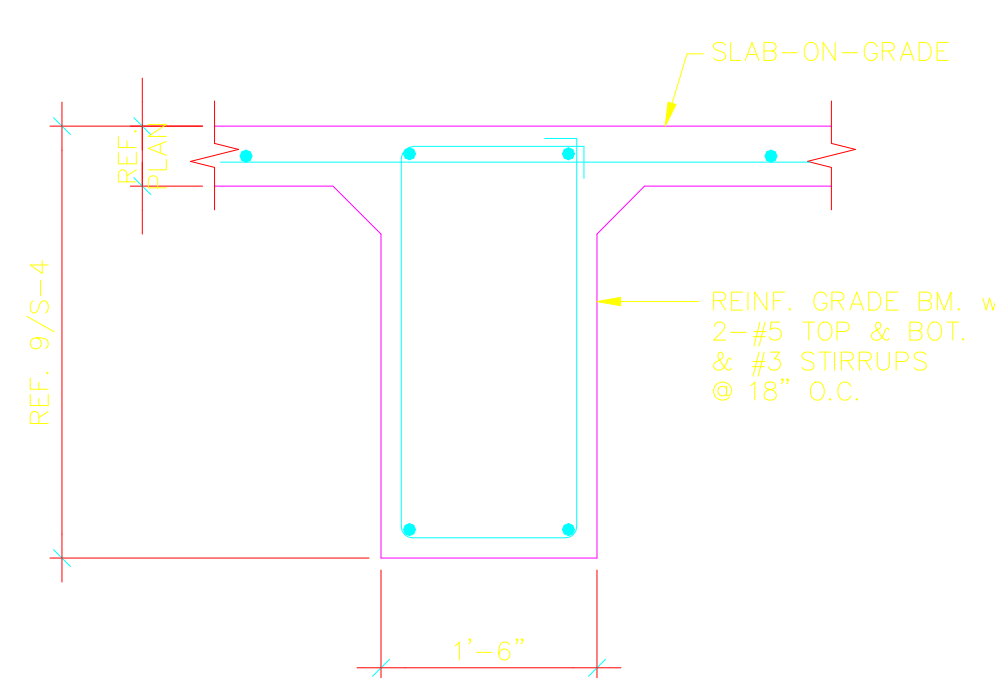
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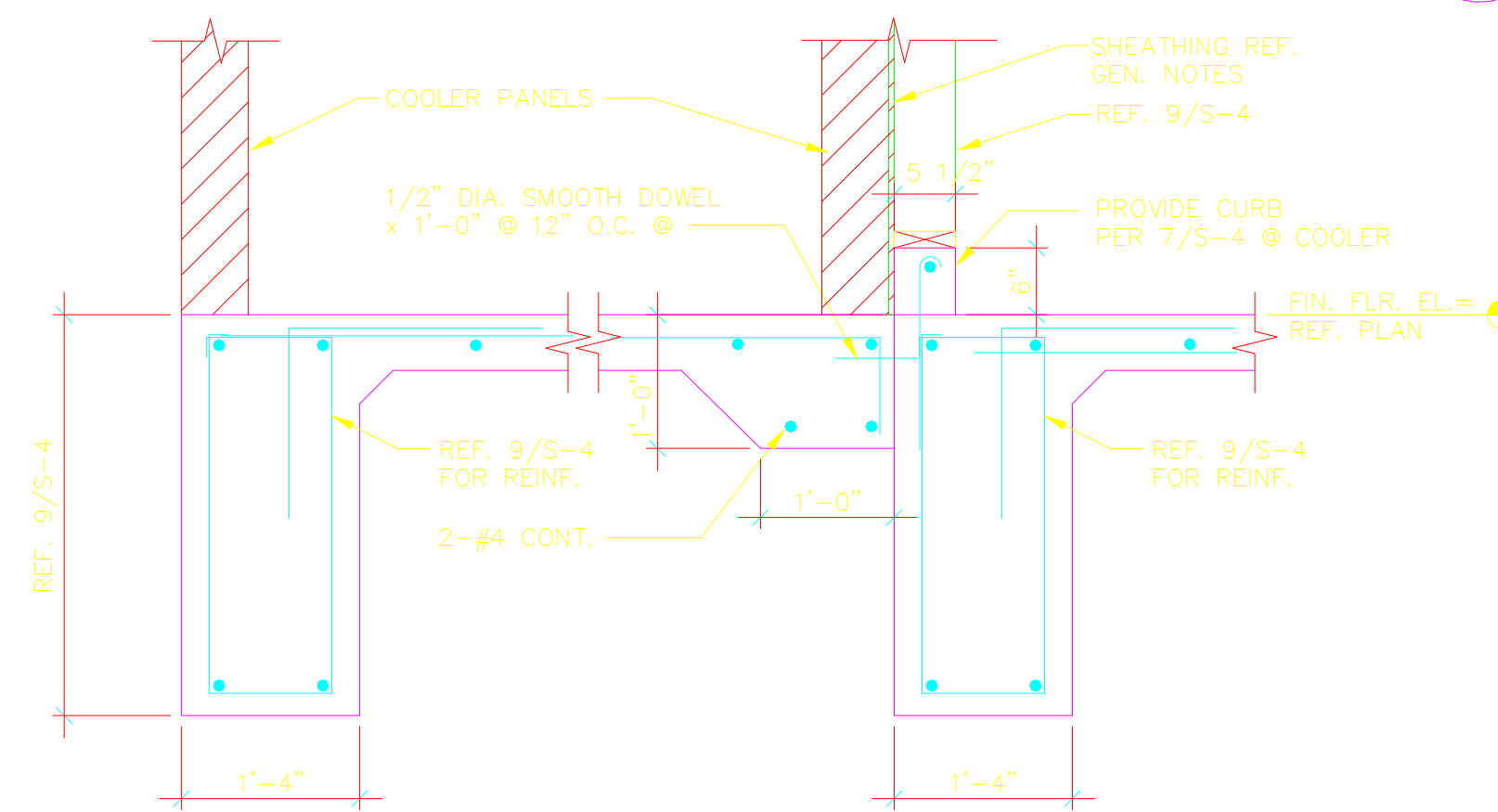
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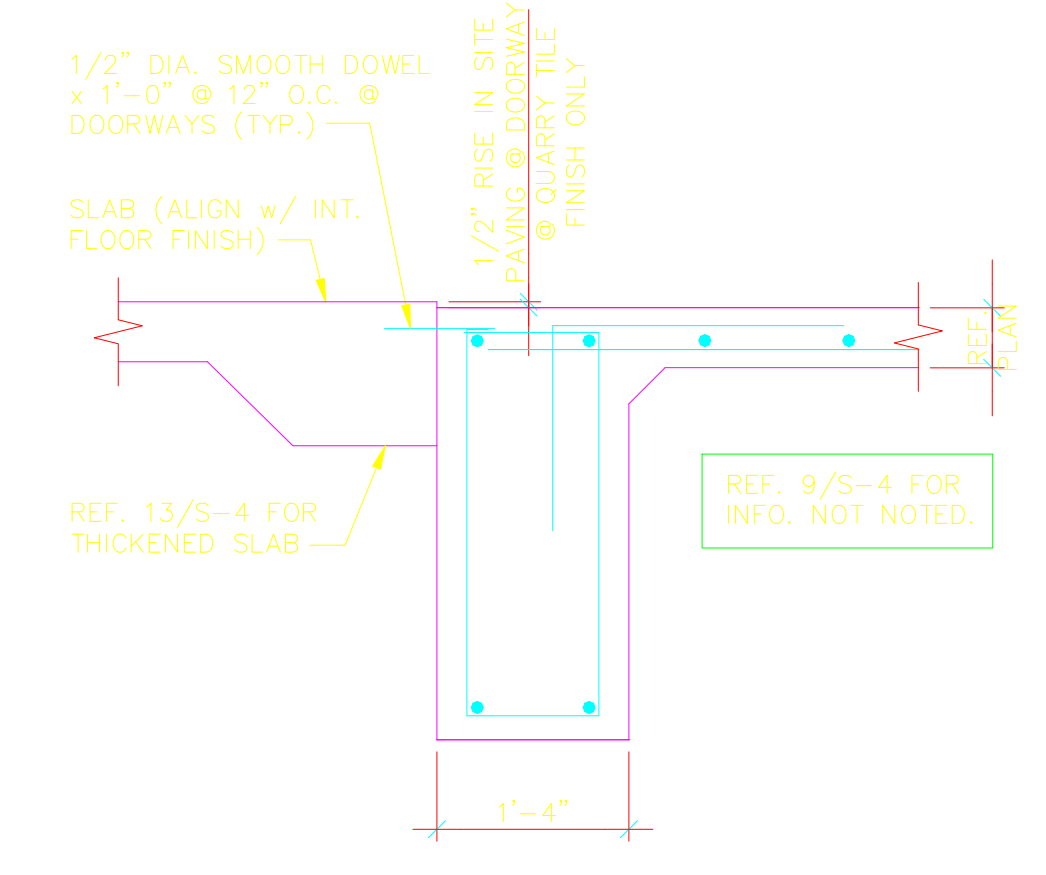
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SCALE: 3/4"=1'-0"



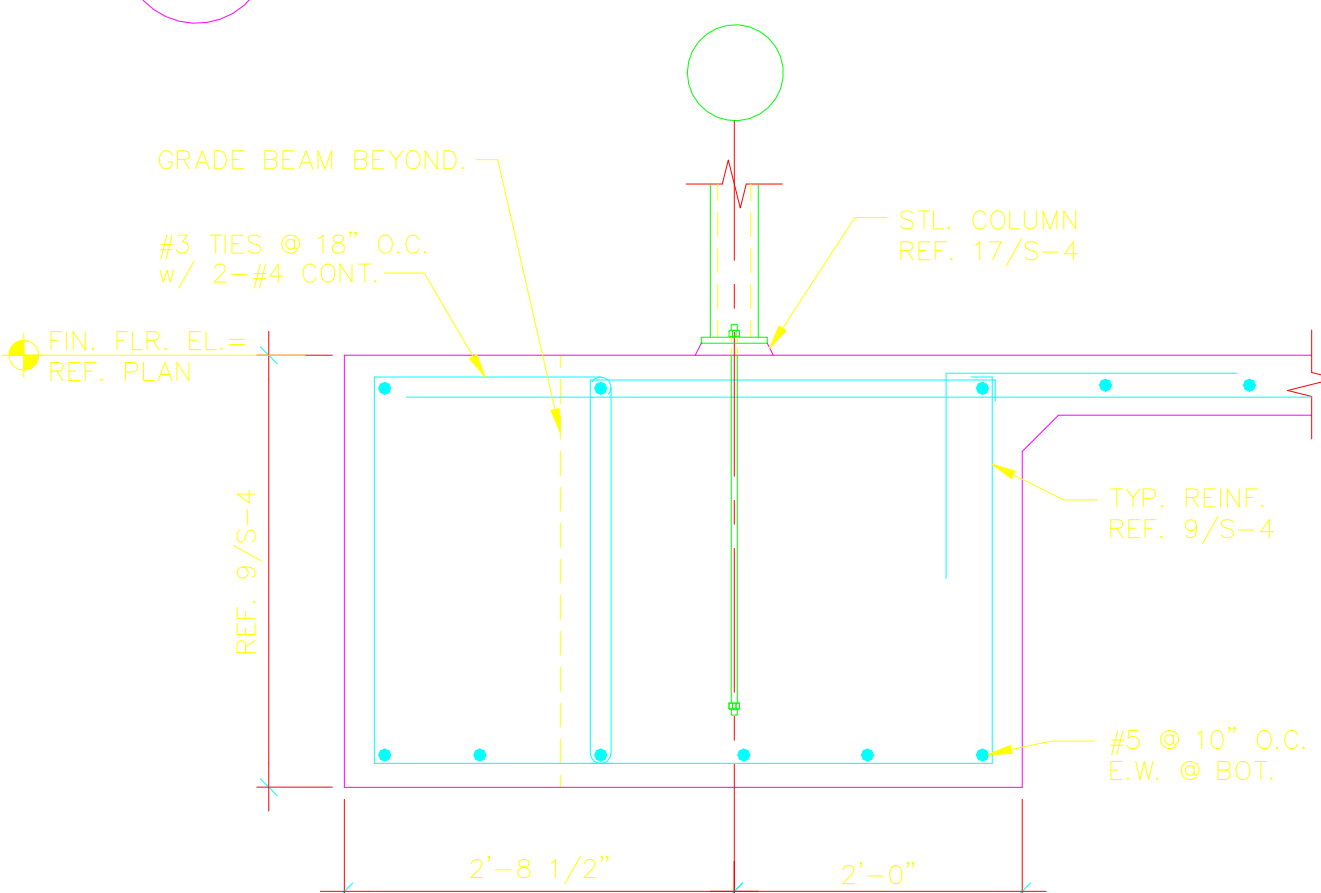
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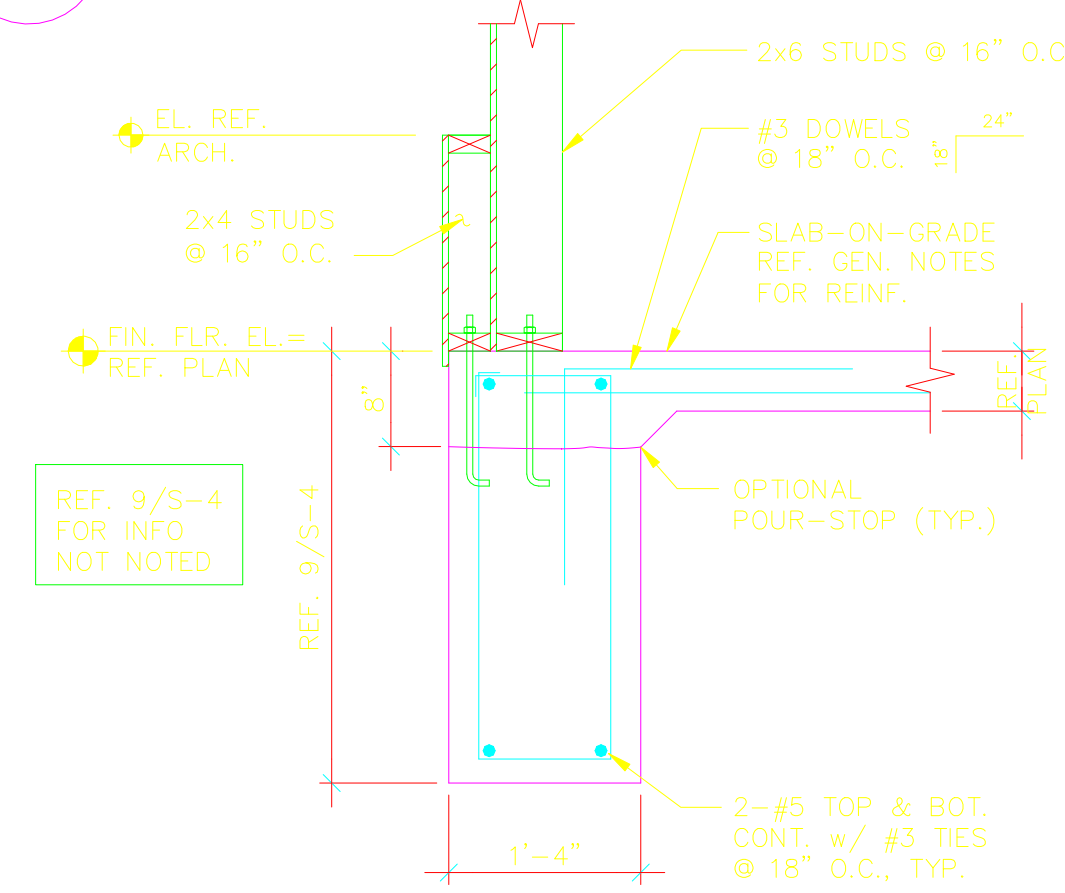
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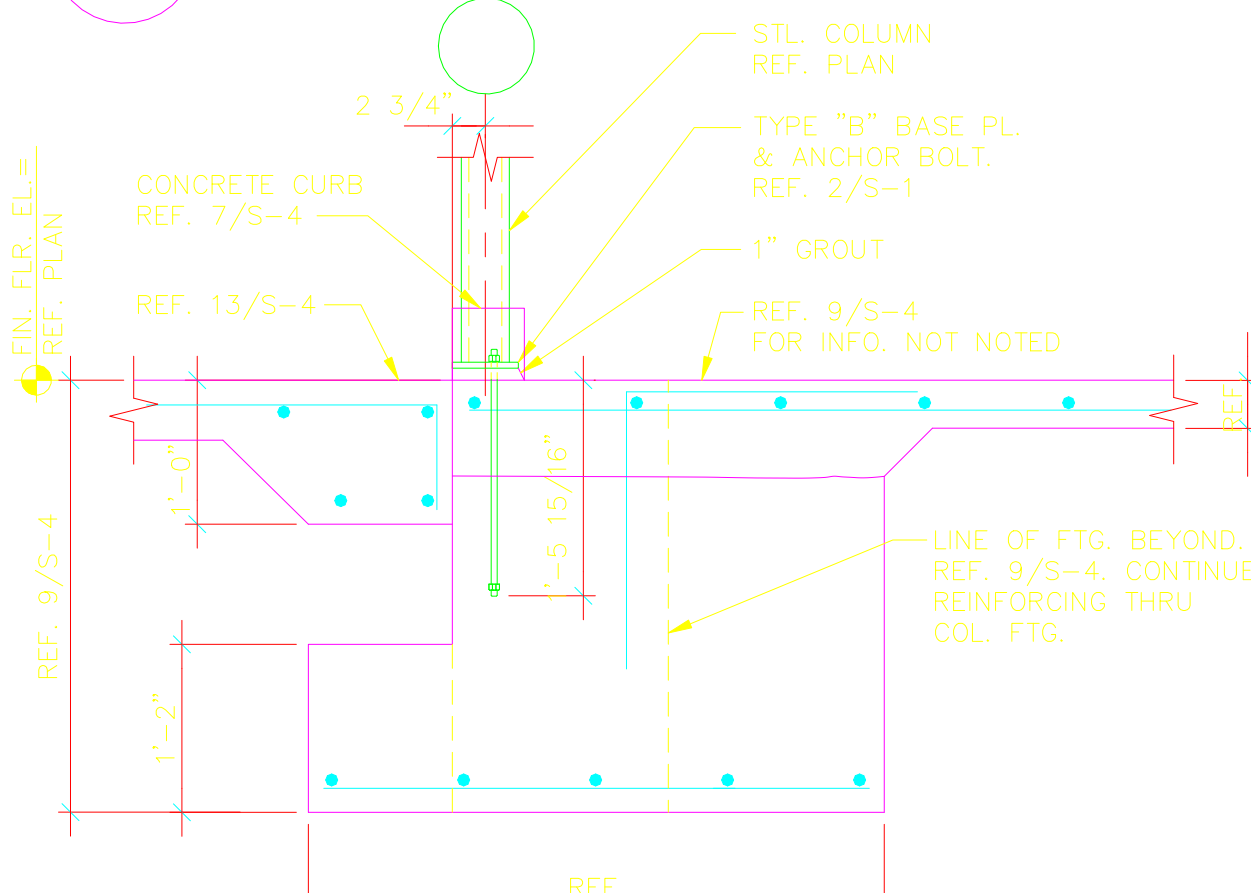
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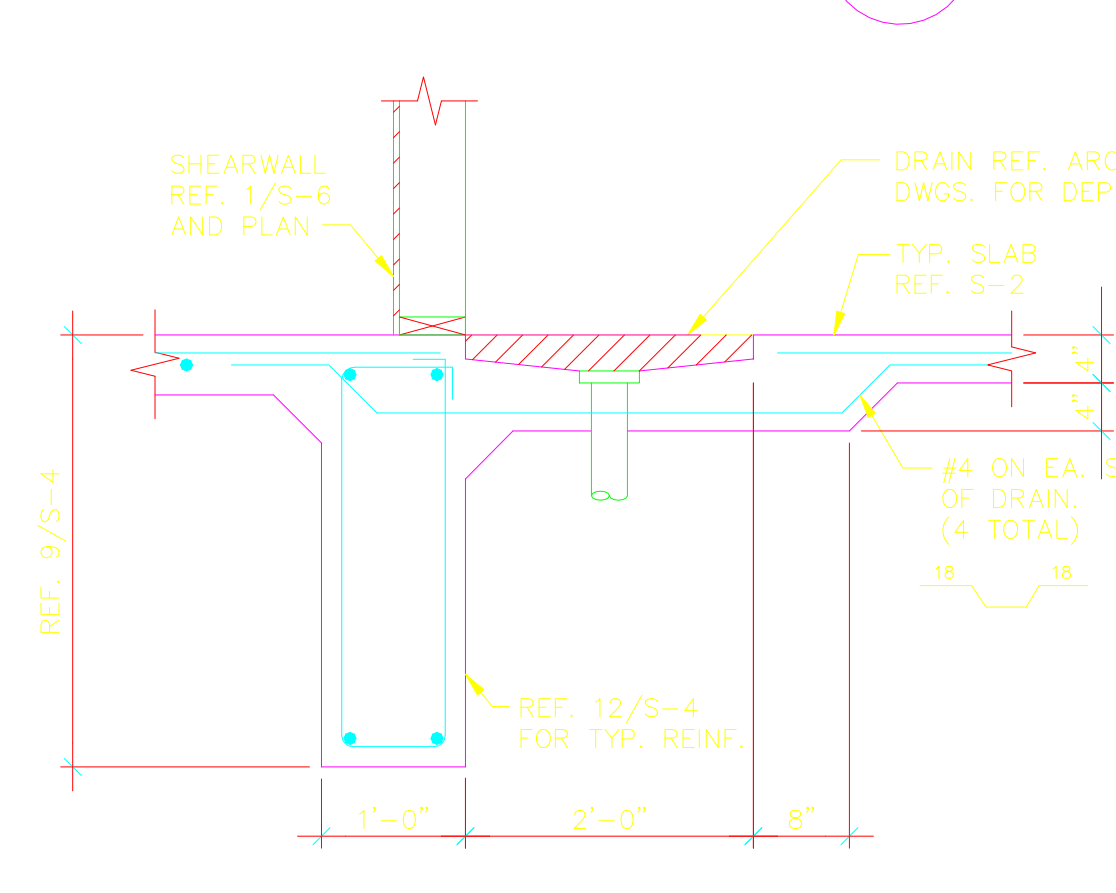
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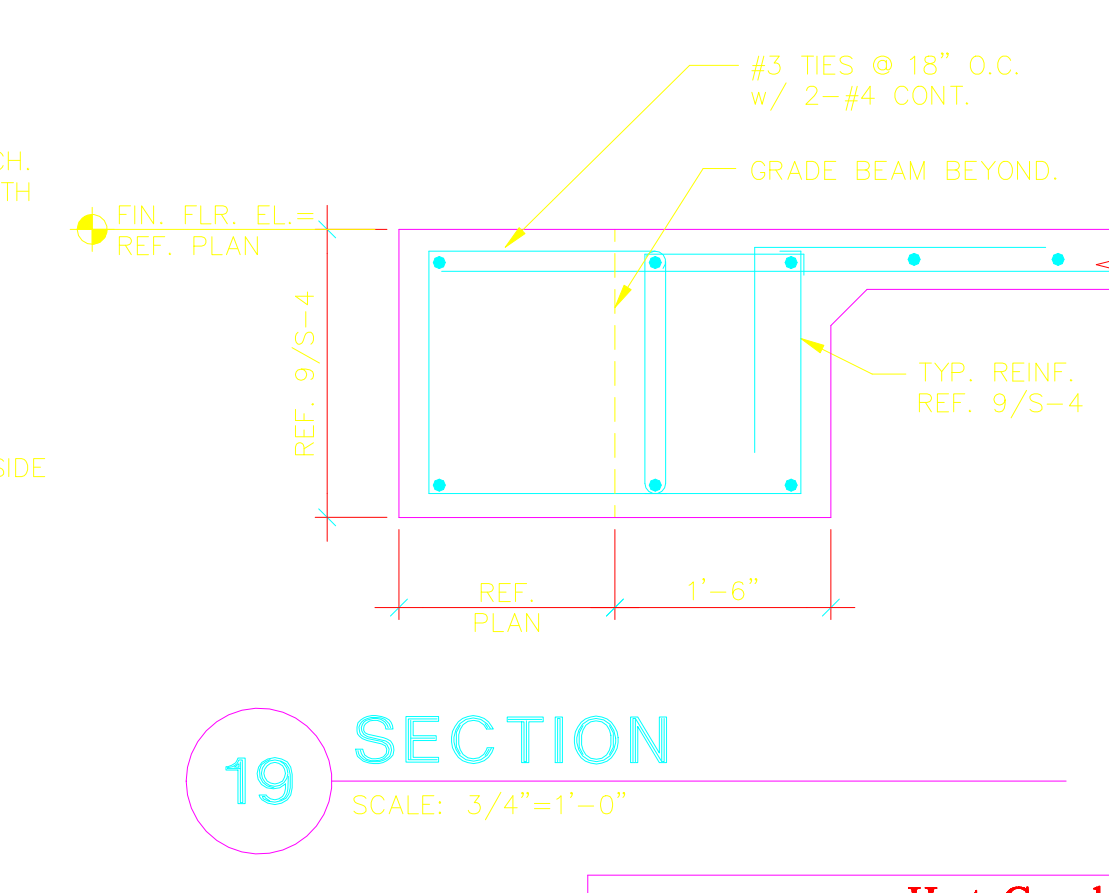
**16 SECTION**  
SCALE: 3/4"=1'-0"



**17 SECTION**  
SCALE: 3/4"=1'-0"



**18 RECESS AT MOP SINK**  
SCALE: 3/4"=1'-0"



**19 SECTION**  
SCALE: 3/4"=1'-0"

**HGA** Hart, Gauger & Associates, Inc.  
Structural Engineering Services  
12801 N. Central Expressway, #1400  
Dallas, TX 75243  
972-239-5111 / 972-239-5055 (Fax)

PERMIT ISSUE:

22 SEPT. 2003

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REVISIONS:

Δ	28 OCT. 2003	ADDENDUM #1
Δ	12 NOV. 2003	CITY COMMENTS
Δ	18 DEC. 2003	CITY COMMENTS
Δ	28 JAN. 2004	OWNER COMMENTS

PROTO 90  
HWY 287 AT PEARL RD.  
CASA GRANDE, AZ

Johnny  
**Carino's**  
COUNTRY ITALIAN

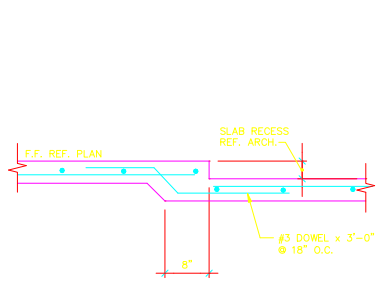
nca Architecture

6500 Greenville Ave  
Suite 307  
Dallas, TX 75206  
214.361.9901  
Fax 361.9906

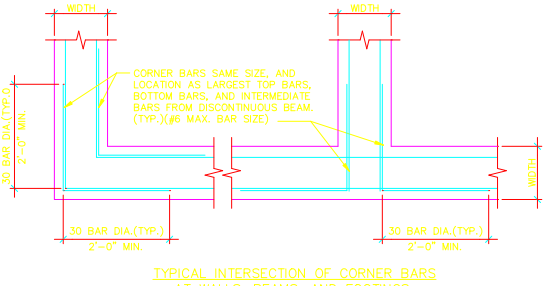
SHEET NO:

S-4

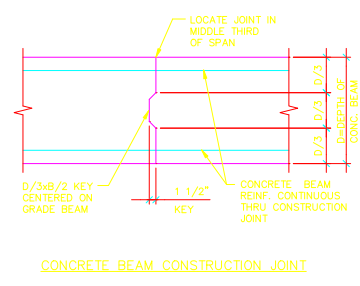
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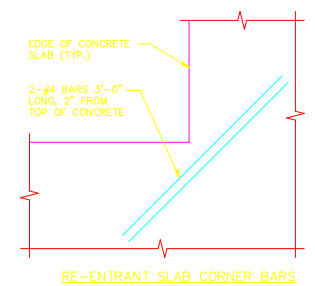
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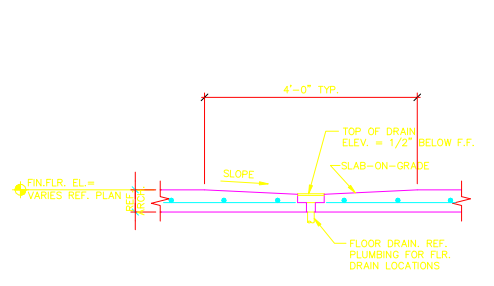
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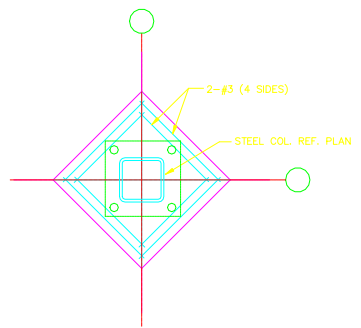
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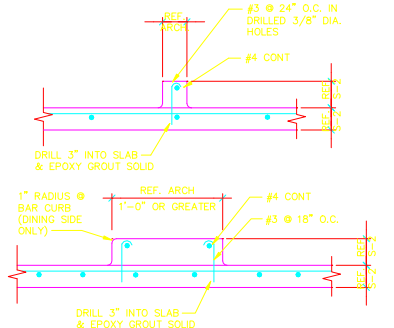
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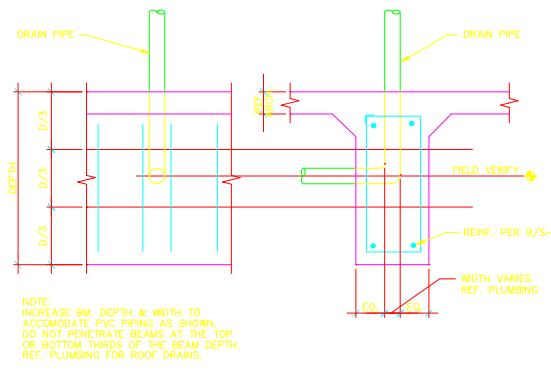
**5 TYPICAL DRAIN DETAIL**  
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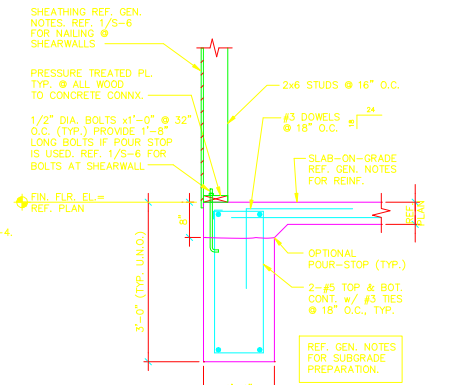
**6 TYP. SLAB LEAVE-OUT AT INTERIOR COLUMN**  
SCALE: NO SCALE



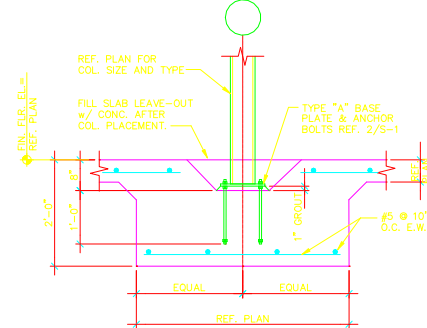
**7 TYPICAL CURB DETAIL**  
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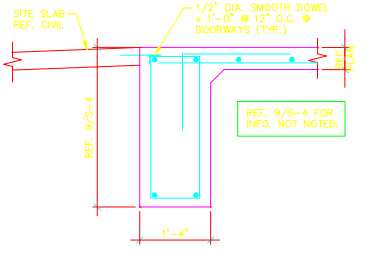
**8 TYP. GRADE BM. PENETRATION**  
SCALE: NO SCALE



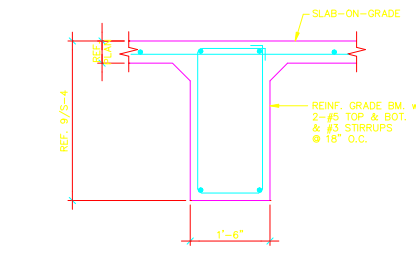
**9 SECTION**  
SCALE: 3/4"=1'-0"



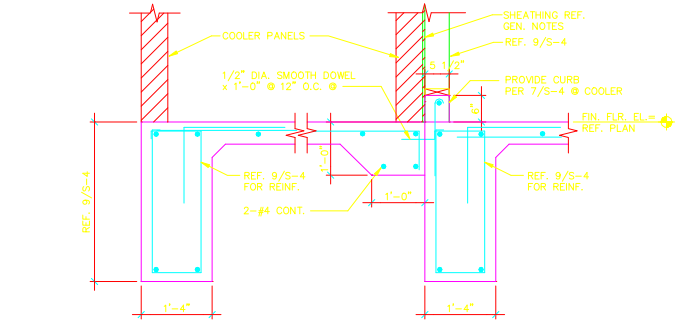
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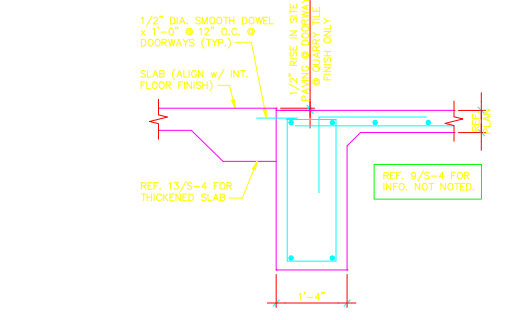
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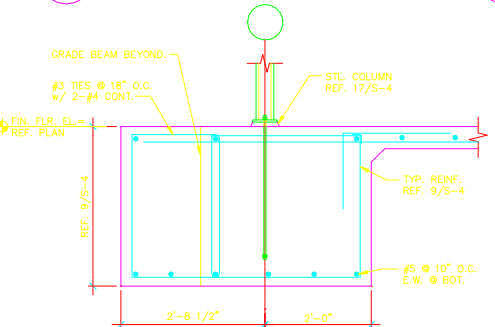
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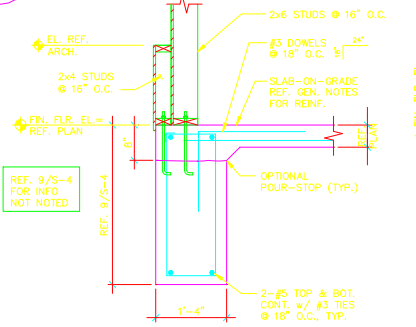
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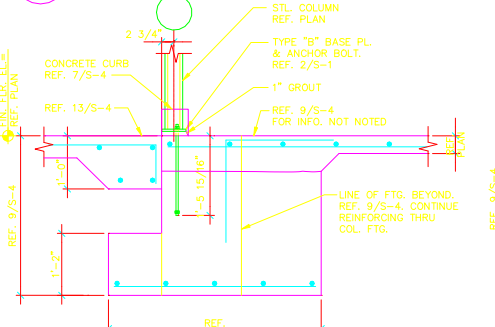
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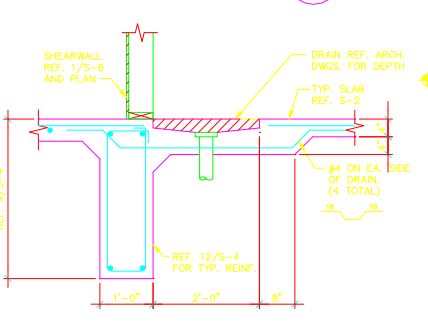
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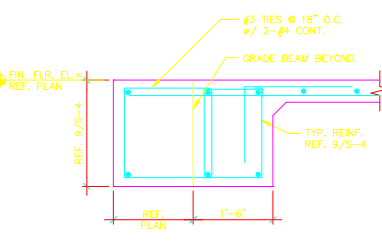
**16 SECTION**  
SCALE: 3/4"=1'-0"



**17 SECTION**  
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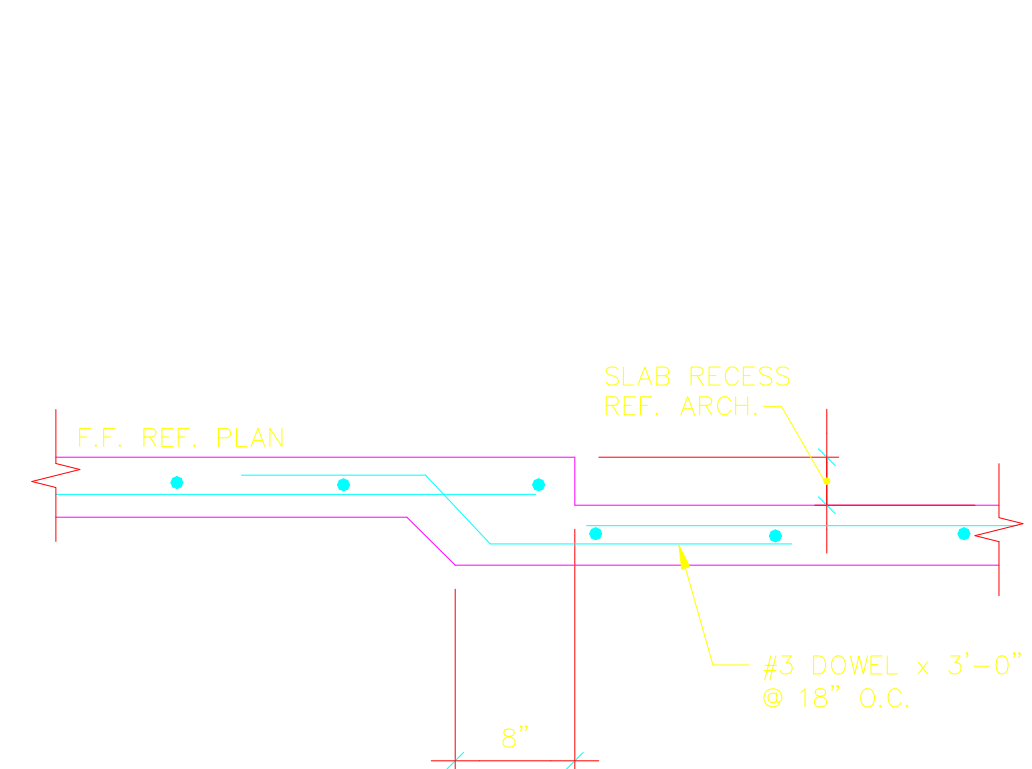


**18 RECESS AT MOP SINK**  
SCALE: 3/4"=1'-0"

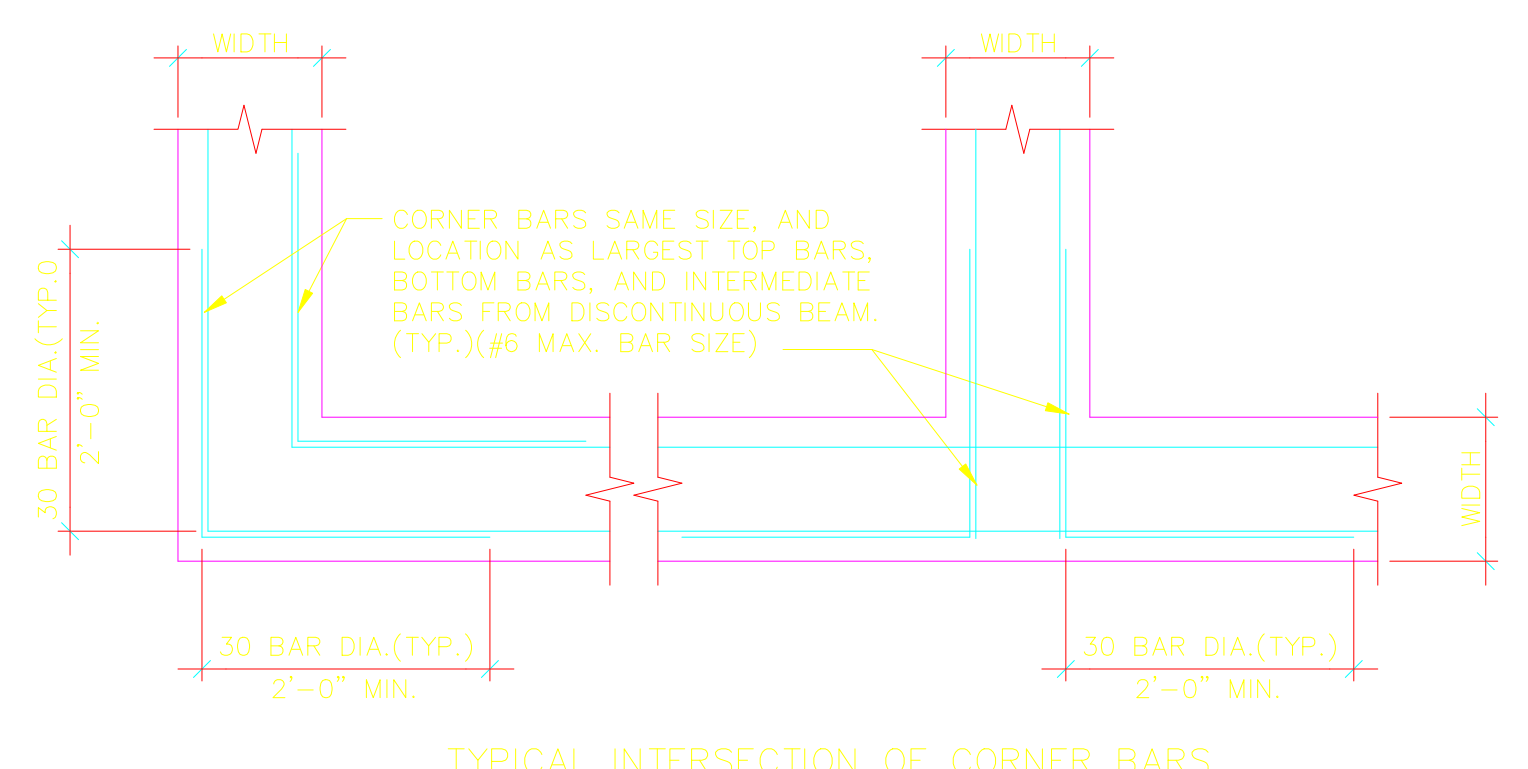


**19 SECTION**  
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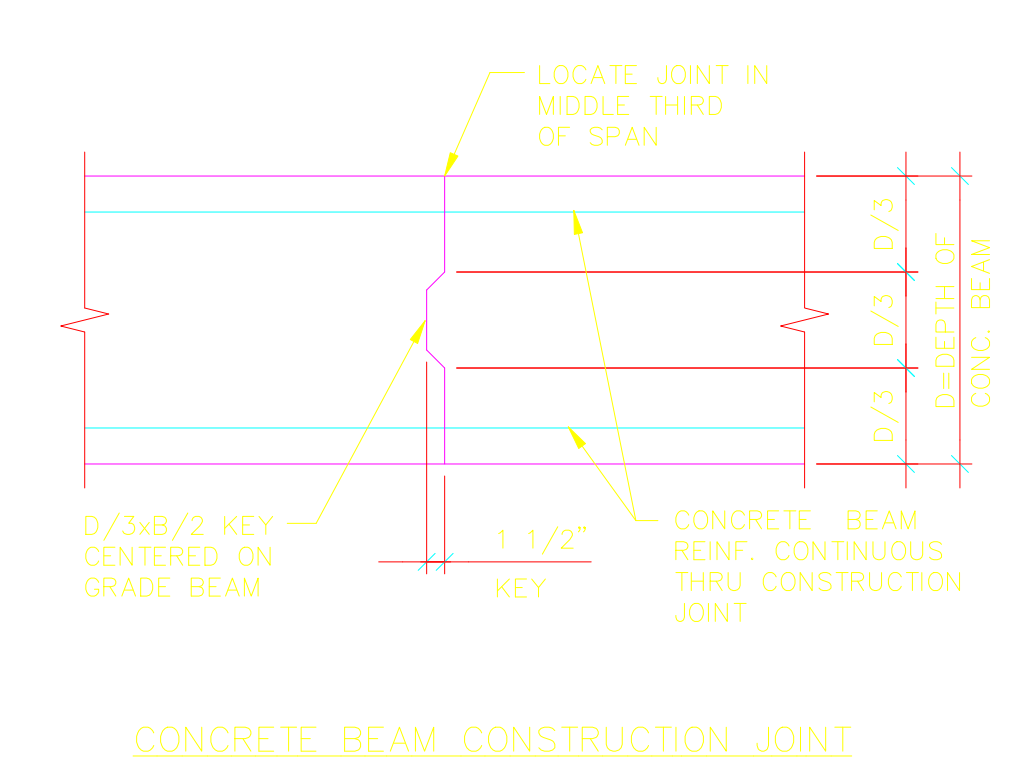




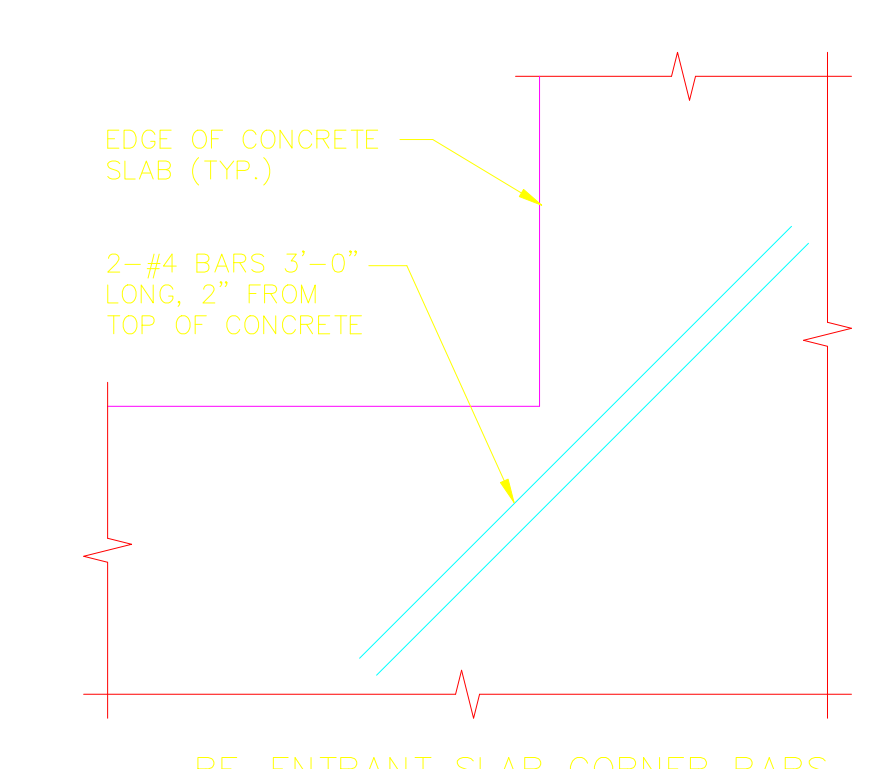
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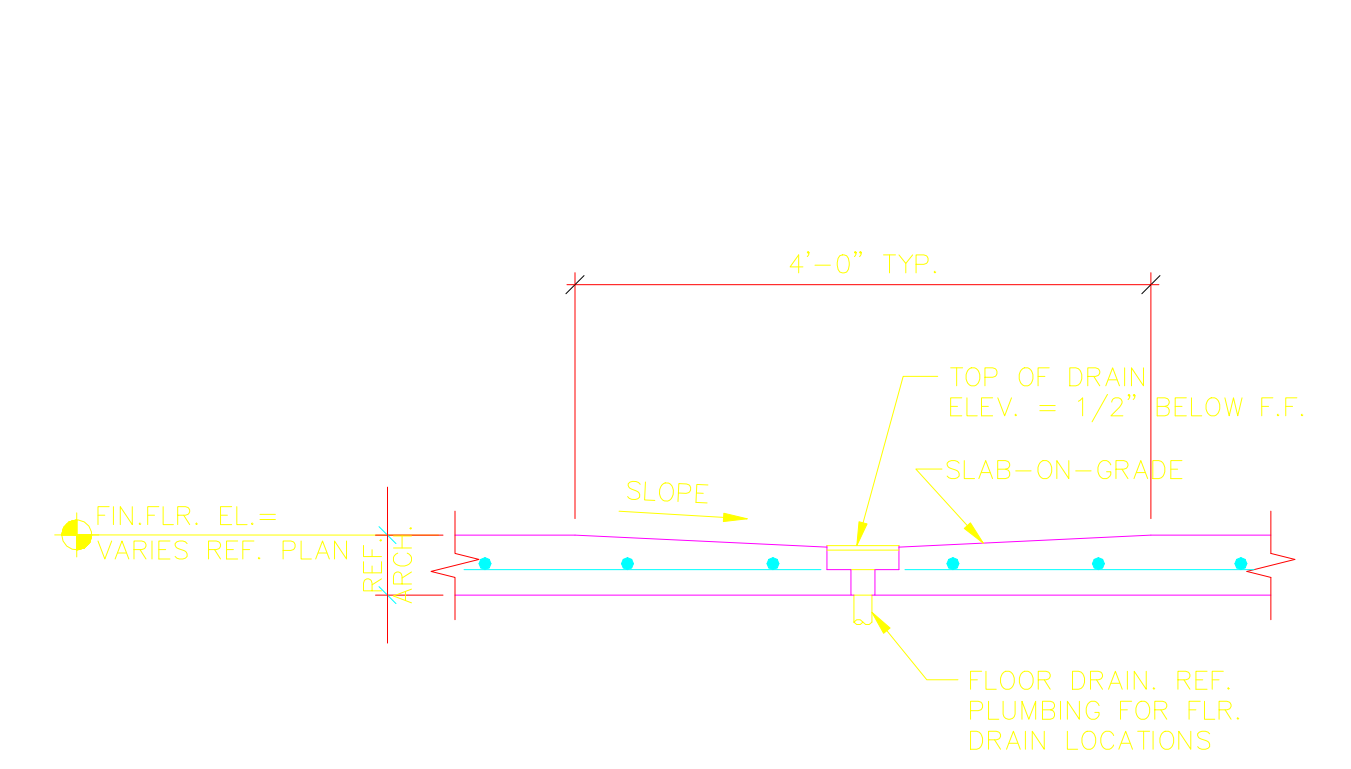
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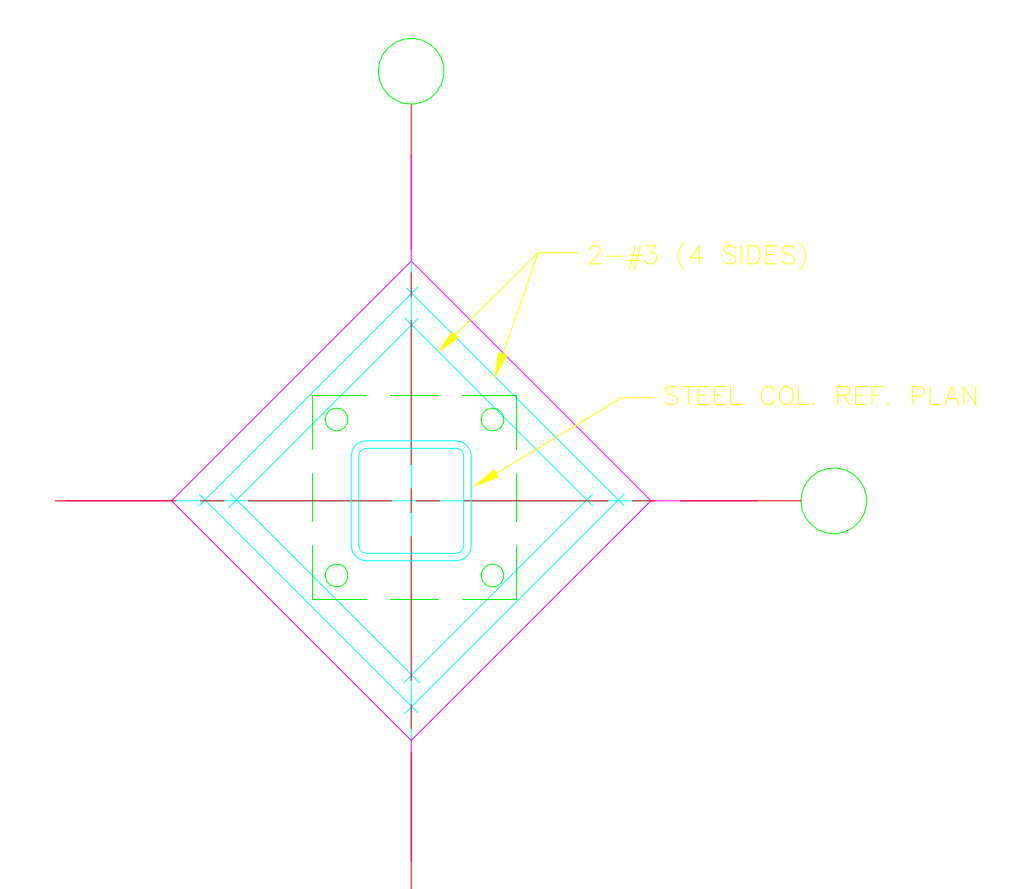
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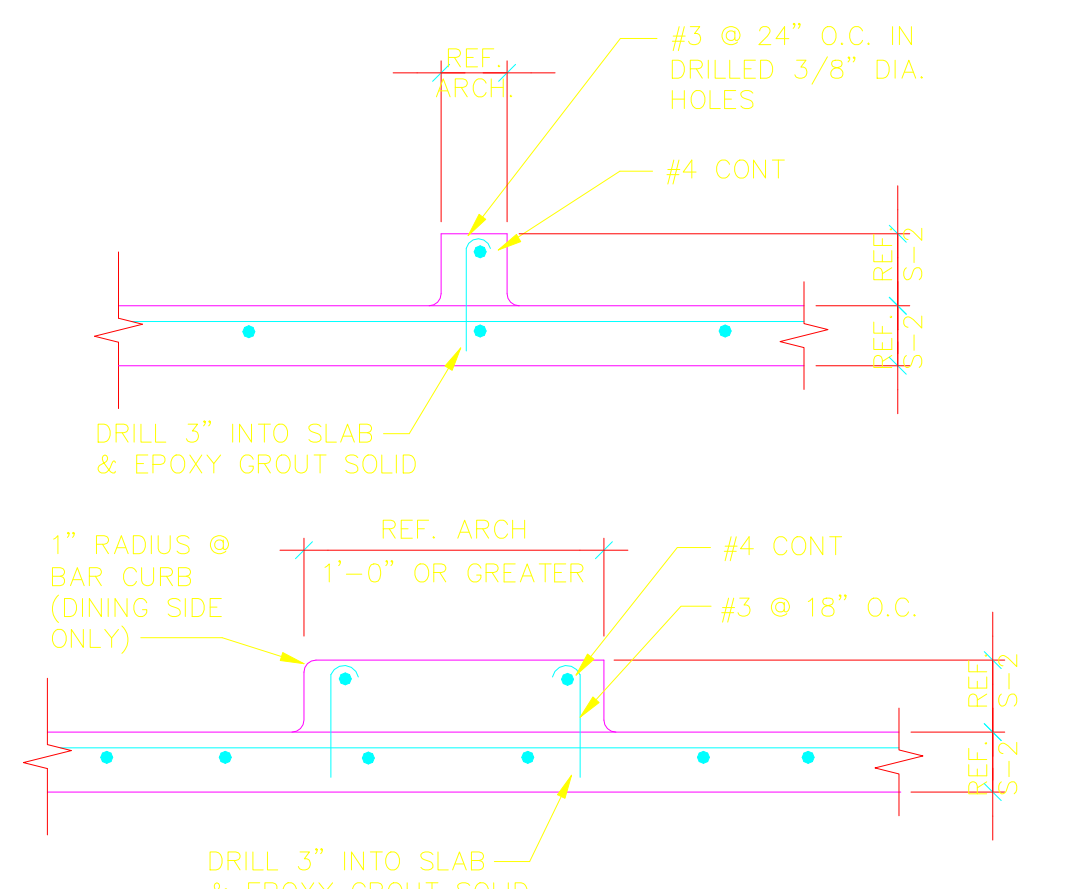
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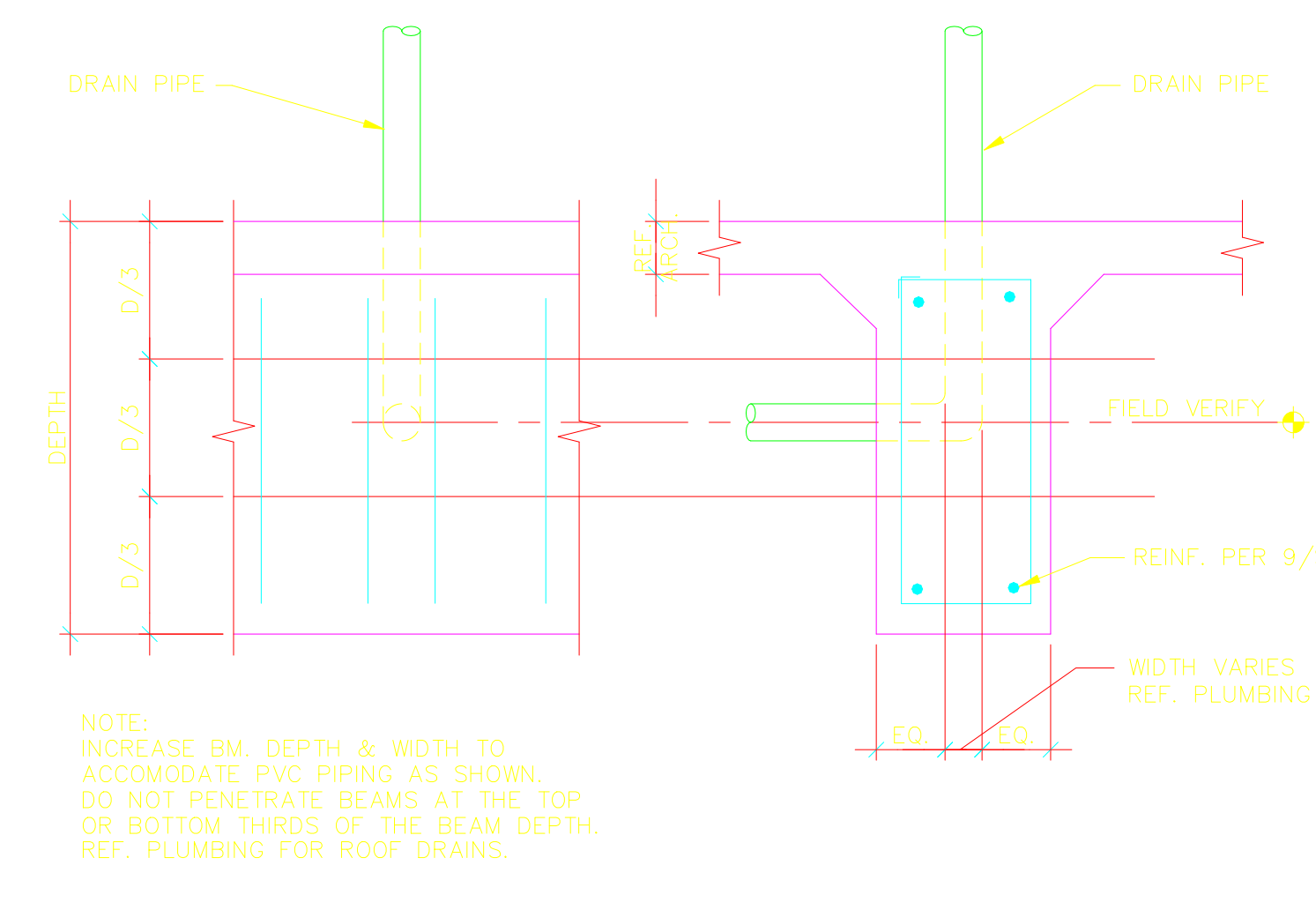
**5 TYPICAL DRAIN DETAIL**  
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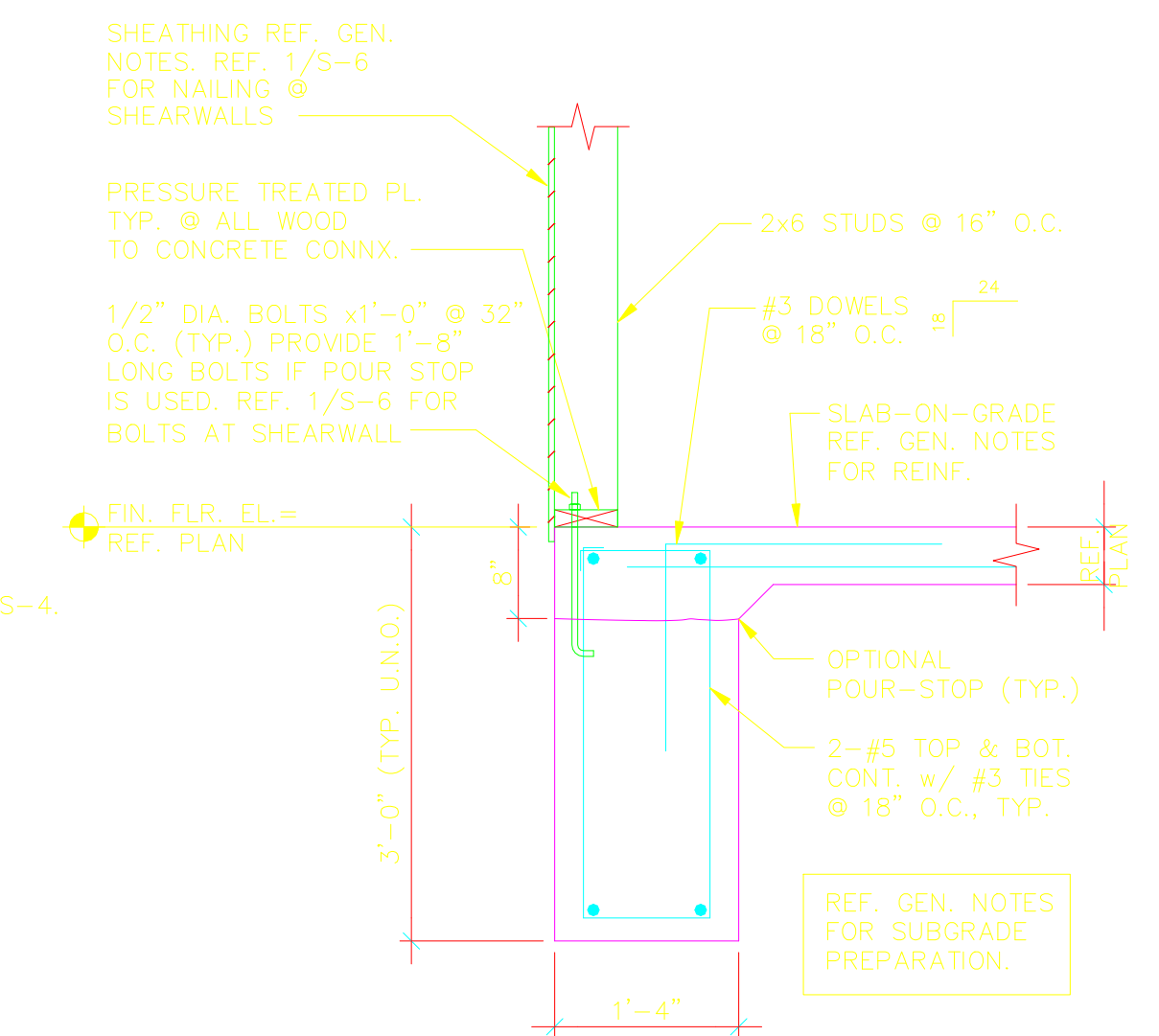
**6 TYP. SLAB LEAVE-OUT AT INTERIOR COLUMN**  
SCALE: NO SCALE



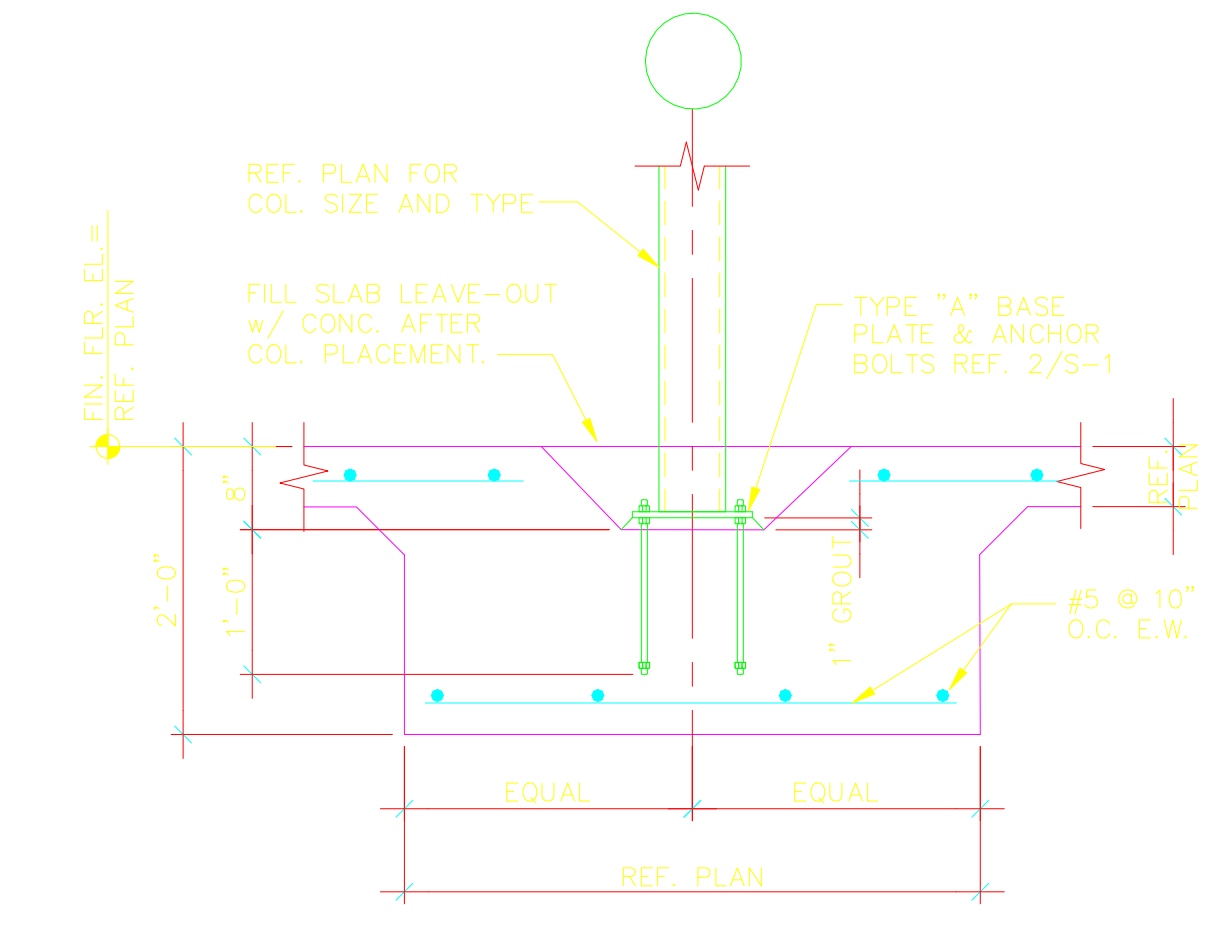
**7 TYPICAL CURB DETAIL**  
SCALE: NO SCALE



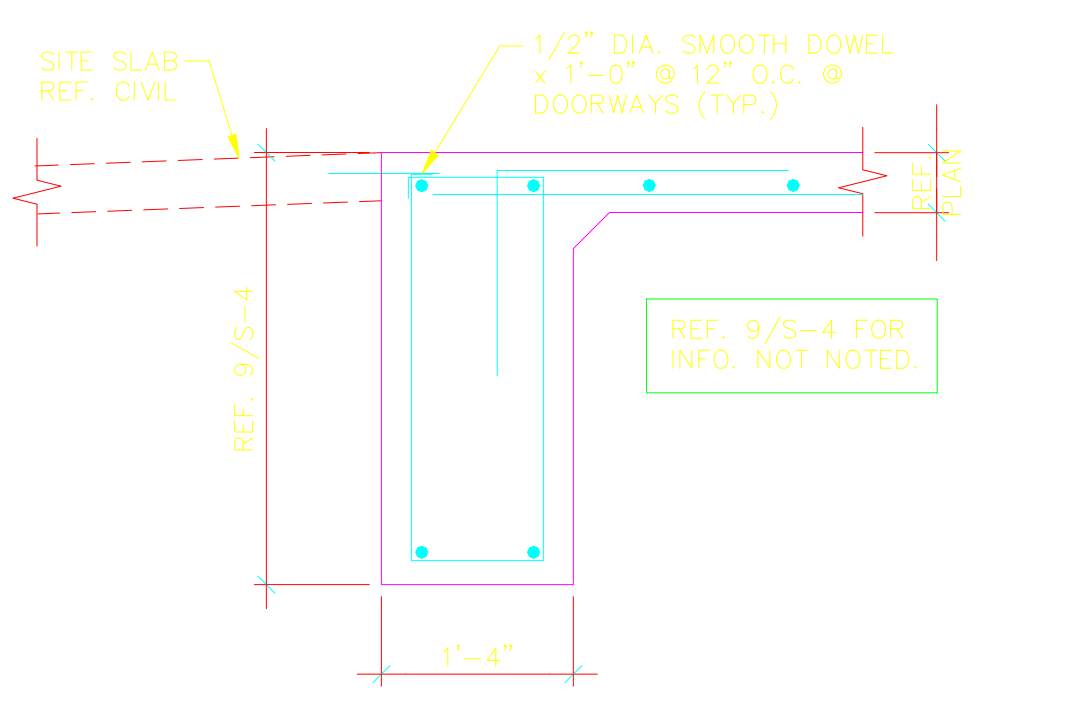
**8 TYP. GRADE BM. PENETRATION**  
SCALE: NO SCALE



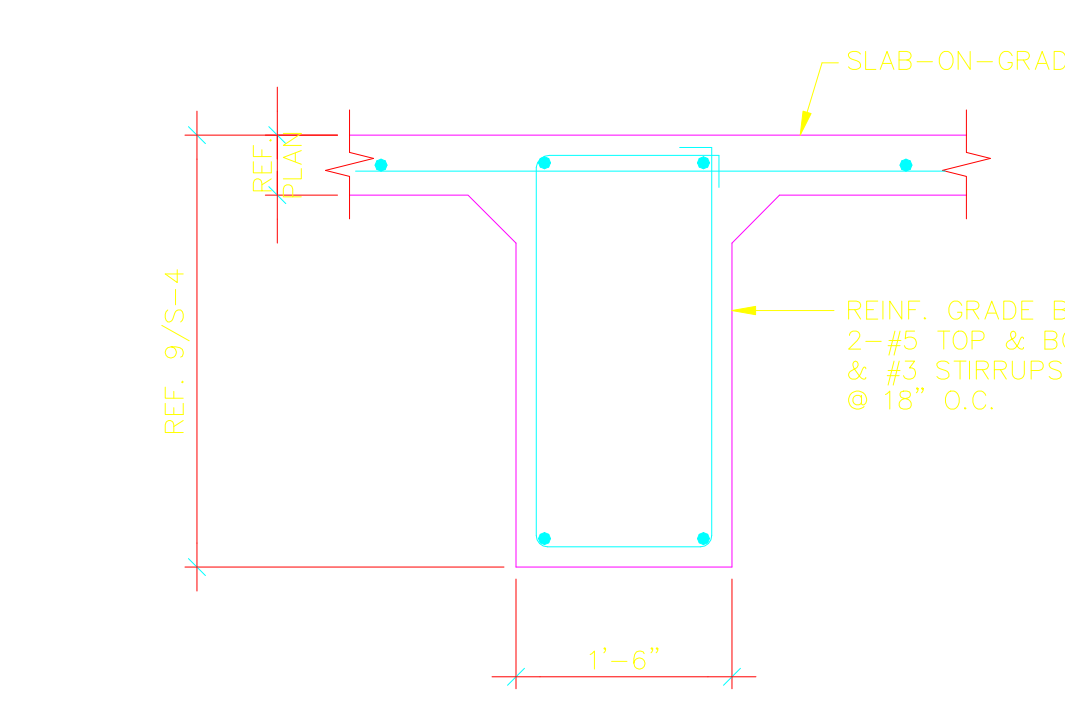
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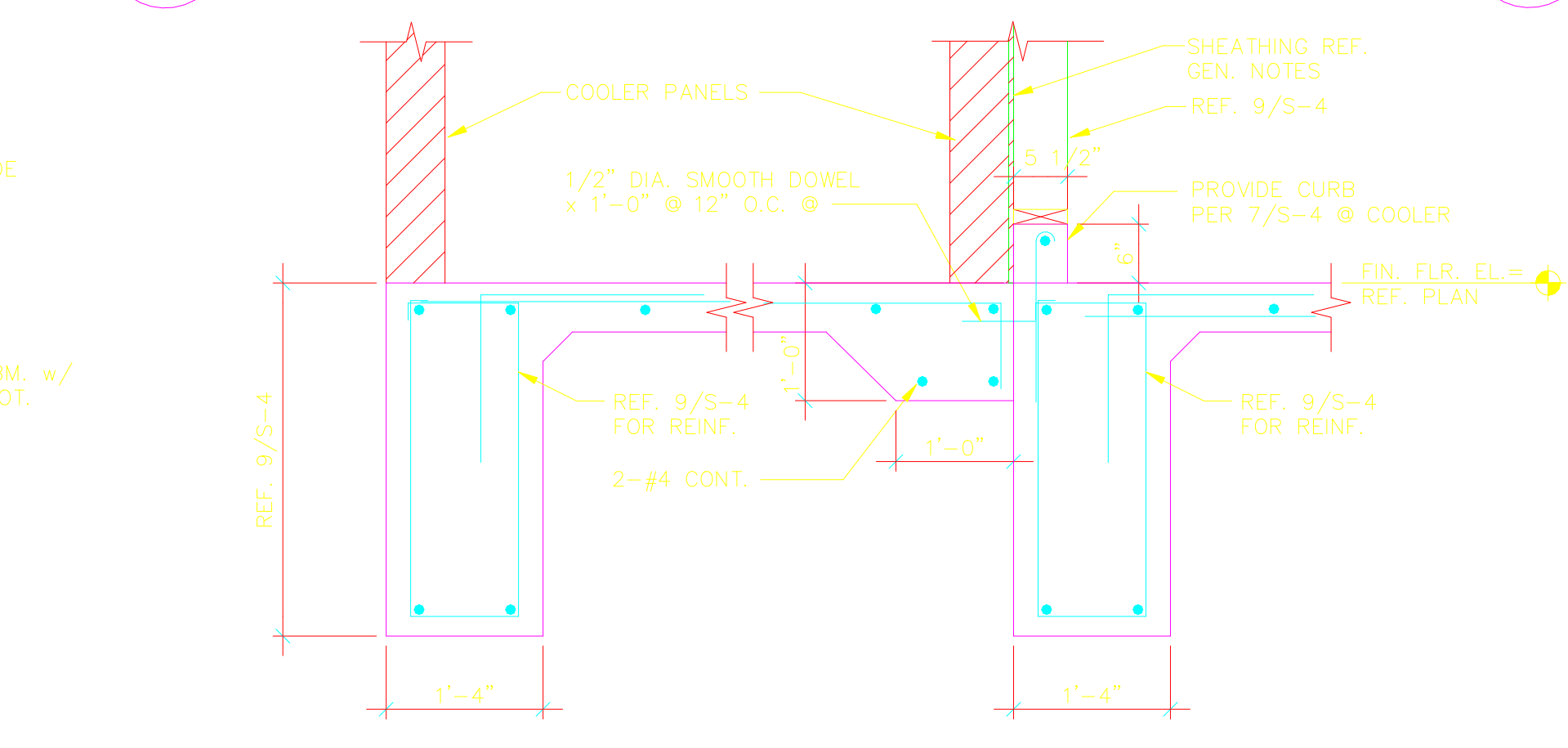
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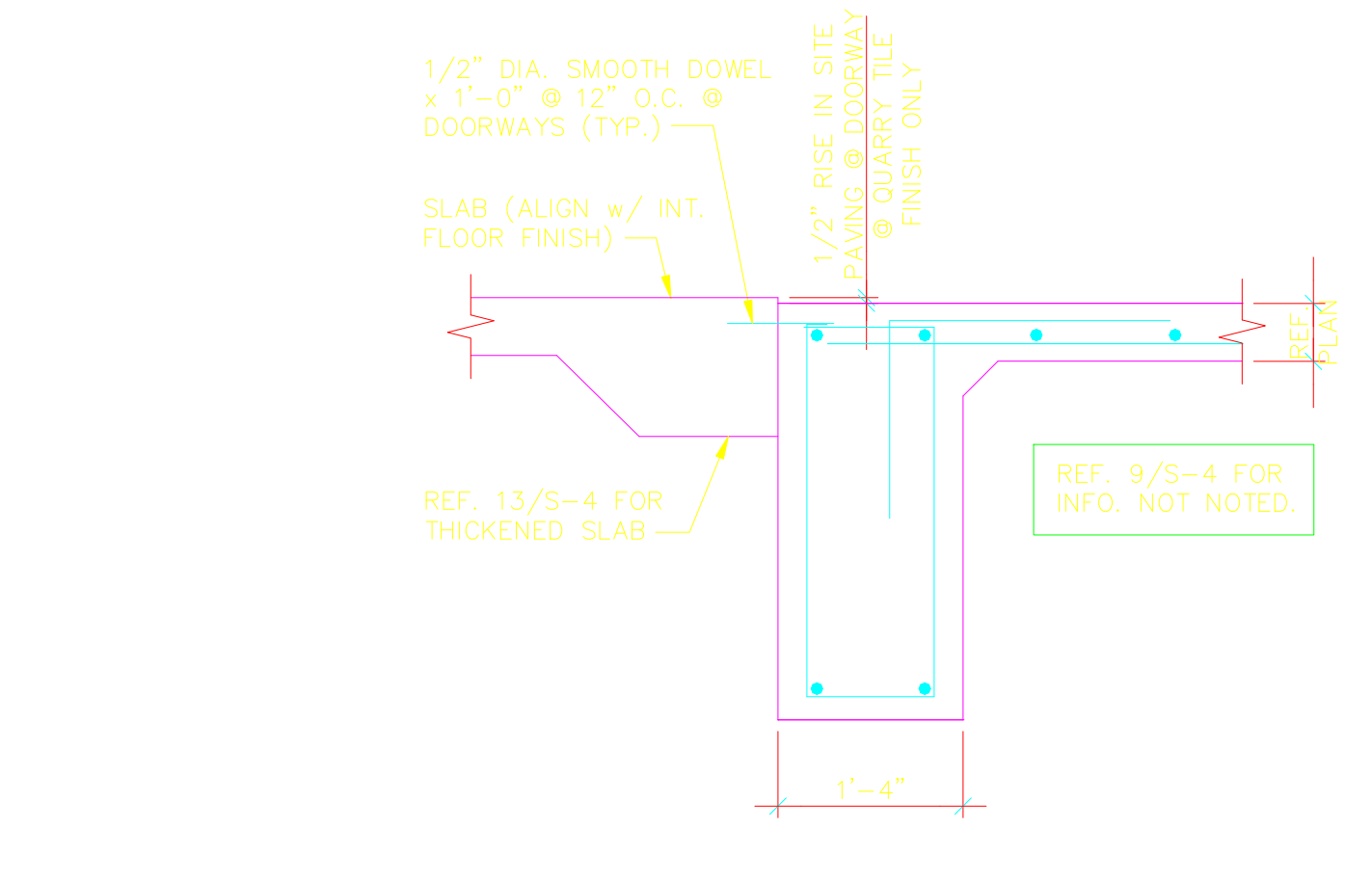
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SCALE: 3/4"=1'-0"



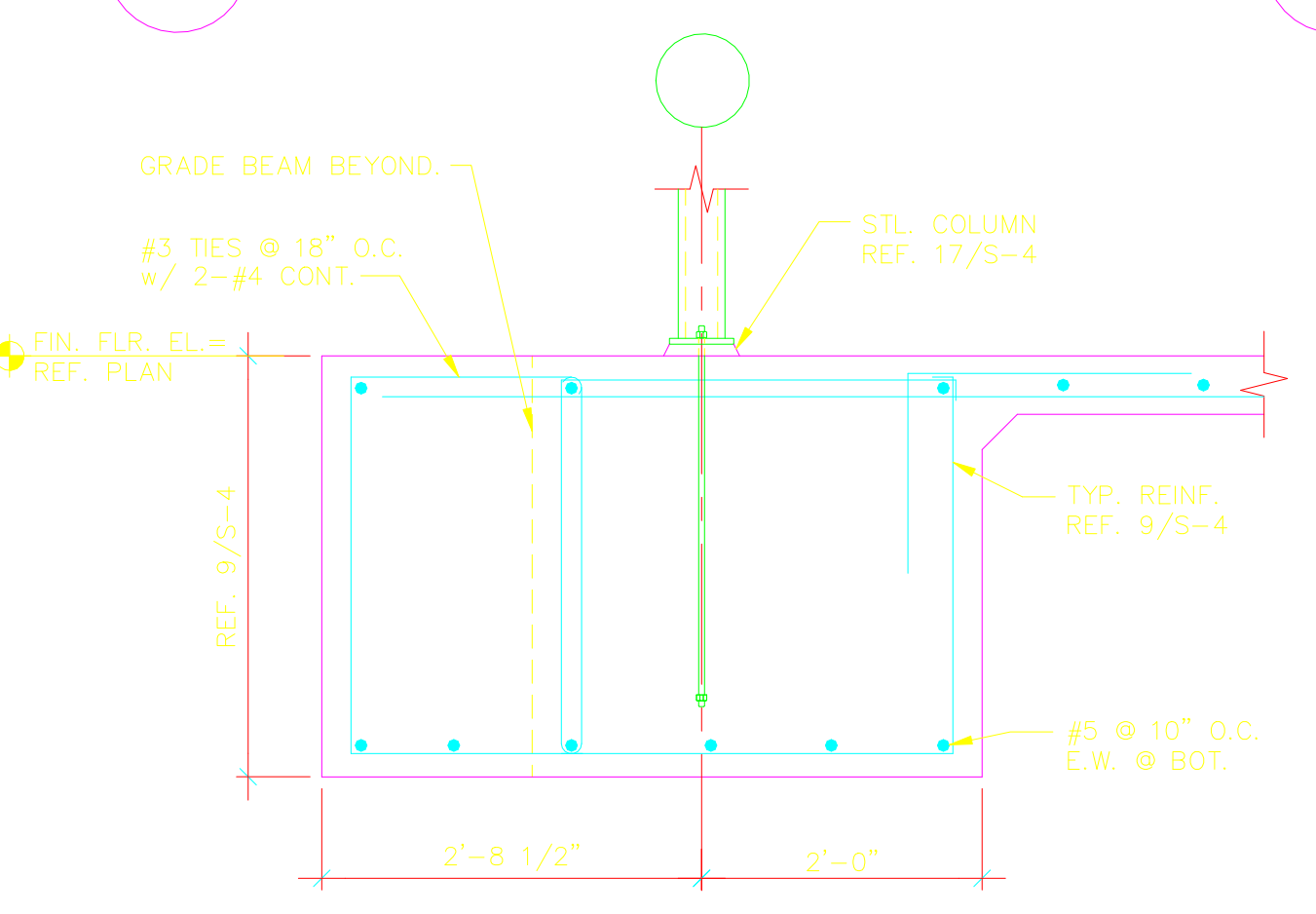
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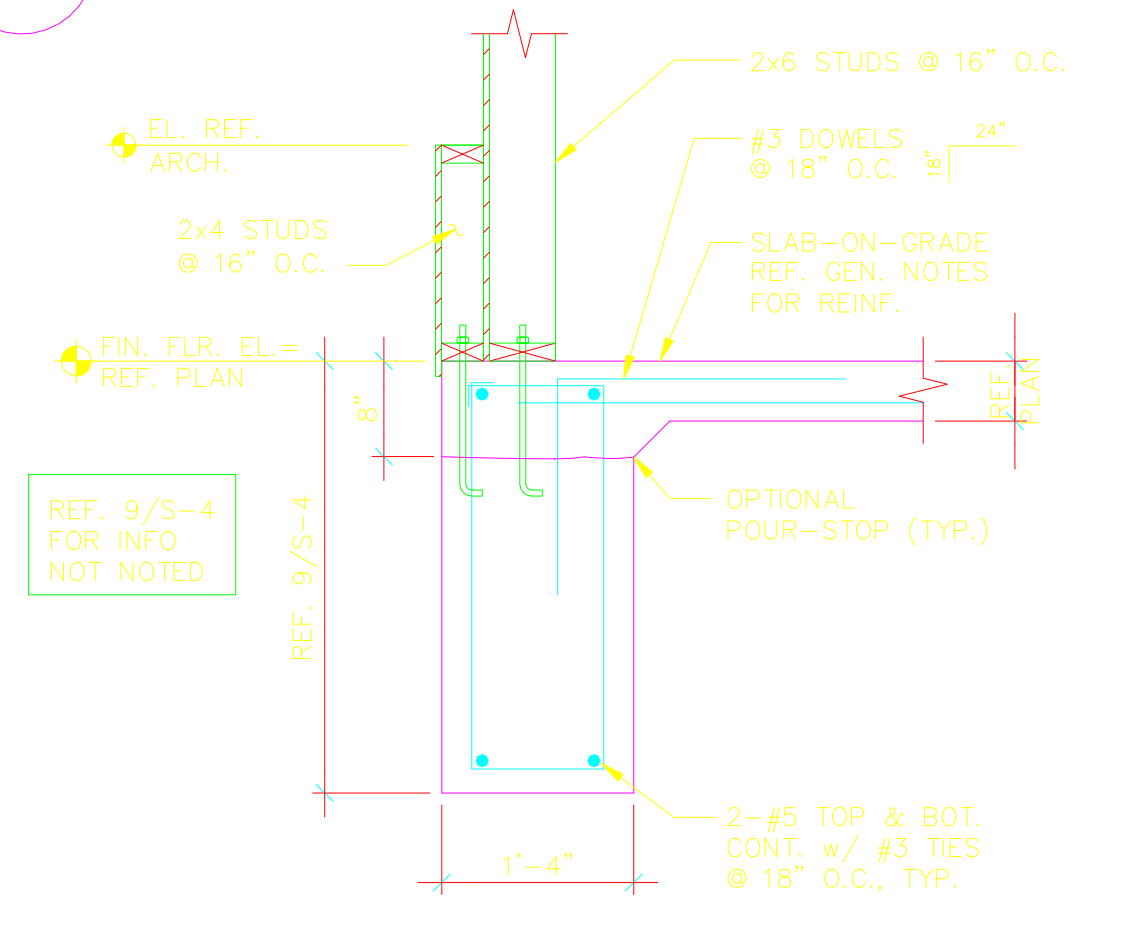
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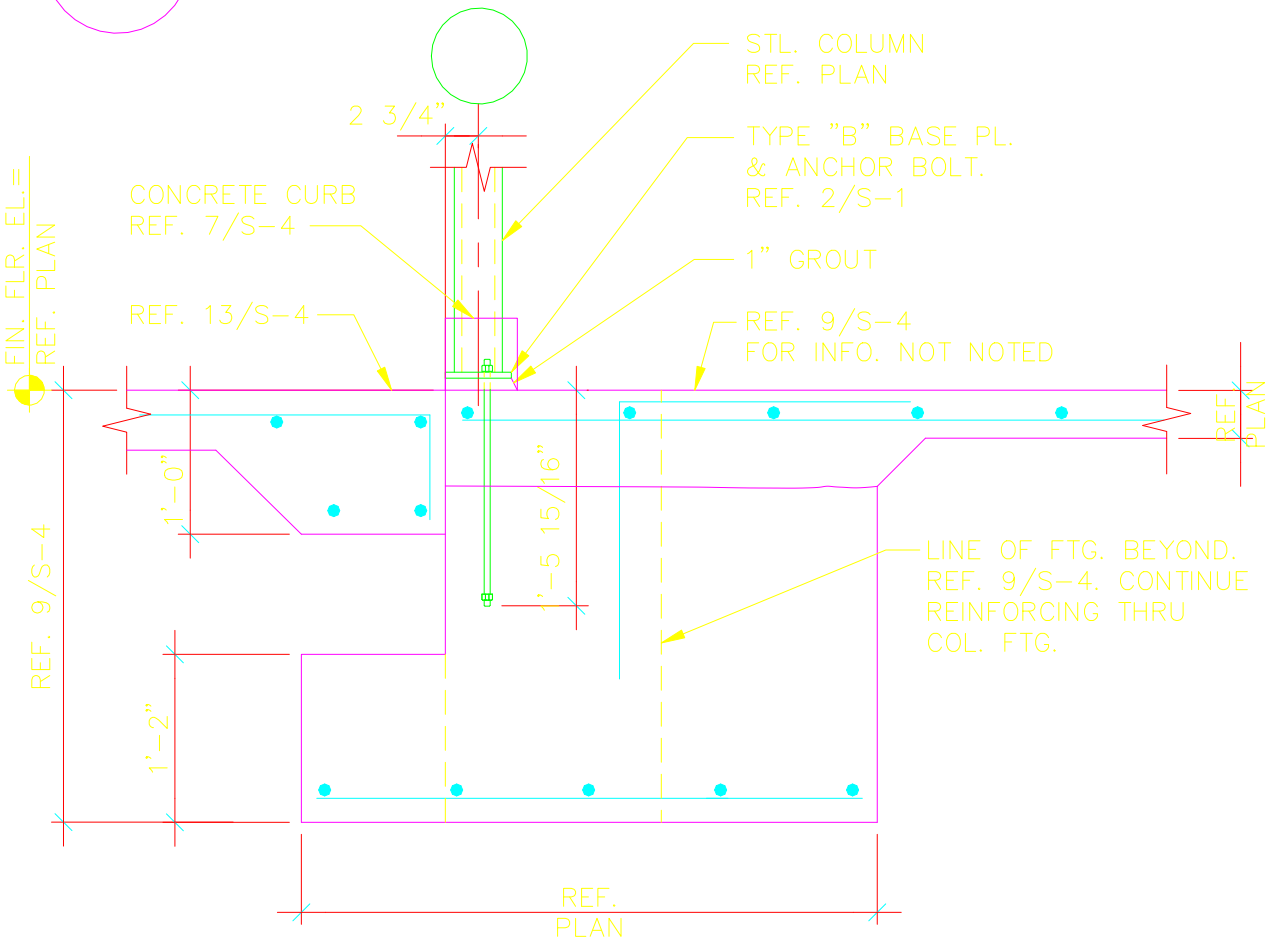
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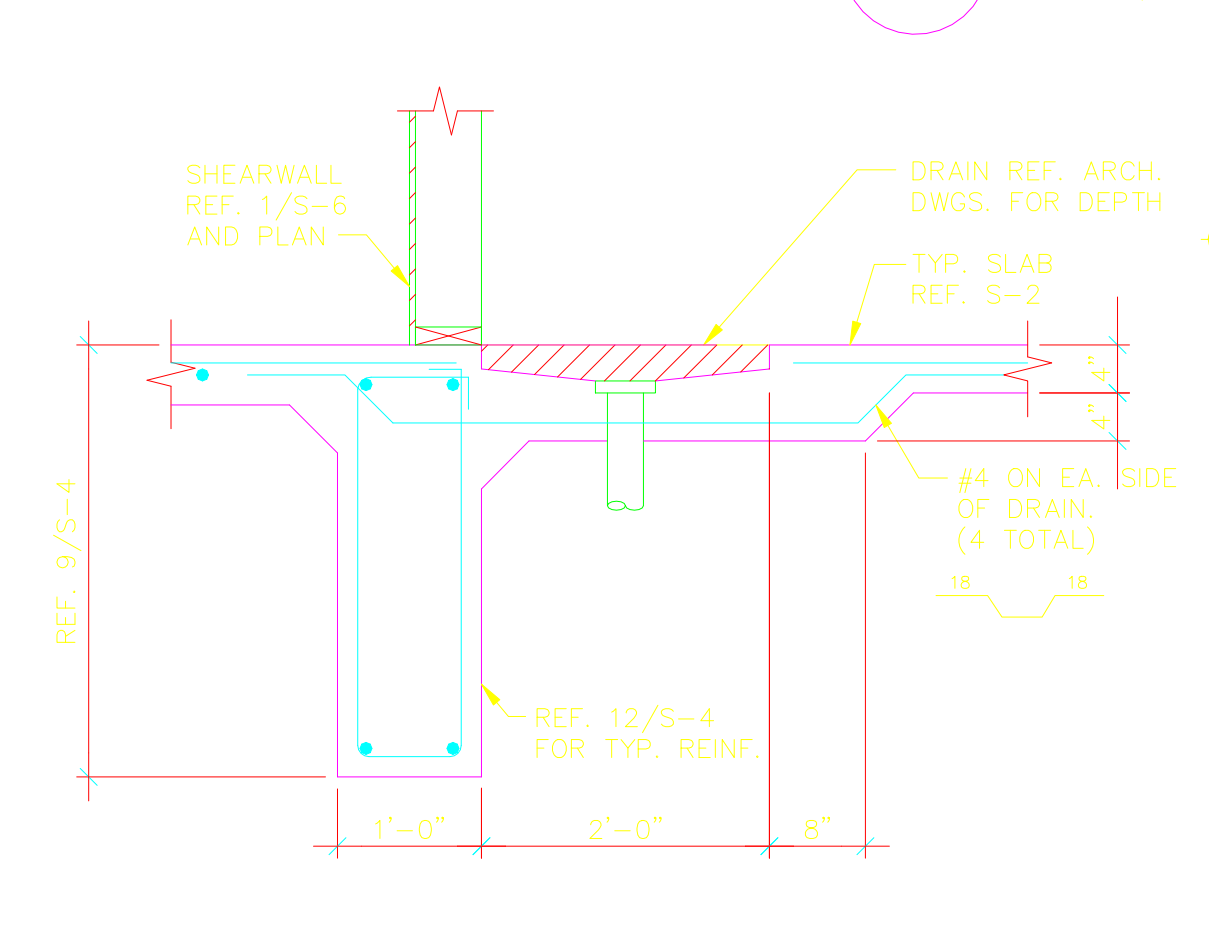
**15 SECTION**  
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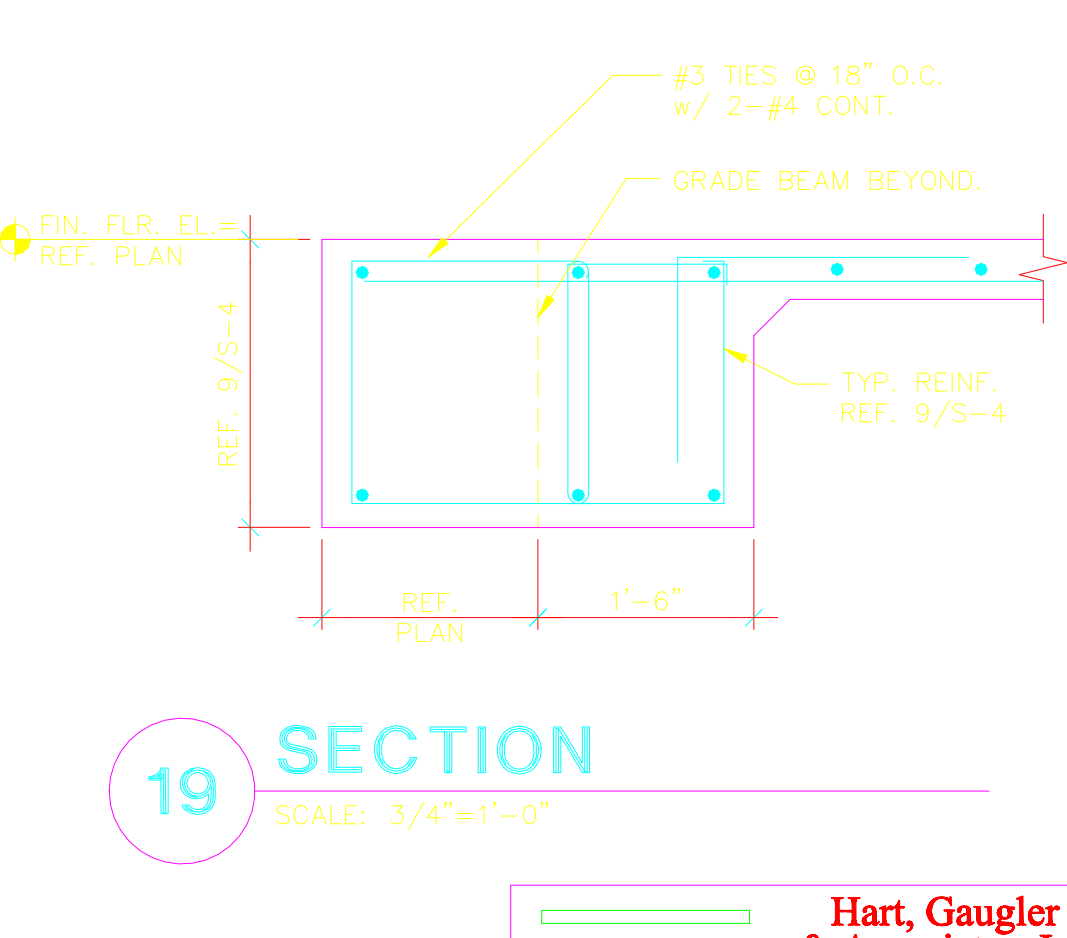
**16 SECTION**  
SCALE: 3/4"=1'-0"



**17 SECTION**  
SCALE: 3/4"=1'-0"



**18 RECESS AT MOP SINK**  
SCALE: 3/4"=1'-0"



**19 SECTION**  
SCALE: 3/4"=1'-0"

**HGA** Hart, Gauger & Associates, Inc.  
Structural Engineering Services  
12801 N. Central Expressway, #1400  
Dallas, TX 75243  
972-239-5111 / 972-239-5055 (Fax)

PERMIT ISSUE:

22 SEPT. 2003

REVISIONS:

28 OCT. 2003 ADDENDUM #1

12 NOV. 2003 CITY COMMENTS

18 DEC. 2003 CITY COMMENTS

28 JAN. 2004 OWNER COMMENTS

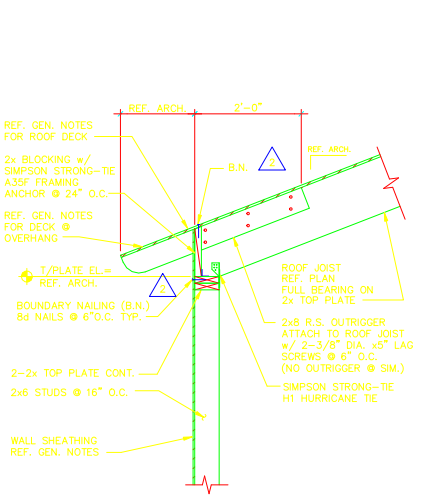
PROTO 90  
HWY 287 AT PEARL RD.  
CASA GRANDE, AZ



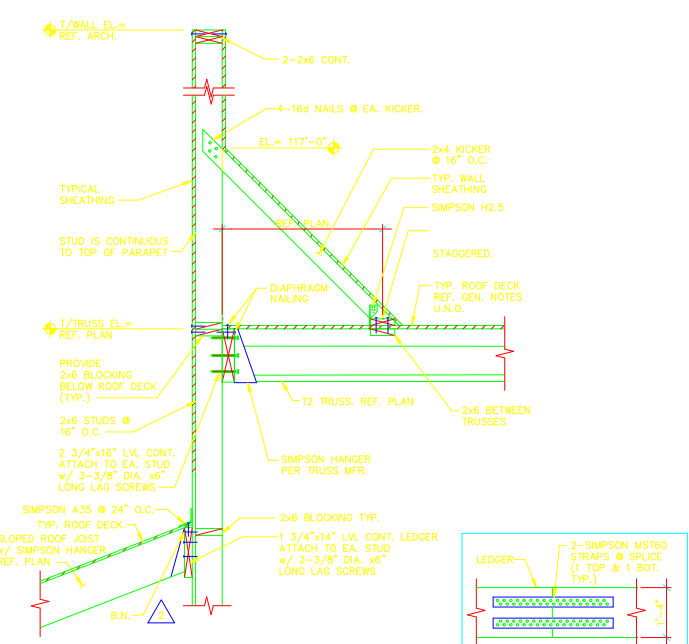
nca Architecture

6500 Greenville Ave  
Suite 307  
Dallas, TX 75206  
214.361.9901  
Fax 361.9906

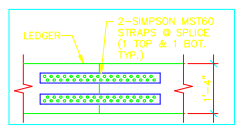
SHEET NO. S-4



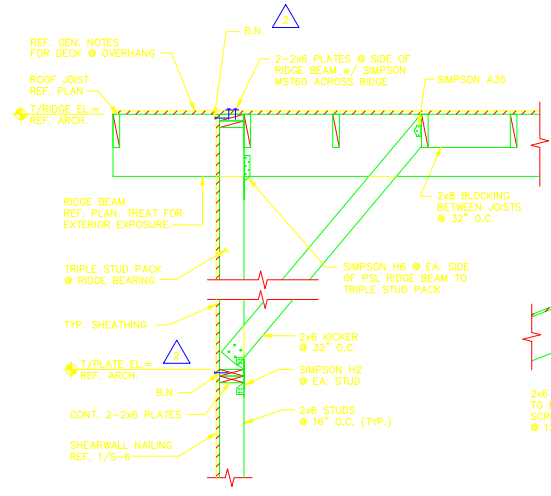
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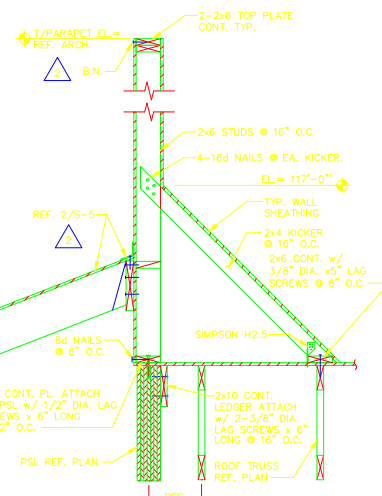
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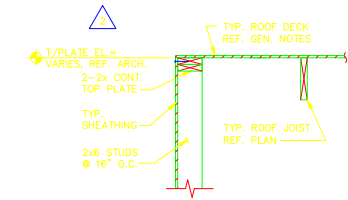
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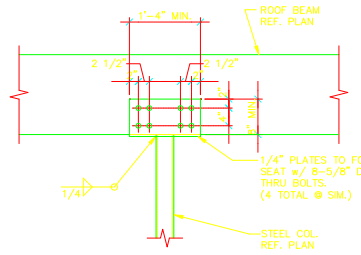
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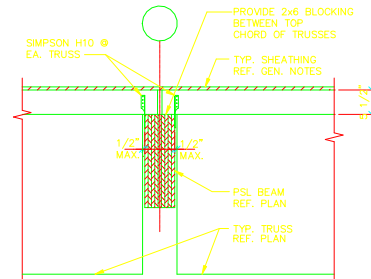
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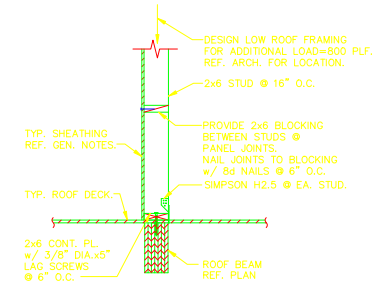
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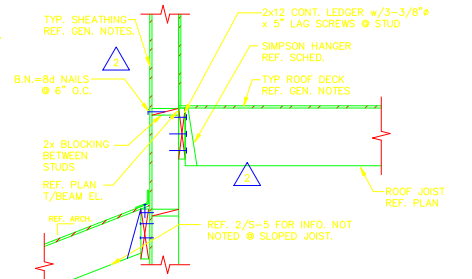
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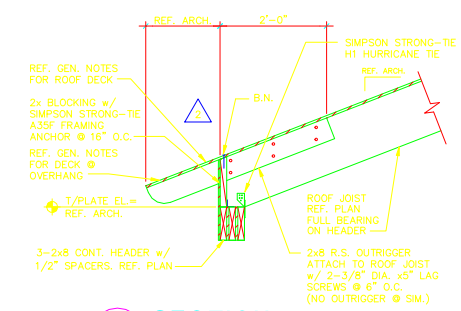
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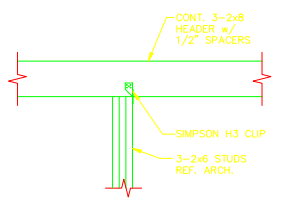
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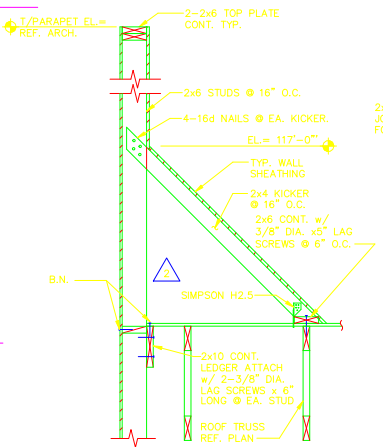
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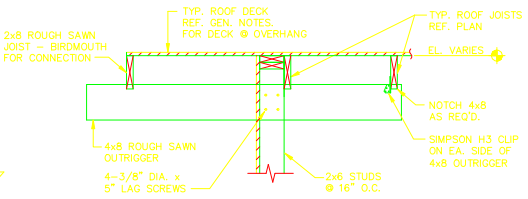
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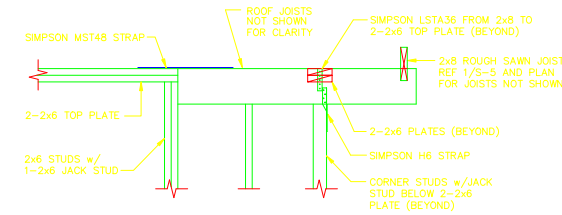
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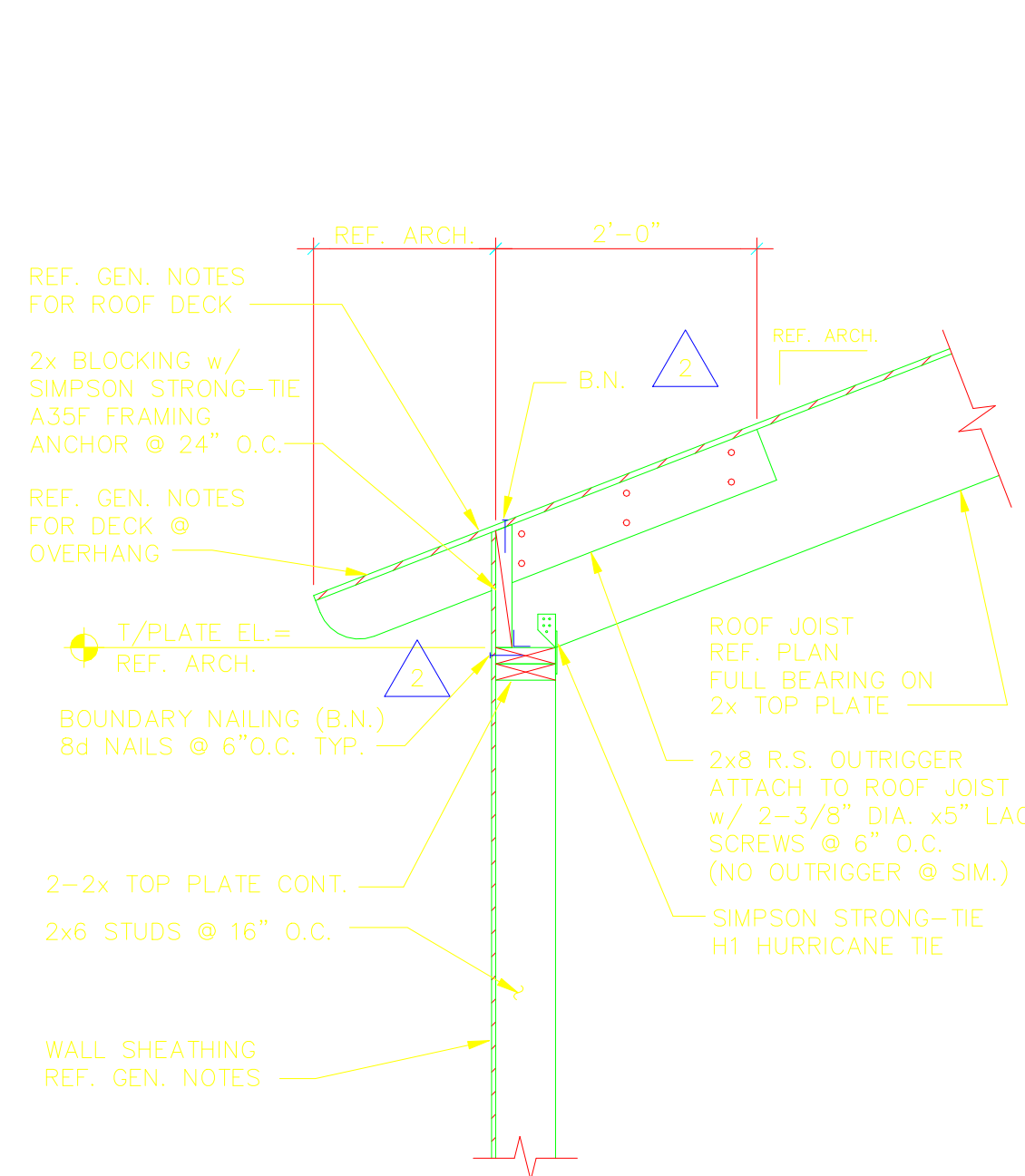
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SCALE: 3/4"=1'-0"



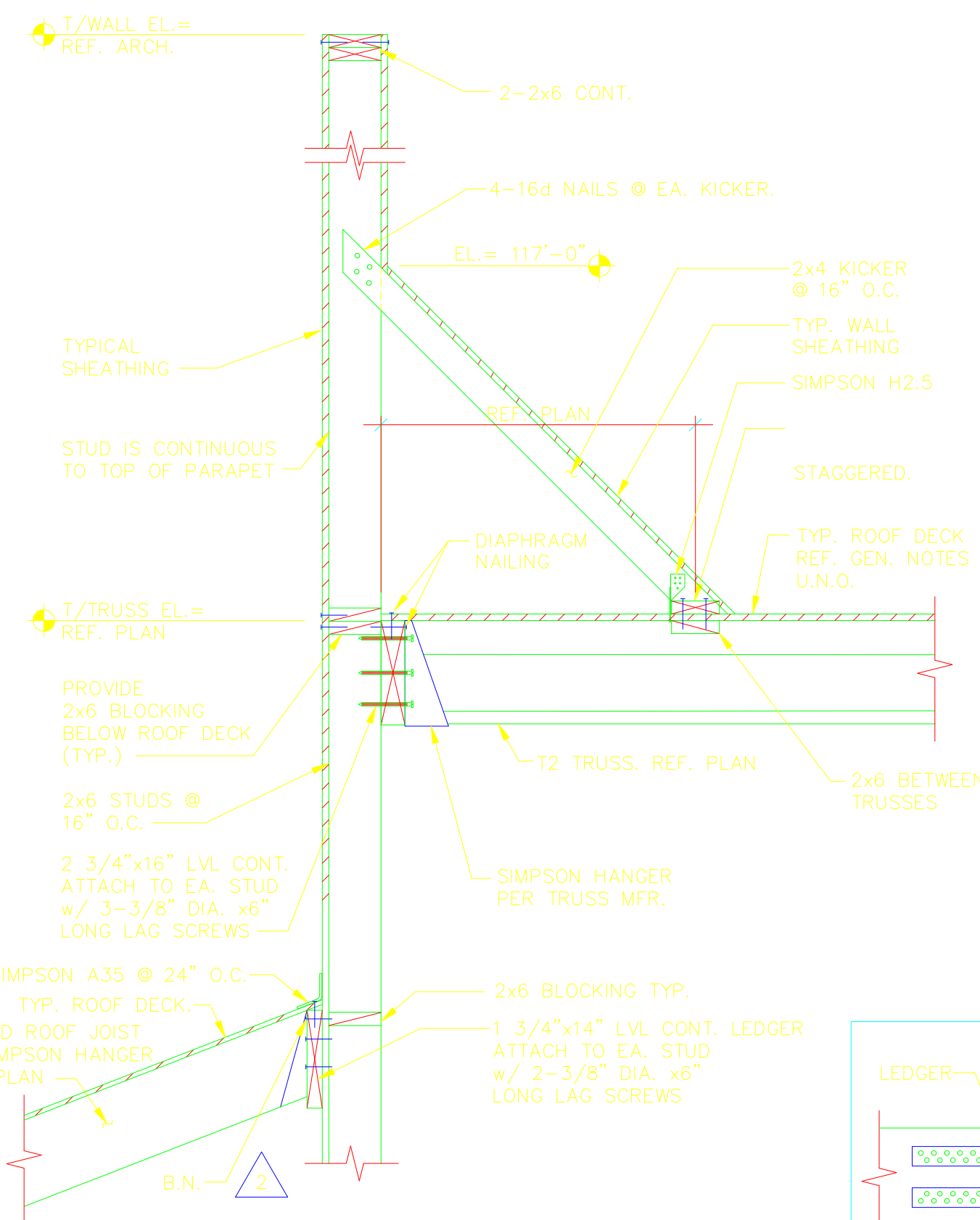
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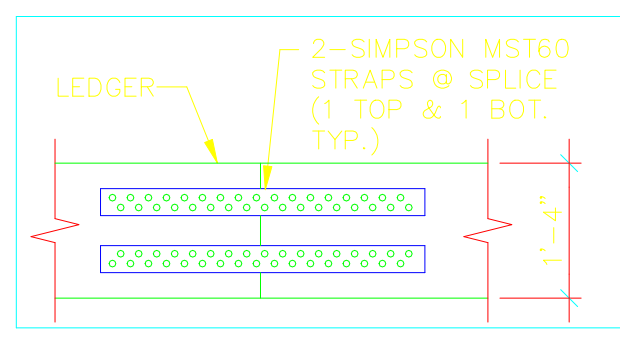
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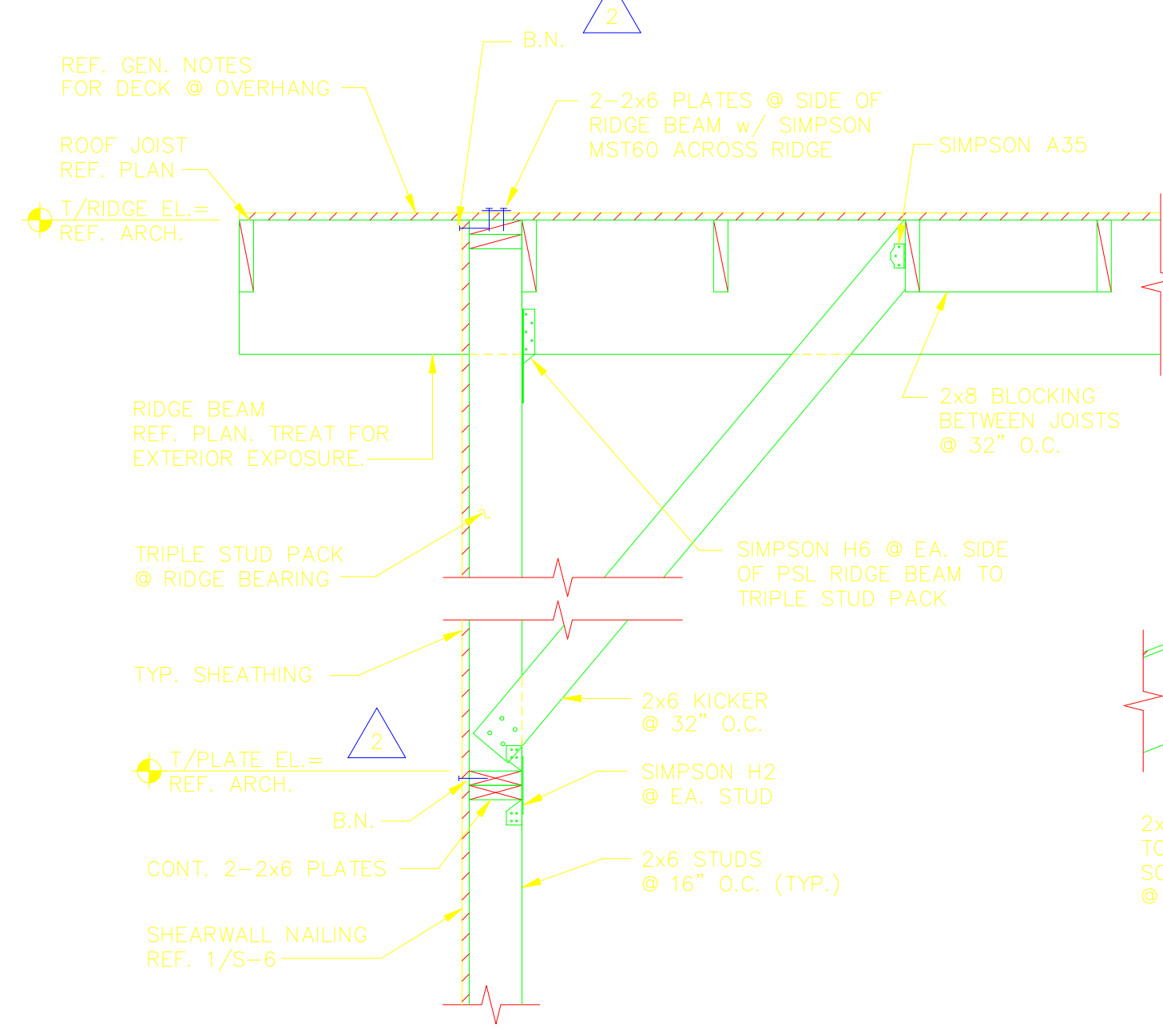
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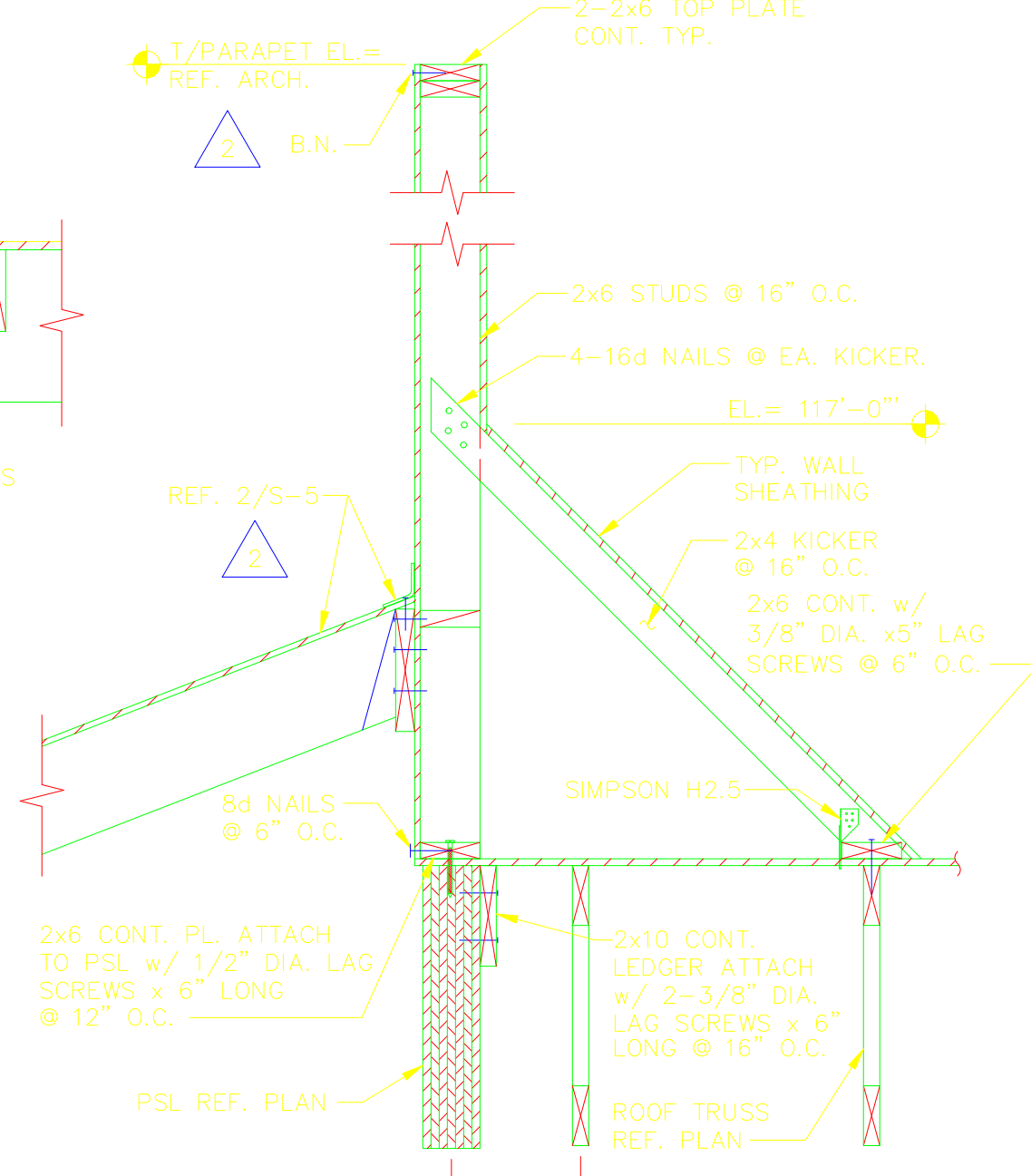
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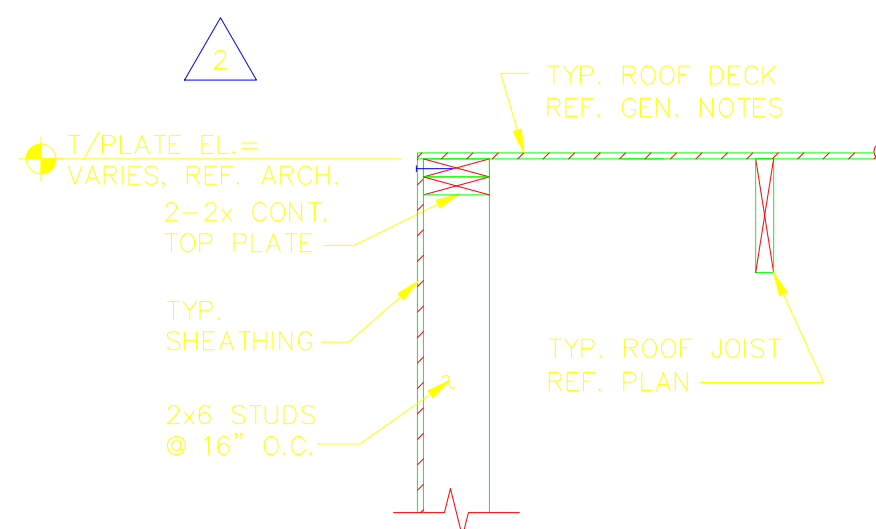
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SCALE: 3/4"=1'-0"



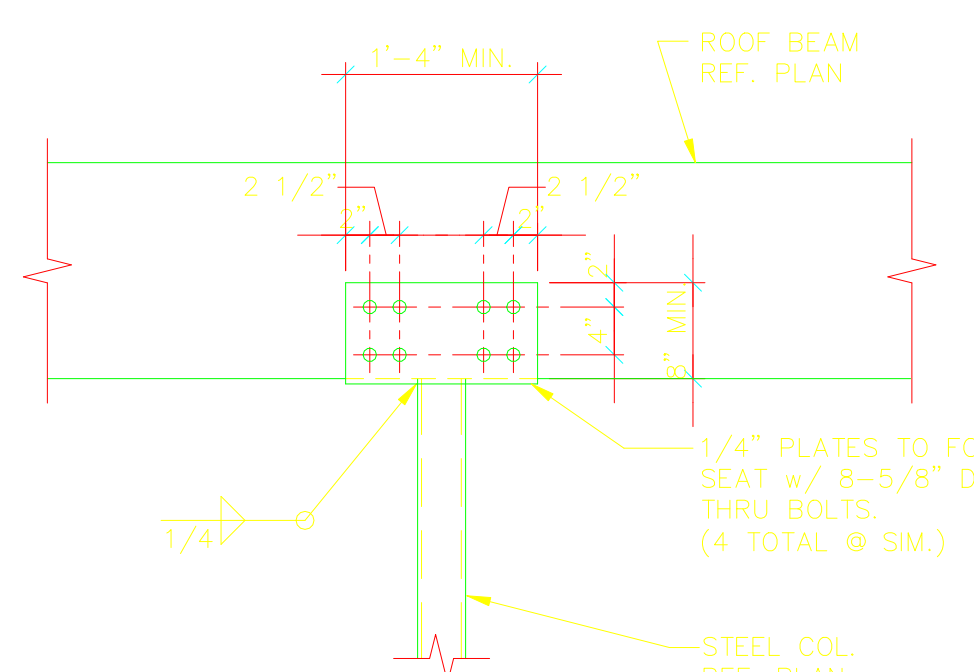
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SCALE: 3/4"=1'-0"



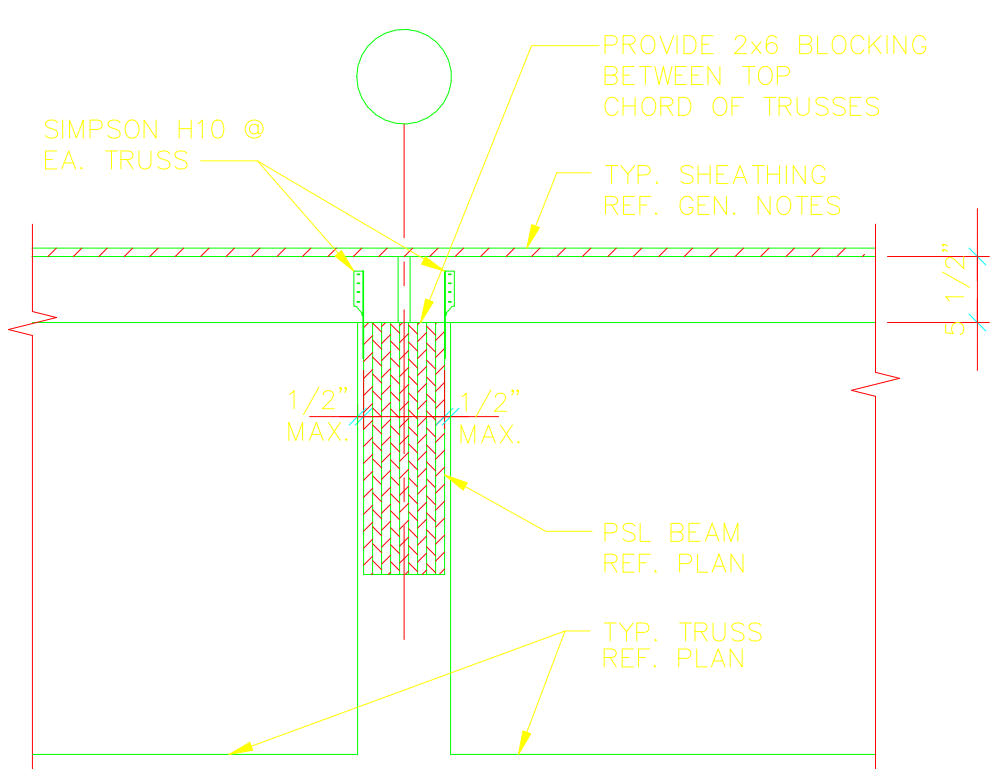
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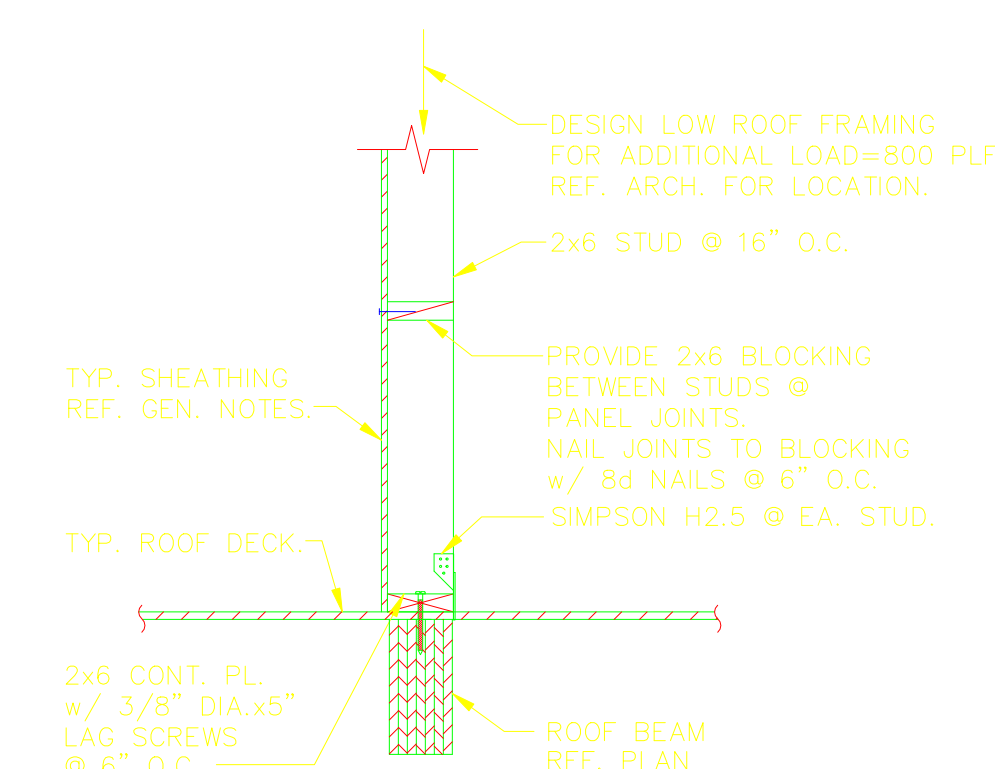
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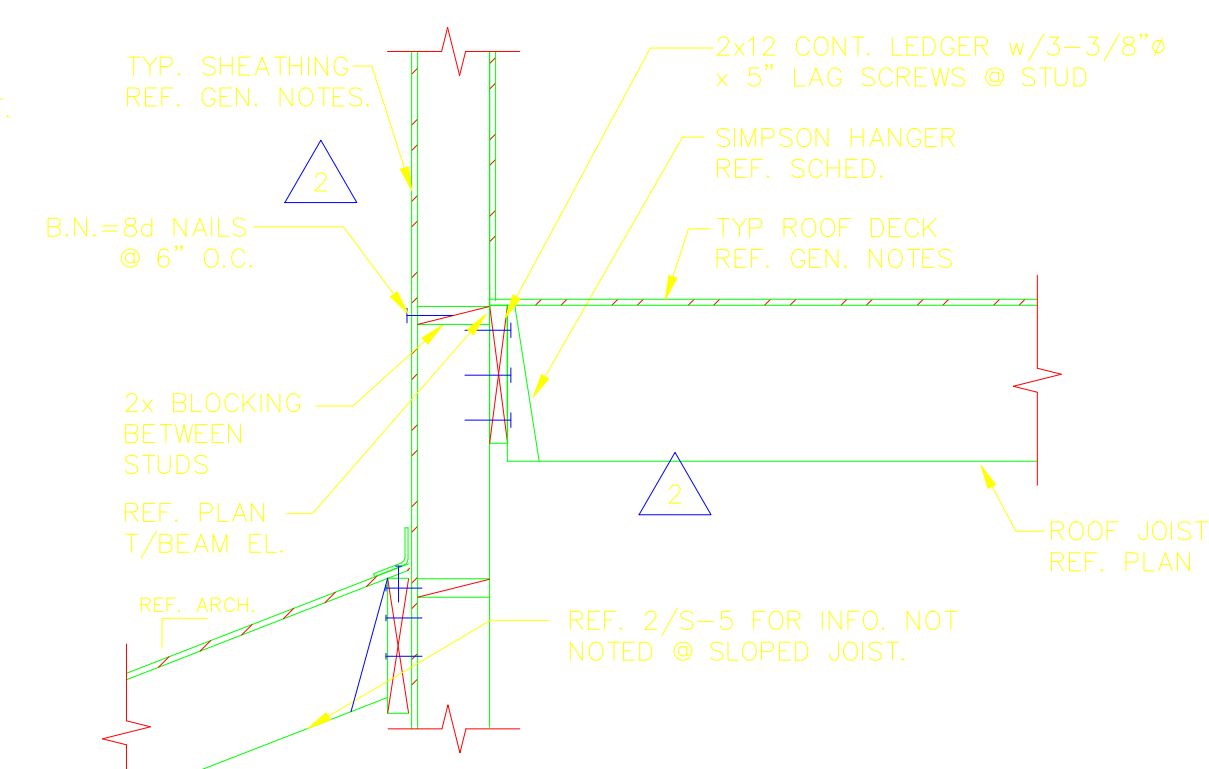
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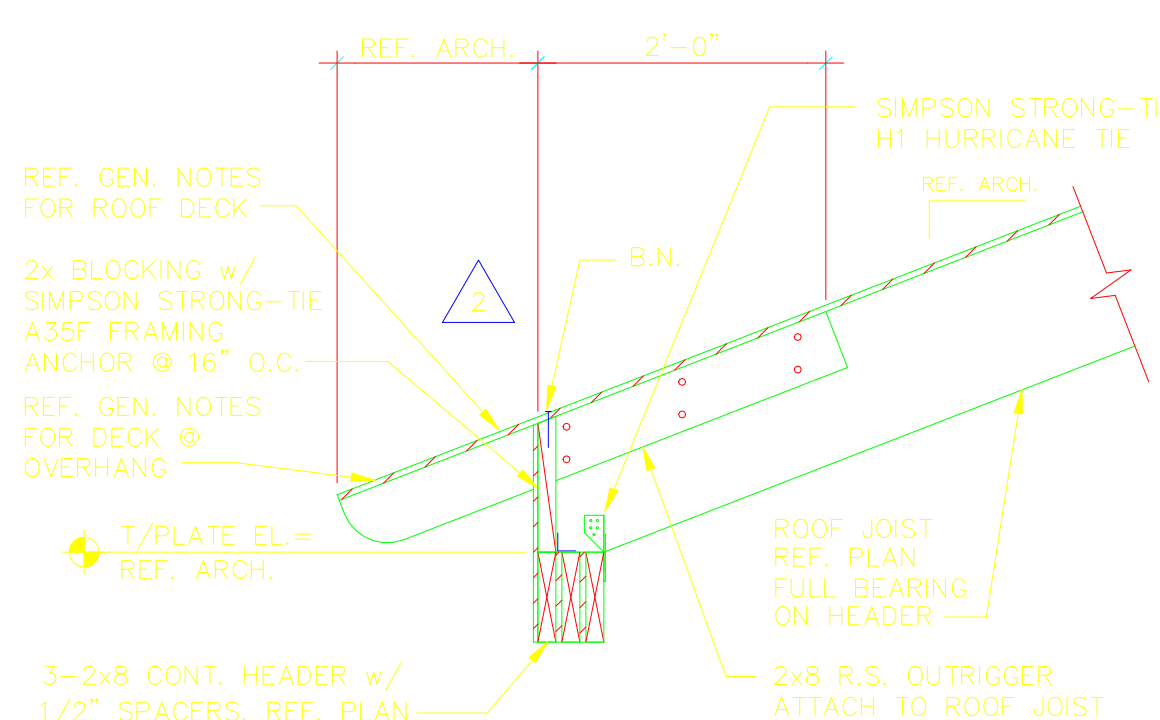
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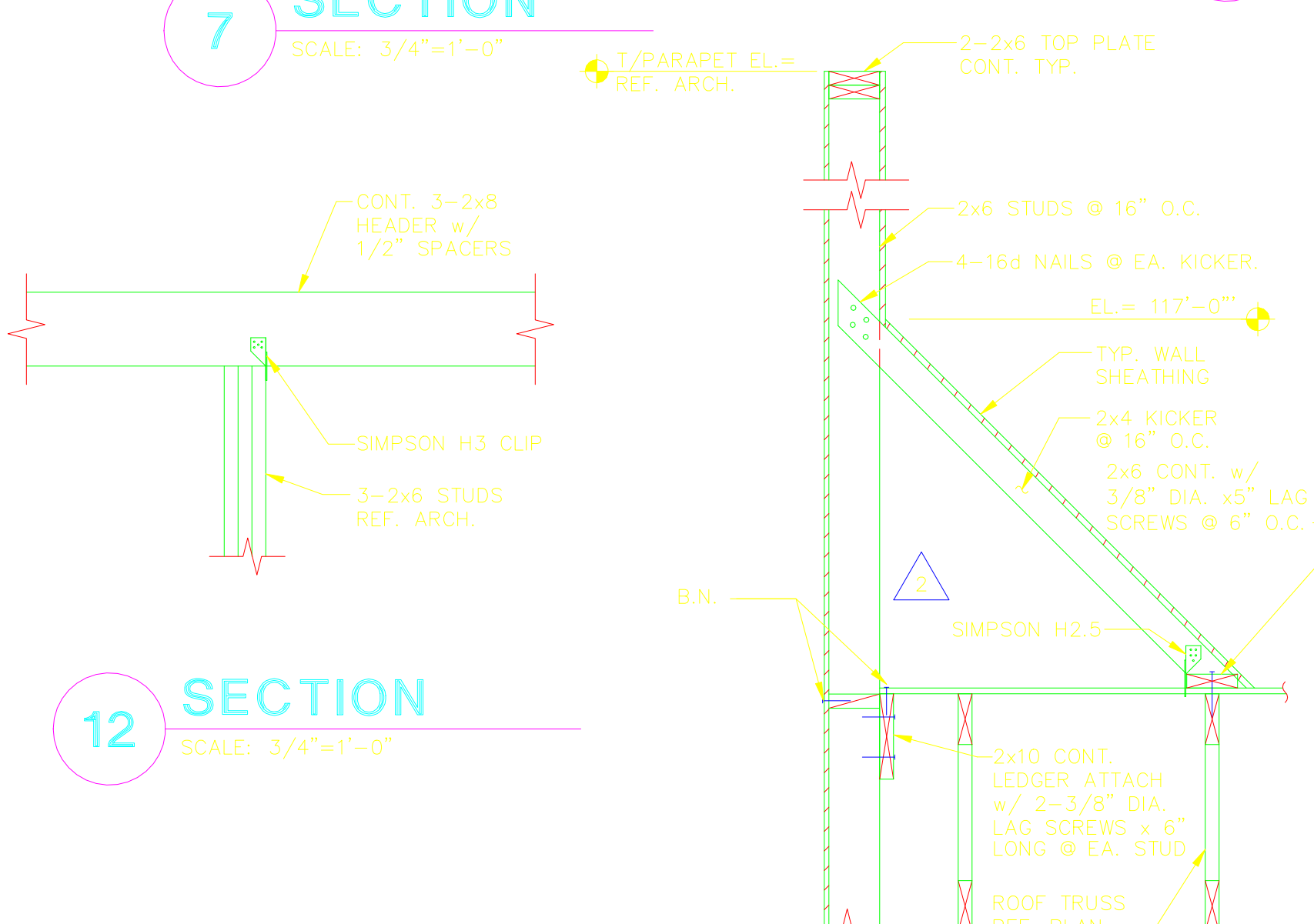
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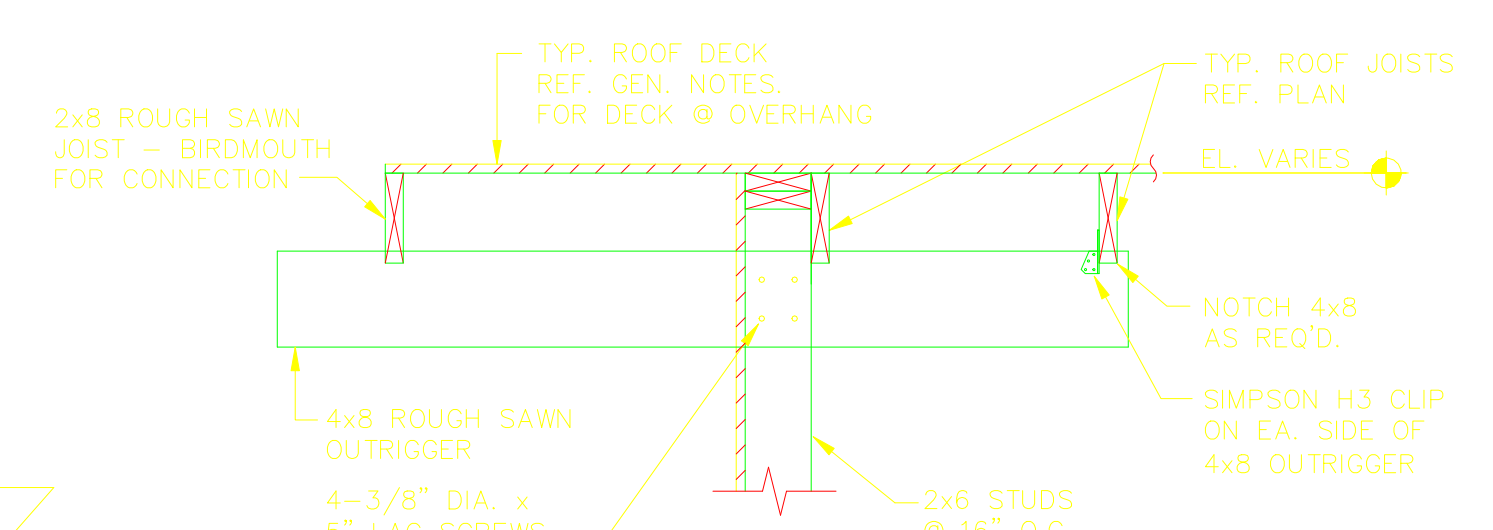
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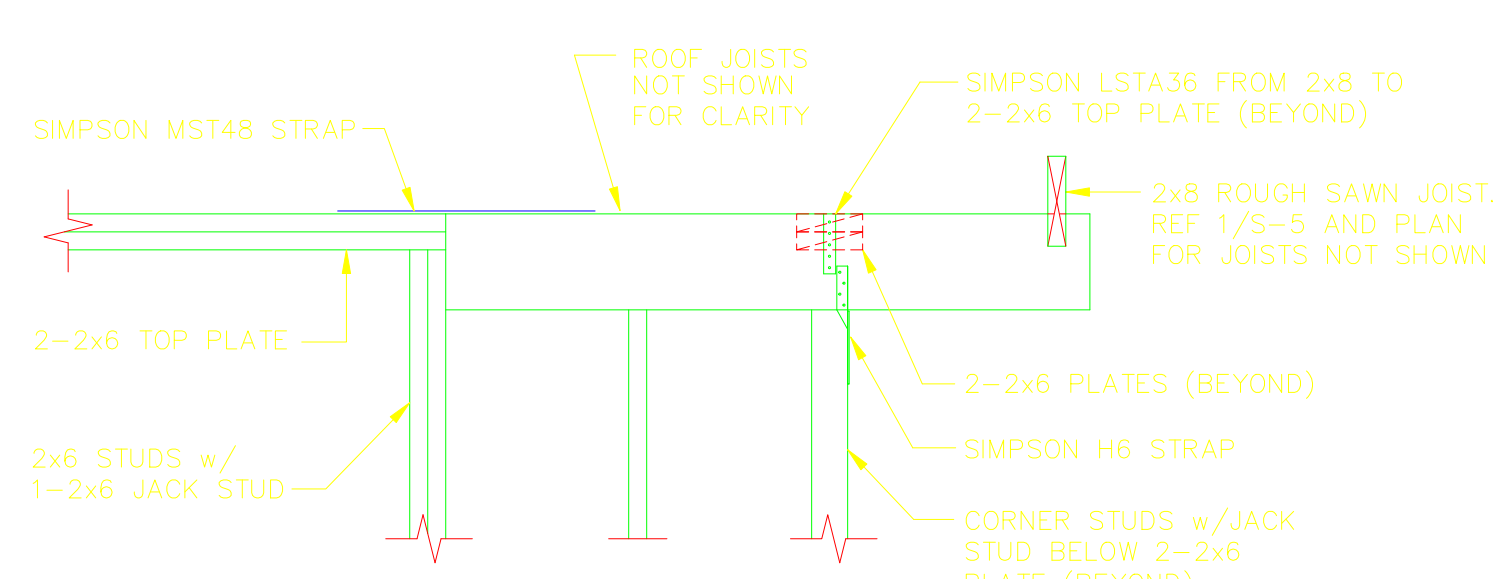
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SCALE: 3/4"=1'-0"



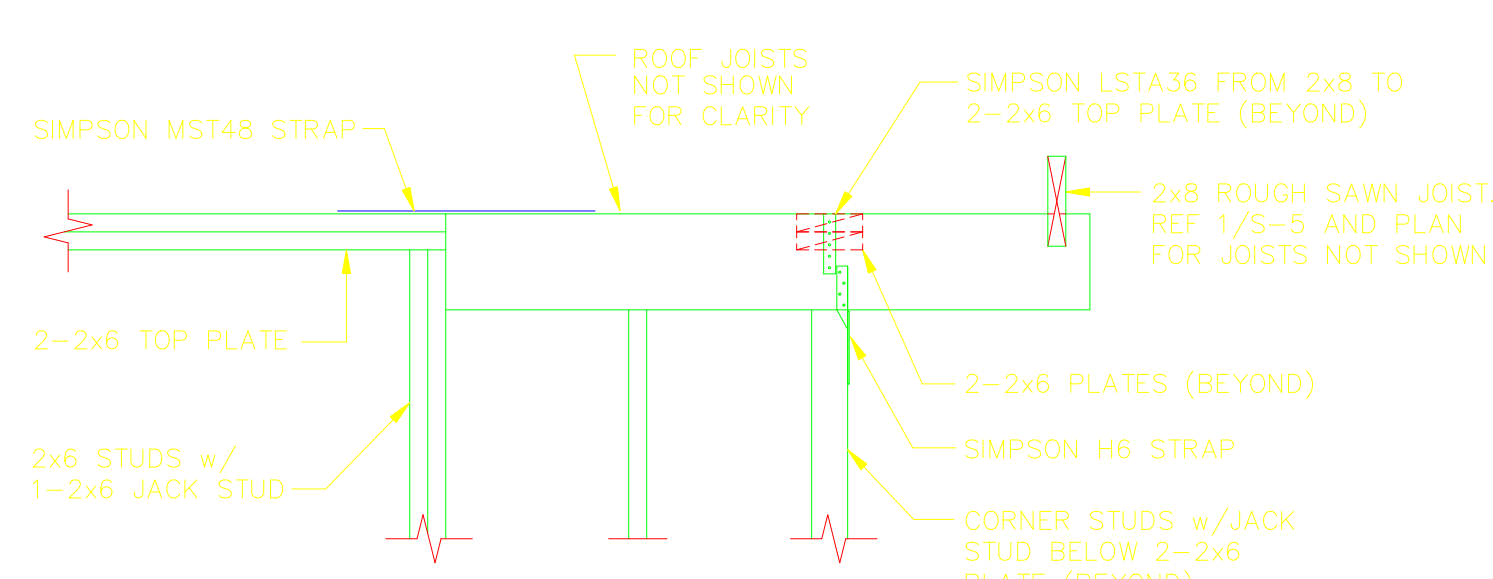
**12 SECTION**  
SCALE: 3/4"=1'-0"



**13 SECTION**  
SCALE: 3/4"=1'-0"



**14 SECTION**  
SCALE: 3/4"=1'-0"



**15 SECTION**  
SCALE: 3/4"=1'-0"

PERMIT ISSUE:

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PROTO 90  
HWY 287 AT PEARL RD.  
CASA GRANDE, AZ

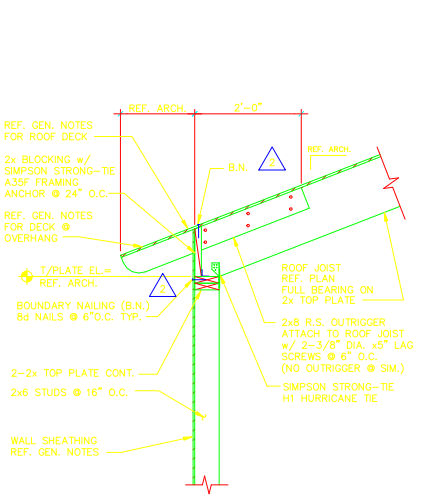


nca Architecture  
6500 Greenville Ave  
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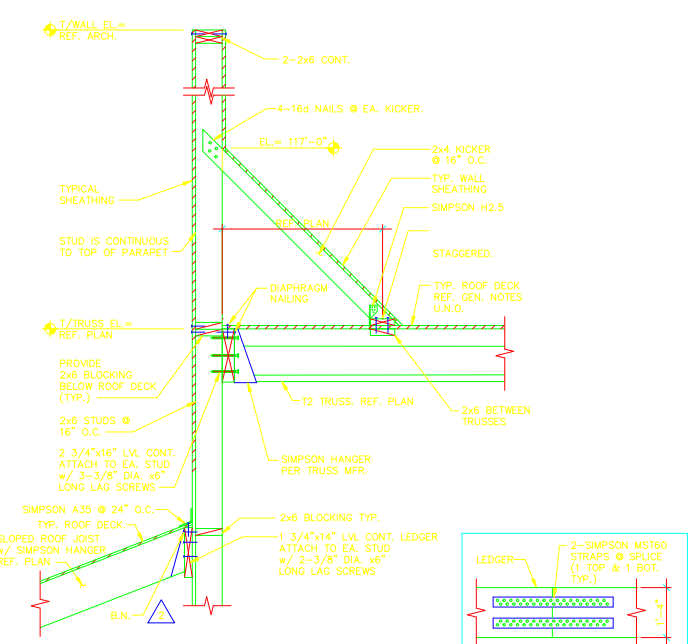
**HGA** Hart, Gauger & Associates, Inc.  
Structural Engineering Services  
12801 N. Central Expressway, #1400  
Dallas, TX 75243  
972-239-5111 / 972-239-5055 (Fax)

SHEET NO. S-5

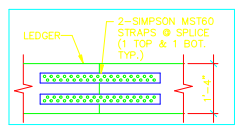




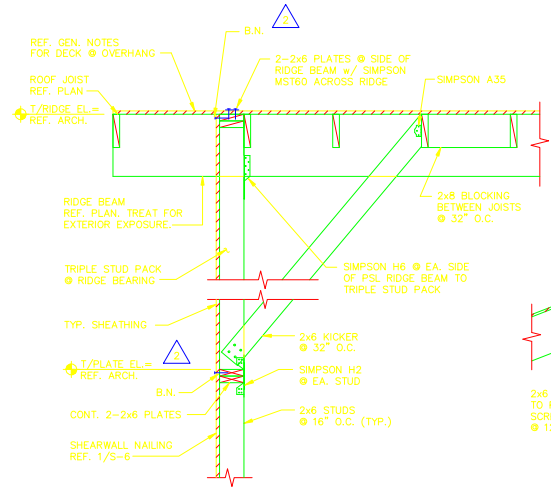
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SCALE: 3/4"=1'-0"



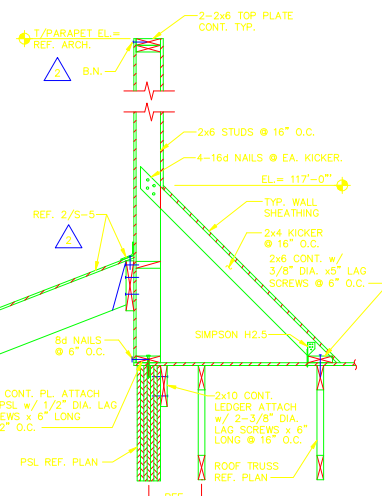
**2 SECTION**  
SCALE: 3/4"=1'-0"



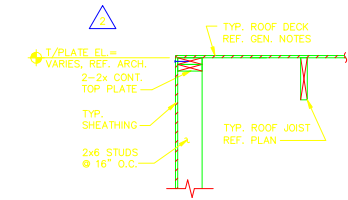
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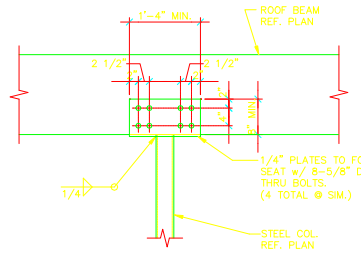
**4 SECTION**  
SCALE: 3/4"=1'-0"



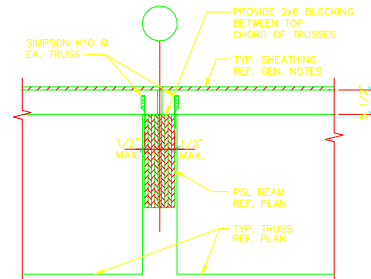
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SCALE: 3/4"=1'-0"



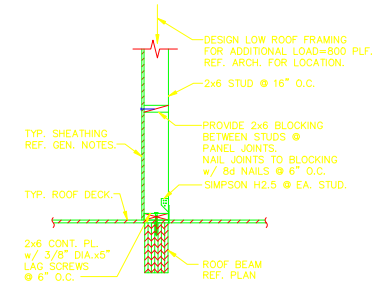
**6 SECTION**  
SCALE: 3/4"=1'-0"



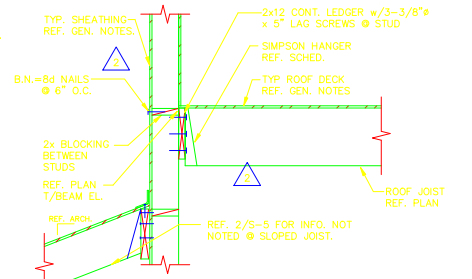
**7 SECTION**  
SCALE: 3/4"=1'-0"



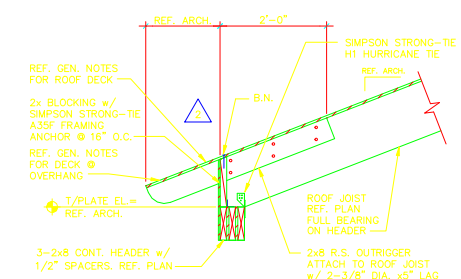
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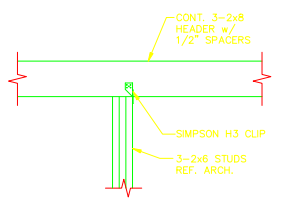
**9 SECTION**  
SCALE: 3/4"=1'-0"



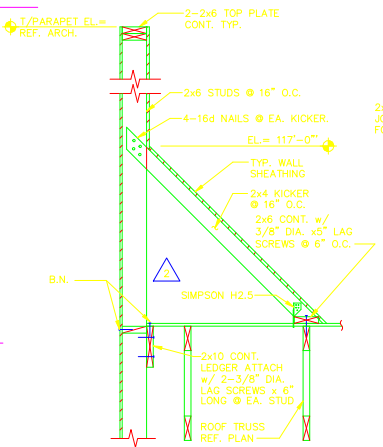
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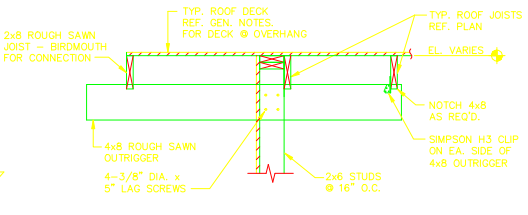
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SCALE: 3/4"=1'-0"



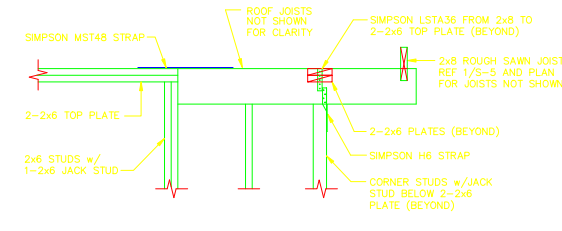
**12 SECTION**  
SCALE: 3/4"=1'-0"



**13 SECTION**  
SCALE: 3/4"=1'-0"



**14 SECTION**  
SCALE: 3/4"=1'-0"



**15 SECTION**  
SCALE: 3/4"=1'-0"

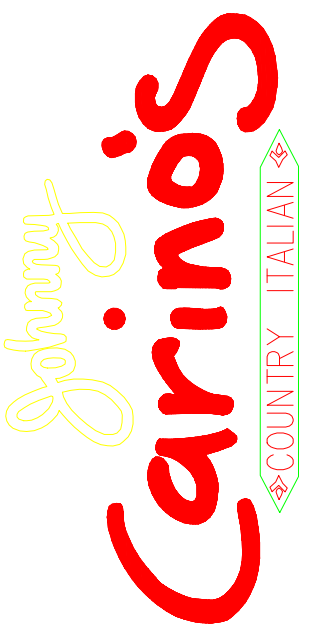
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PROTO 90 HWY 287 AT PEARL RD. CASA GRANDE, AZ

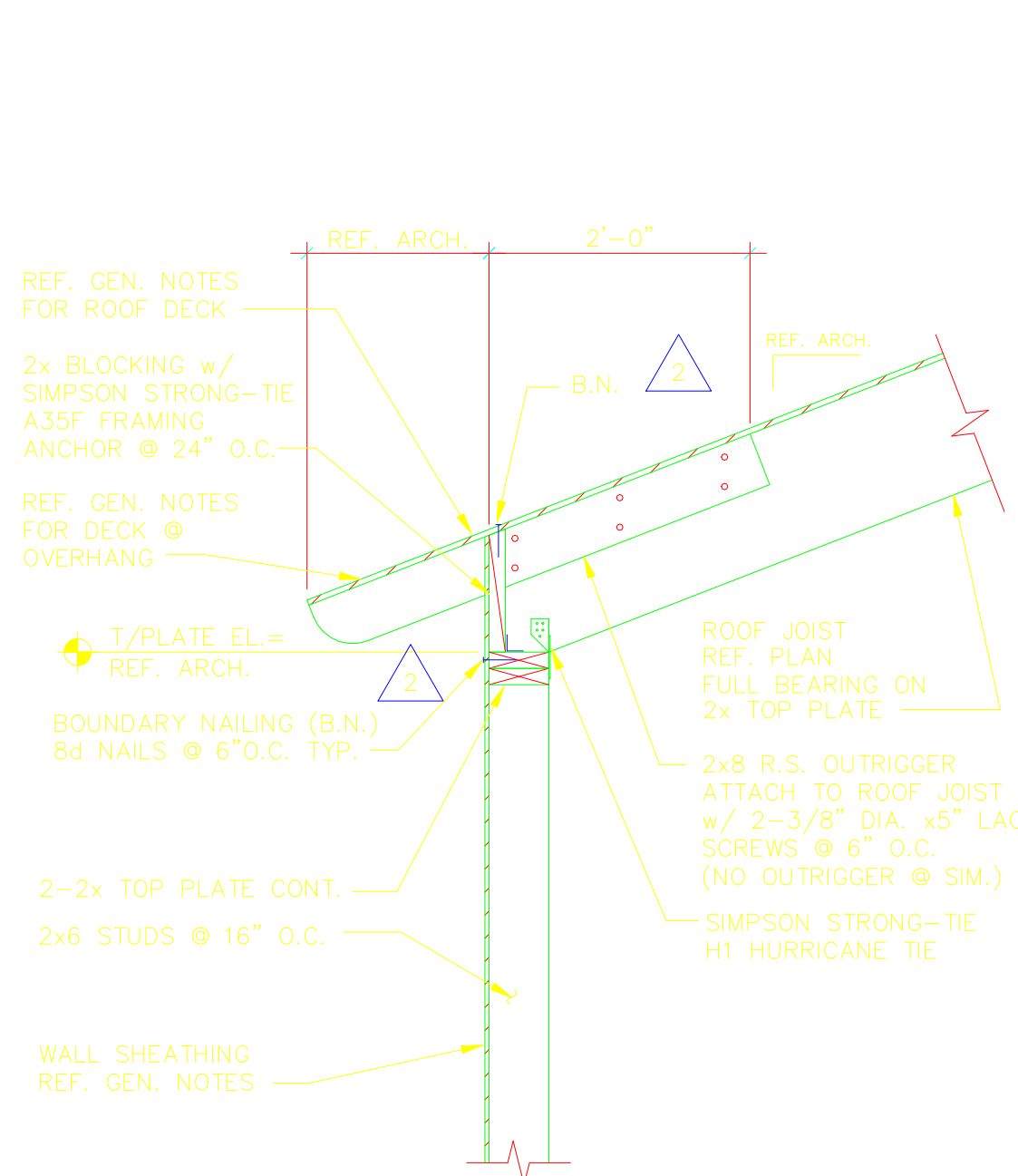


nca Architecture 6500 Greenville Ave Suite 307 Dallas, TX 75206 214.361.9901 Fax 361.9906

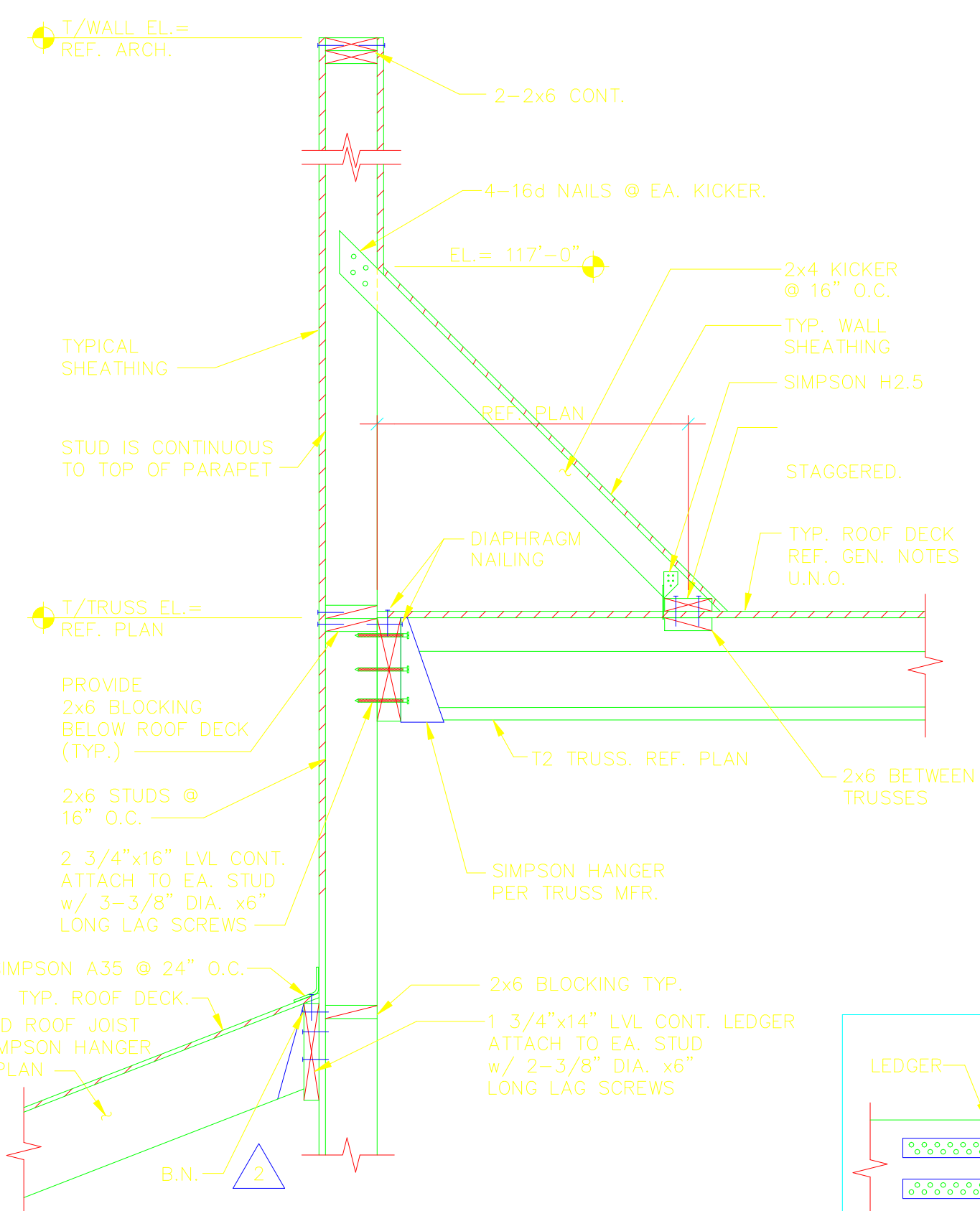
SHEET NO. S-5

HGA Hart, Gauger & Associates, Inc. Structural Engineering Services 12801 N. Central Expressway, #1400 Dallas, TX 75243 972-239-5111 / 972-239-5055 (Fax)

BRAWN JLL DESIGN JEK HGA JOB # 203180

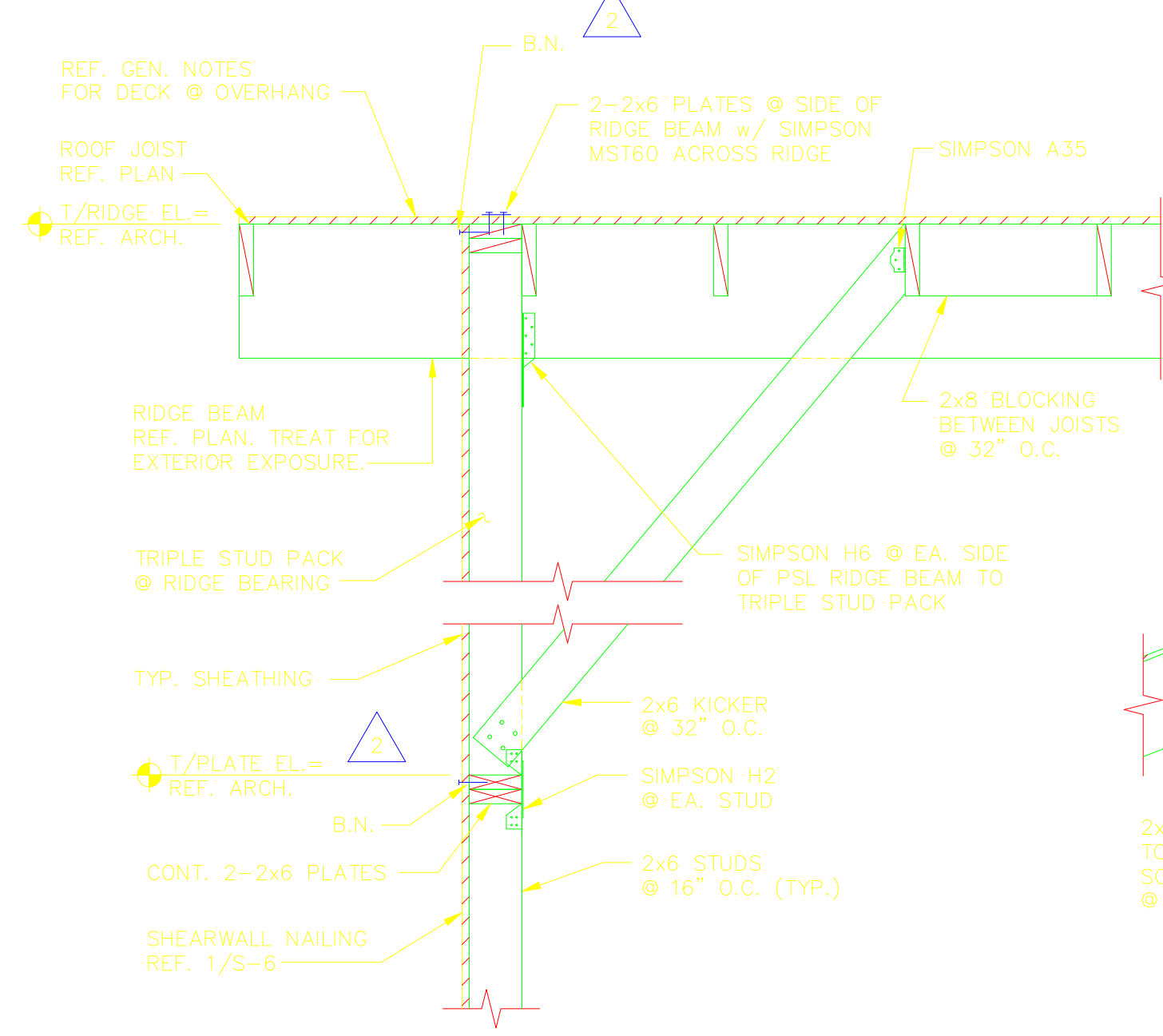


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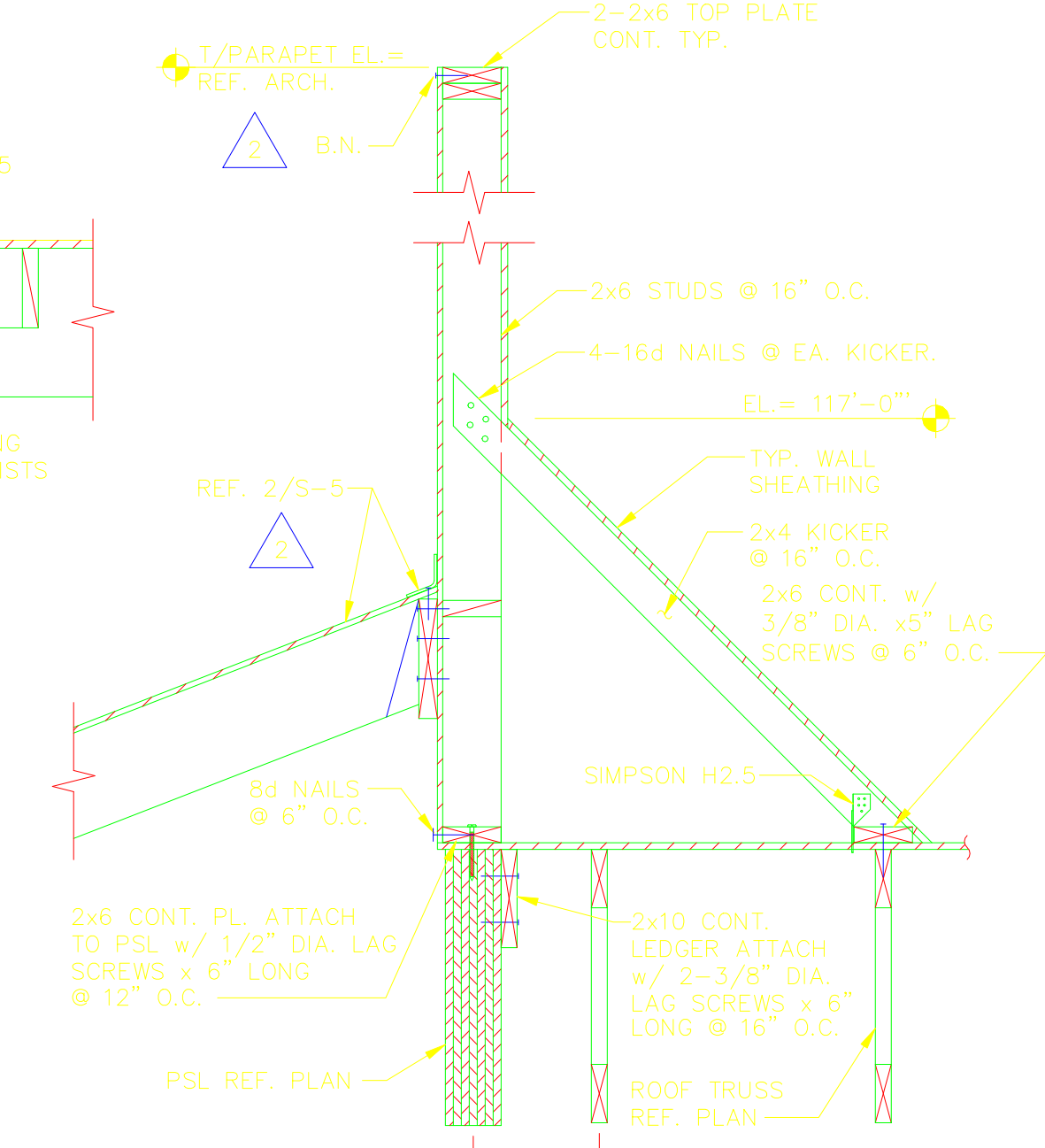


2 SECTION SCALE: 3/4"=1'-0"

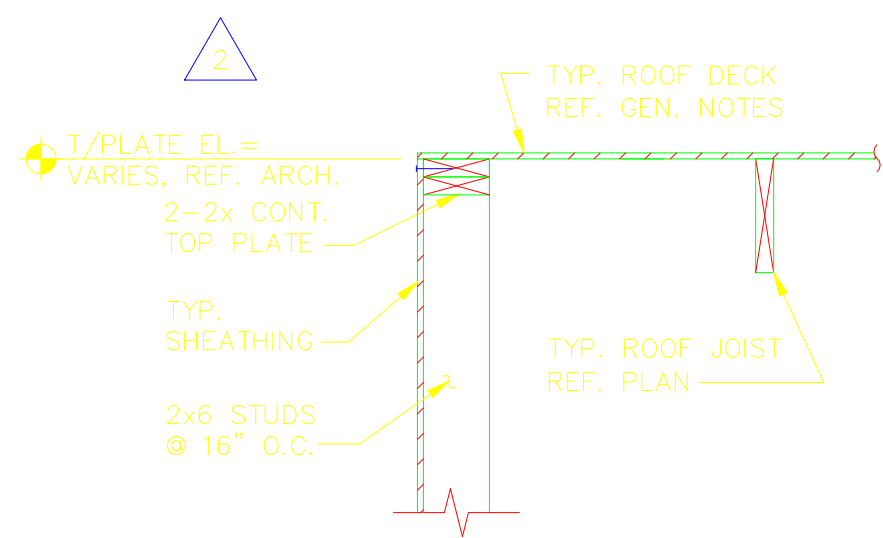
3 NOT USED SCALE: 3/4"=1'-0"



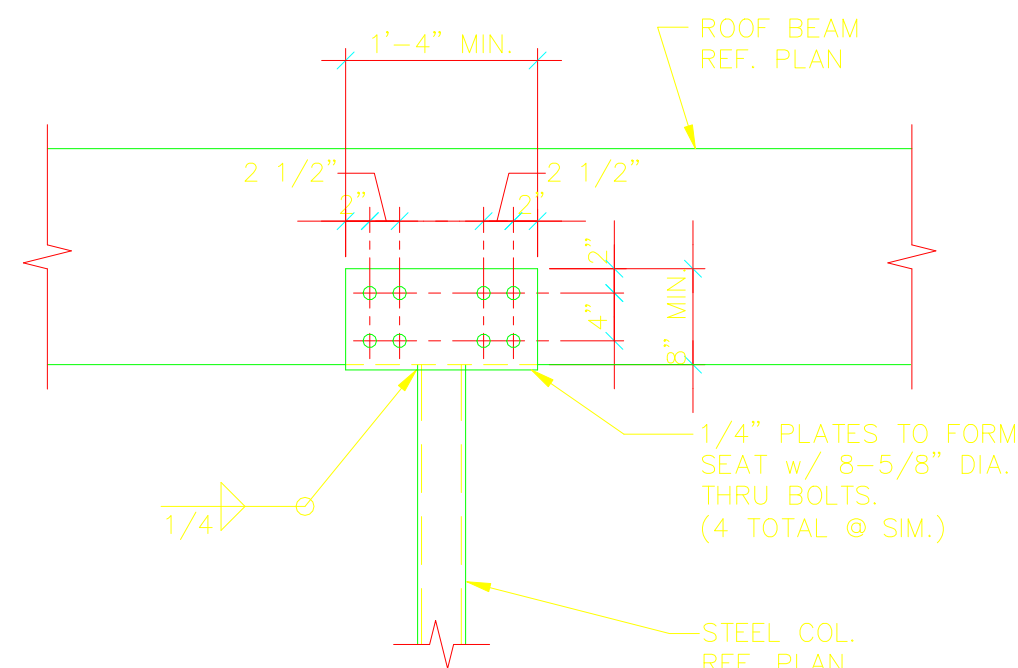
4 SECTION SCALE: 3/4"=1'-0"



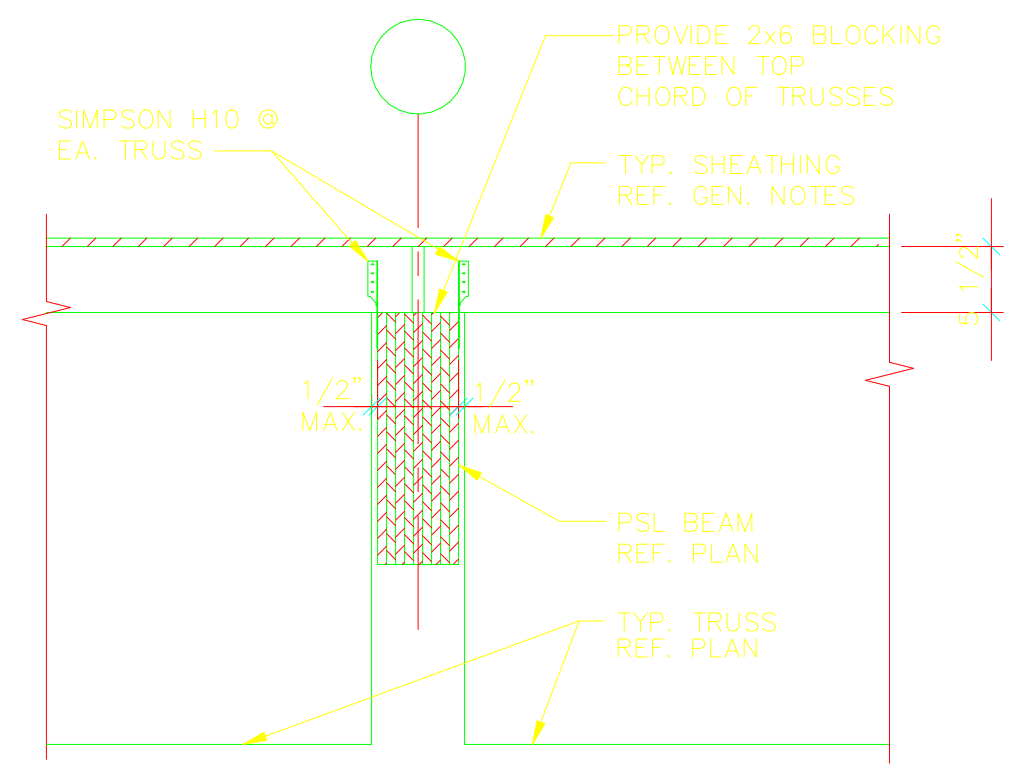
5 SECTION SCALE: 3/4"=1'-0"



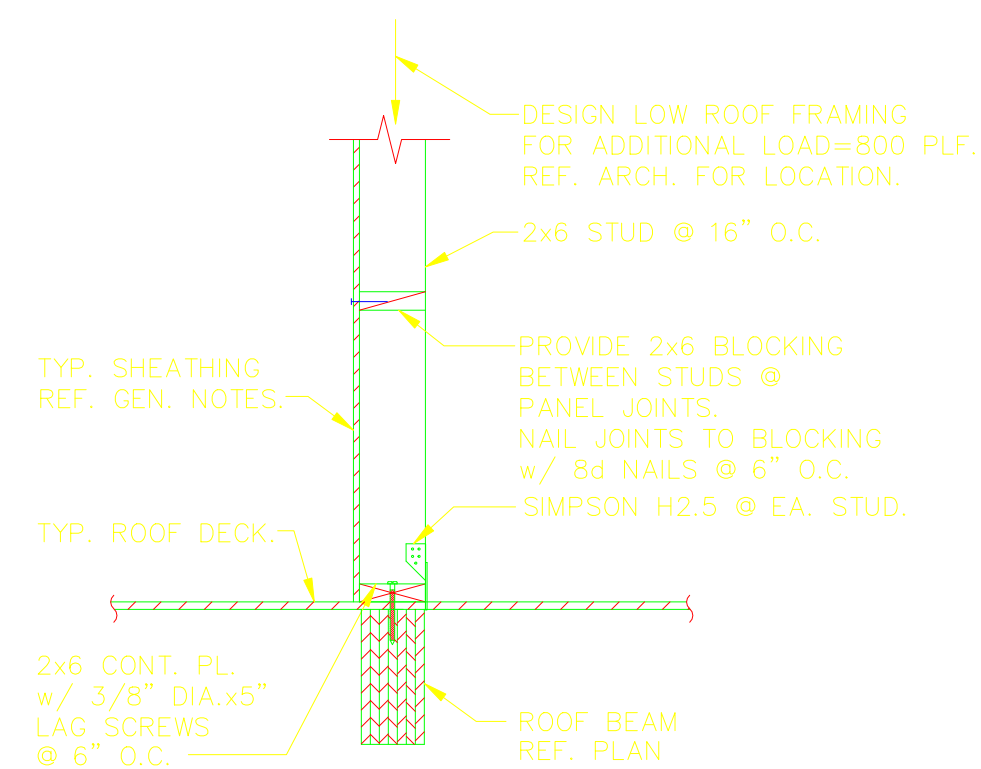
6 SECTION SCALE: 3/4"=1'-0"



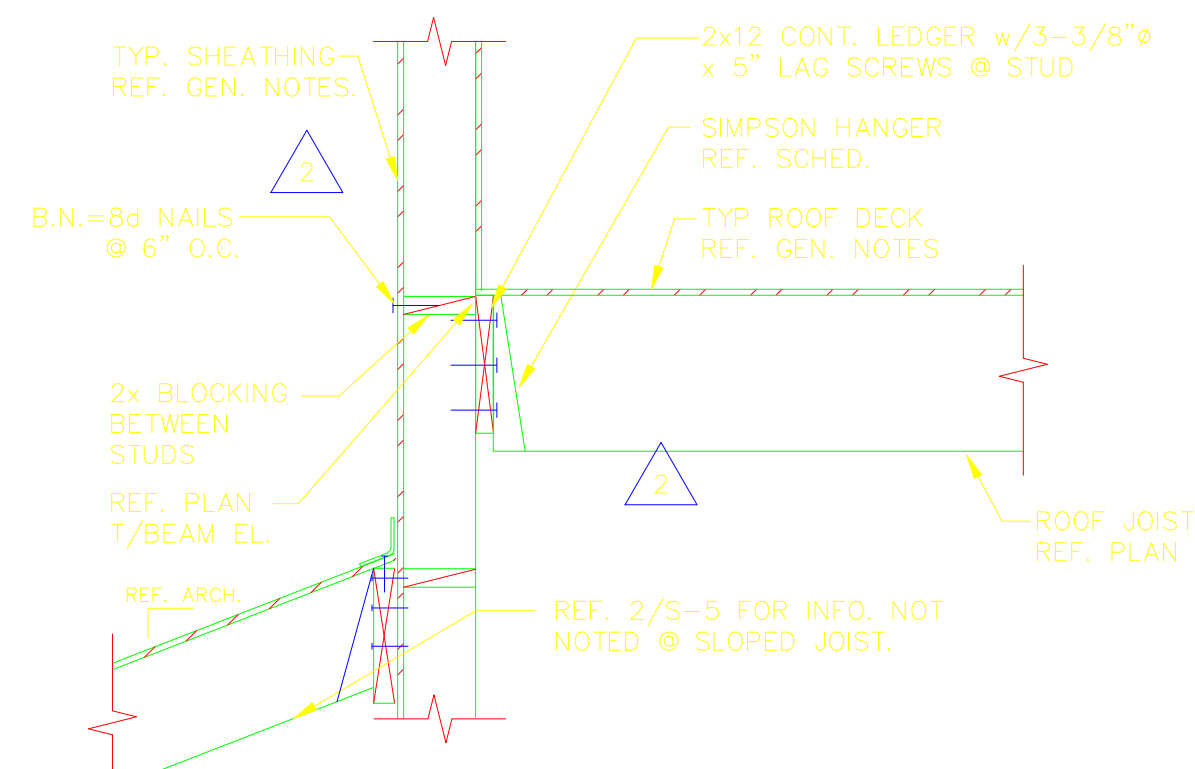
7 SECTION SCALE: 3/4"=1'-0"



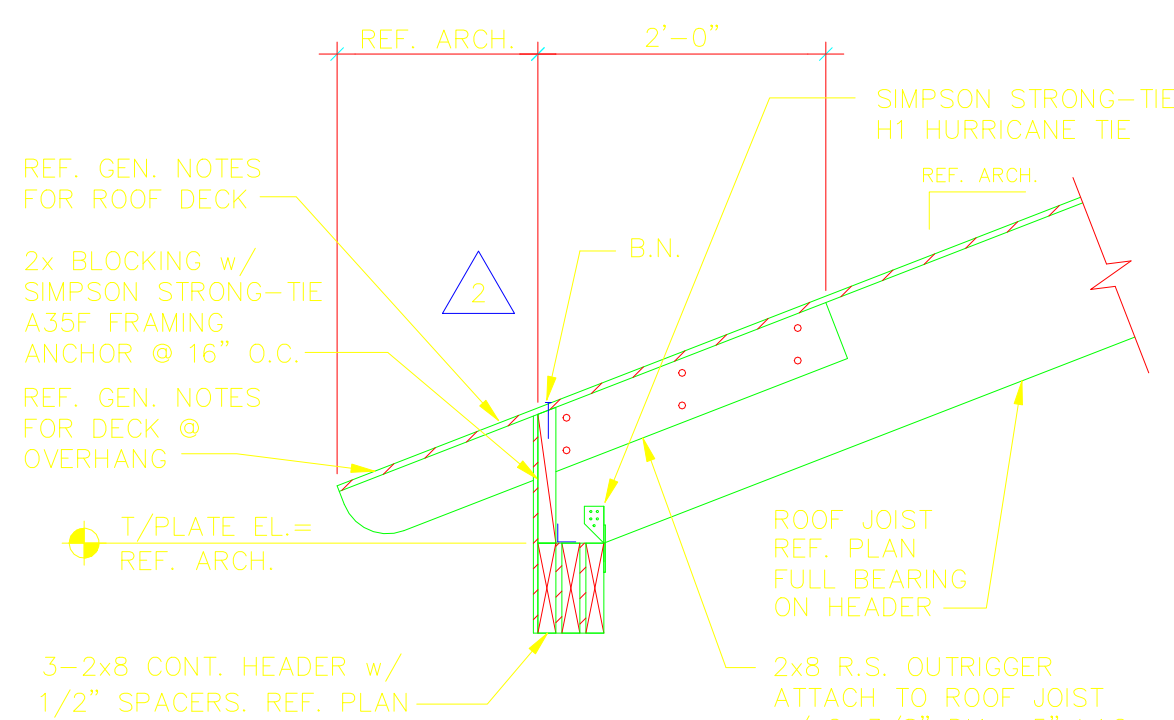
8 SECTION SCALE: 3/4"=1'-0"



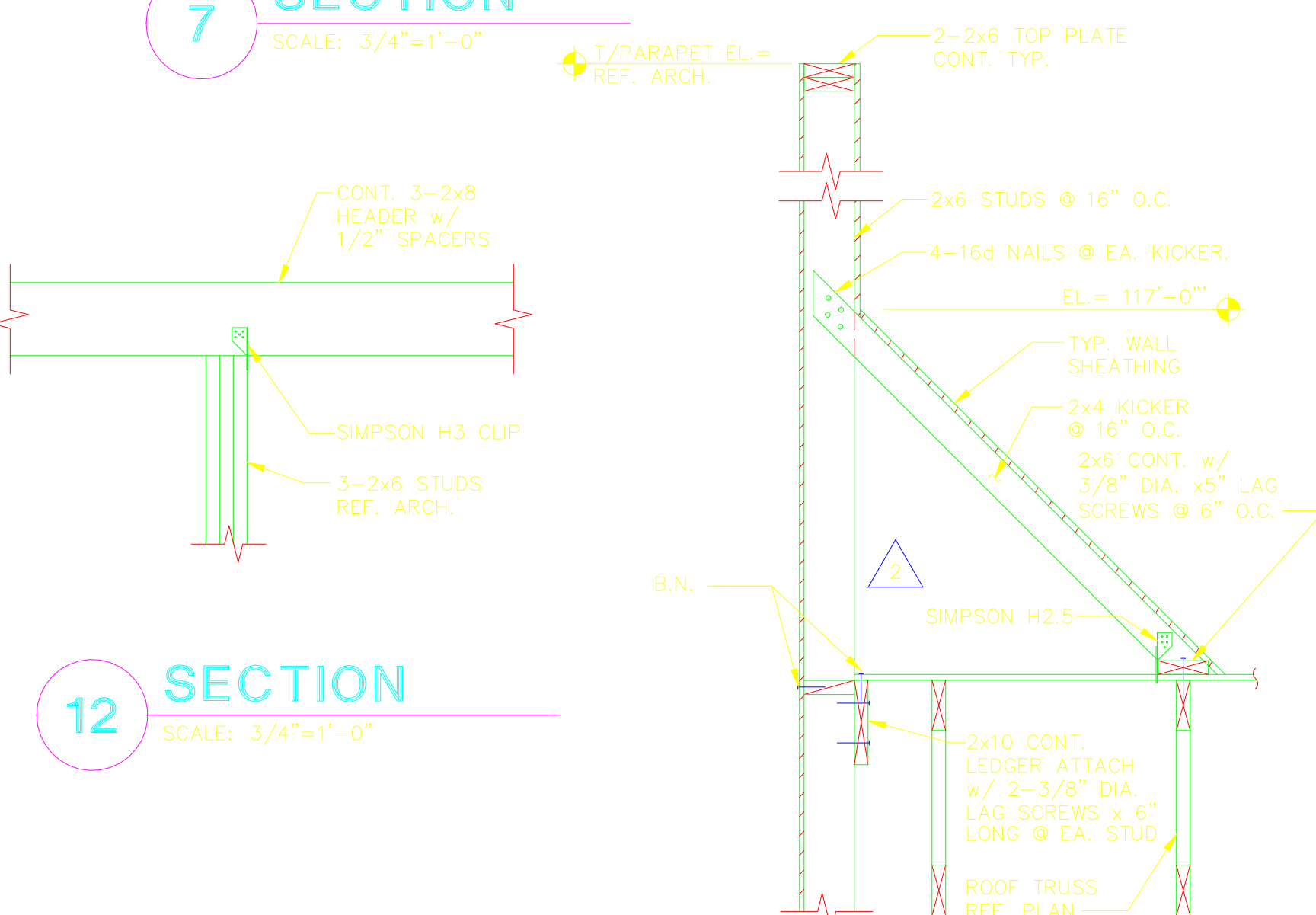
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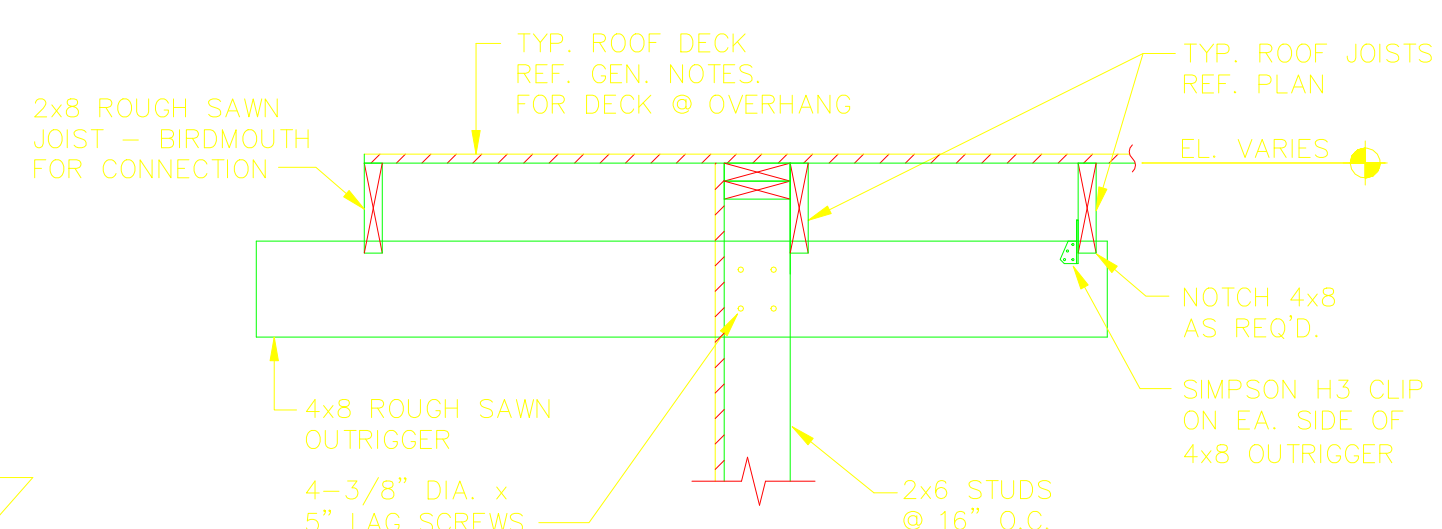
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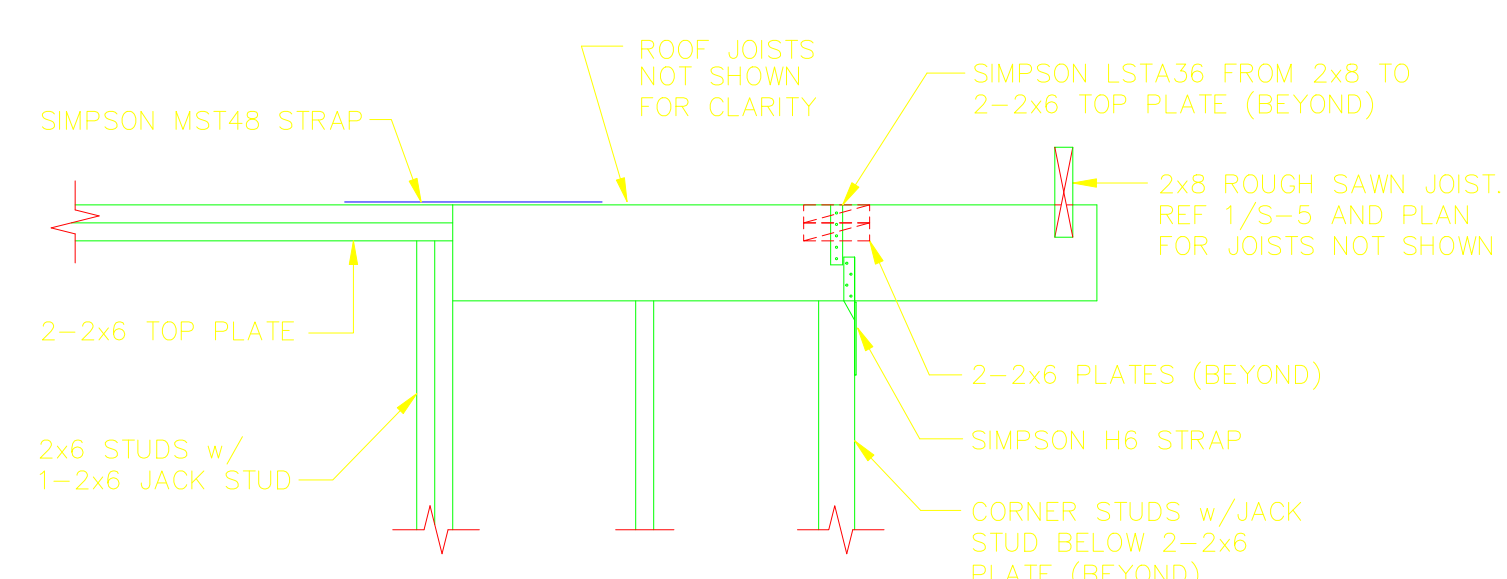
11 SECTION SCALE: 3/4"=1'-0"



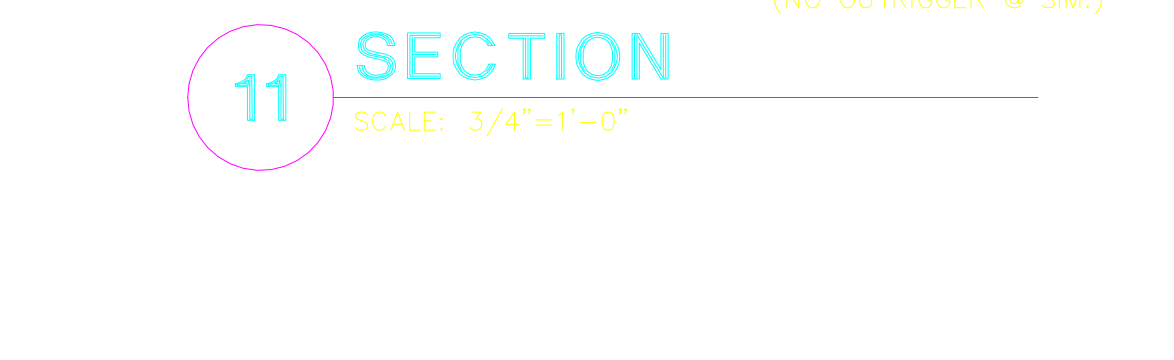
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13 SECTION SCALE: 3/4"=1'-0"



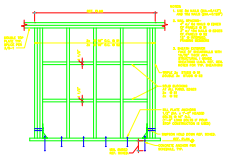
14 SECTION SCALE: 3/4"=1'-0"



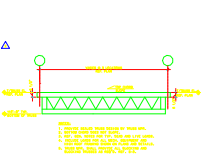
15 SECTION SCALE: 3/4"=1'-0"

16 SECTION SCALE: 3/4"=1'-0"

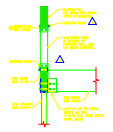
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1 SHEAR WALL ELEVATION



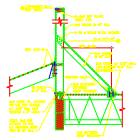
2 T1 TRUSS PROFILE



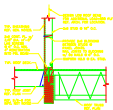
4 SECTION

**SIMPSON HOLD-DOWN SCHEDULE**

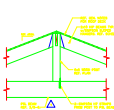
Anchor	Wind Speed (mph)	Wind Zone	Height (ft)	Exposure	Category
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HD30C	140	1	15	C	II
HD30C	140	1	15	D	II
HD30C	140	1	15	E	II
HD30C	140	1	15	F	II
HD30C	140	1	15	G	II
HD30C	140	1	15	H	II
HD30C	140	1	15	I	II
HD30C	140	1	15	J	II
HD30C	140	1	15	K	II
HD30C	140	1	15	L	II
HD30C	140	1	15	M	II
HD30C	140	1	15	N	II
HD30C	140	1	15	O	II
HD30C	140	1	15	P	II
HD30C	140	1	15	Q	II
HD30C	140	1	15	R	II
HD30C	140	1	15	S	II
HD30C	140	1	15	T	II
HD30C	140	1	15	U	II
HD30C	140	1	15	V	II
HD30C	140	1	15	W	II
HD30C	140	1	15	X	II
HD30C	140	1	15	Y	II
HD30C	140	1	15	Z	II



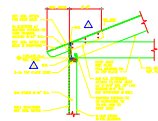
5 SECTION



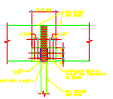
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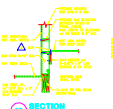
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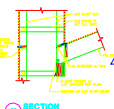
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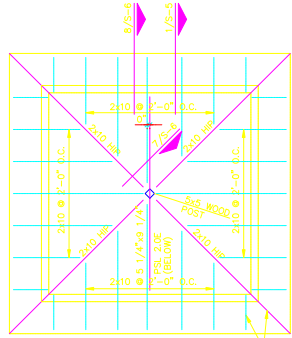
9 SECTION



10 SECTION



11 SECTION

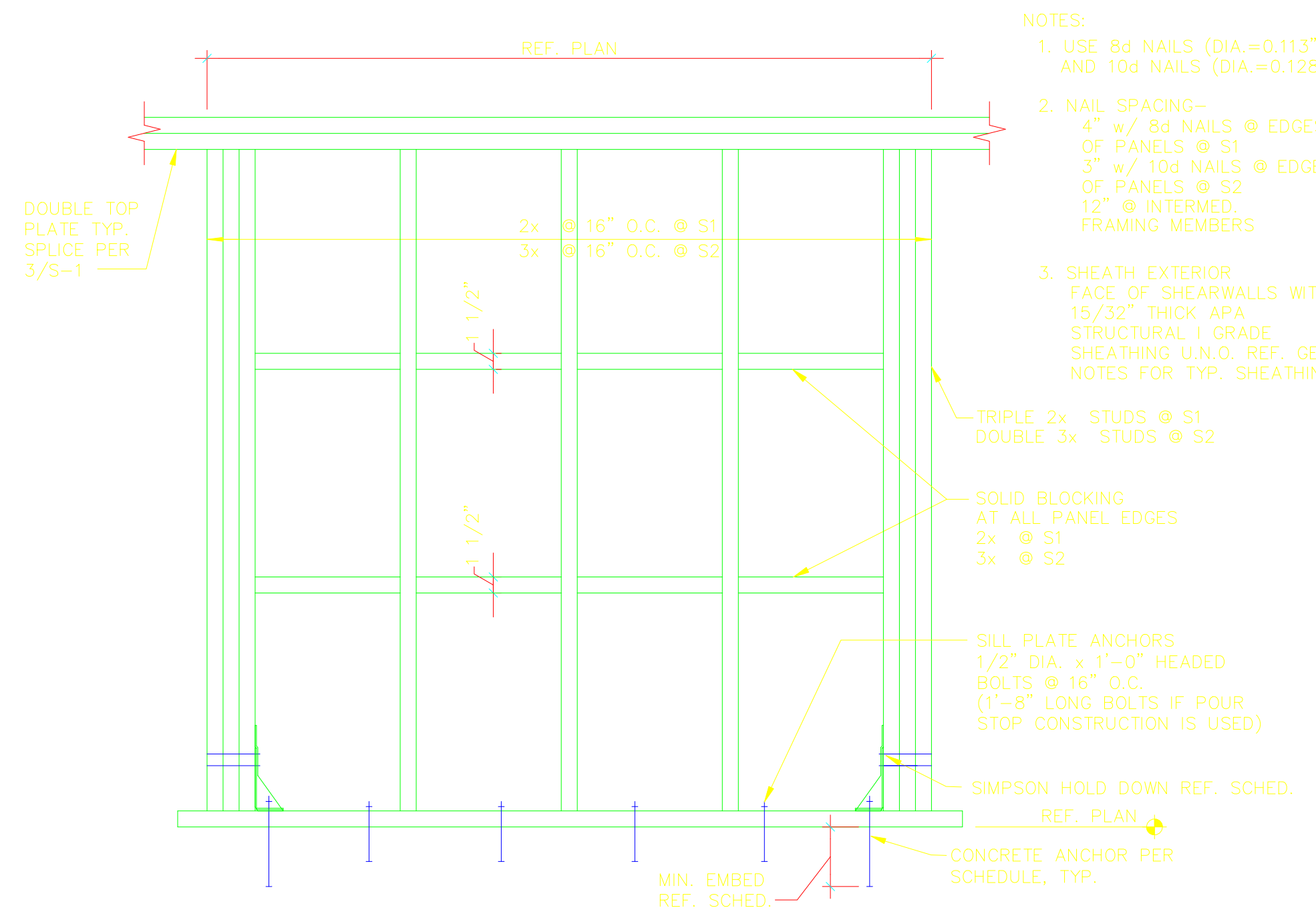


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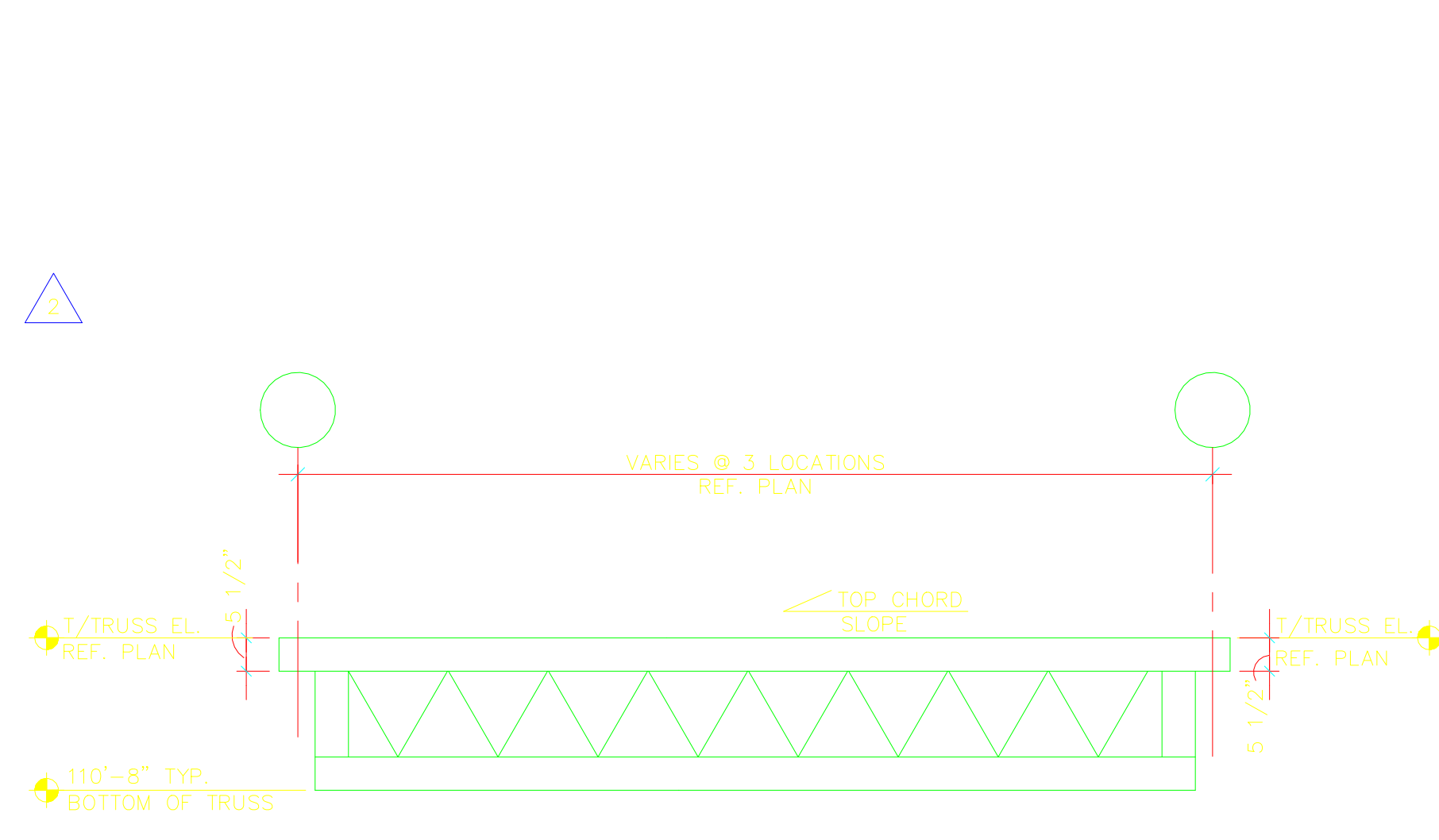
**PARTIAL PLAN @ HIGH ROOF FRAMING**

SCALE: 1/4"=1'-0"

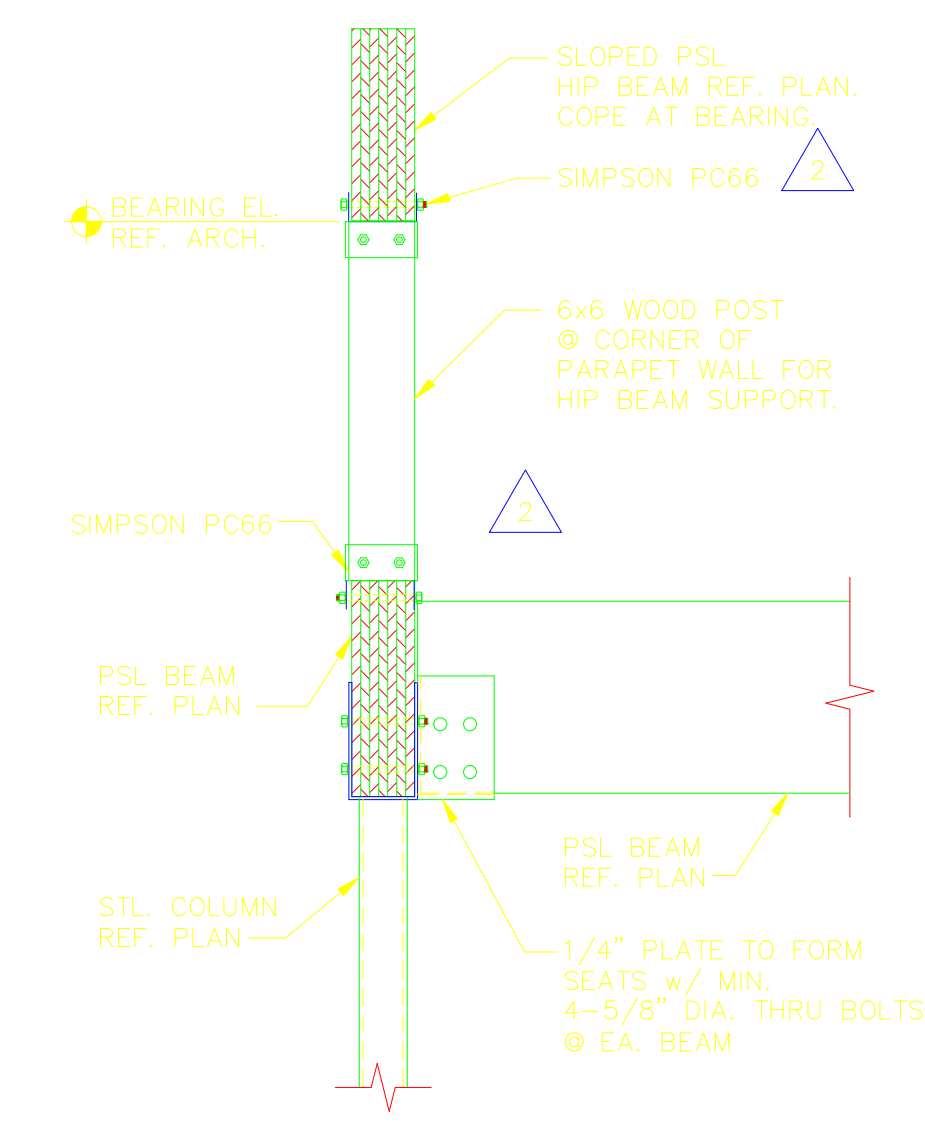
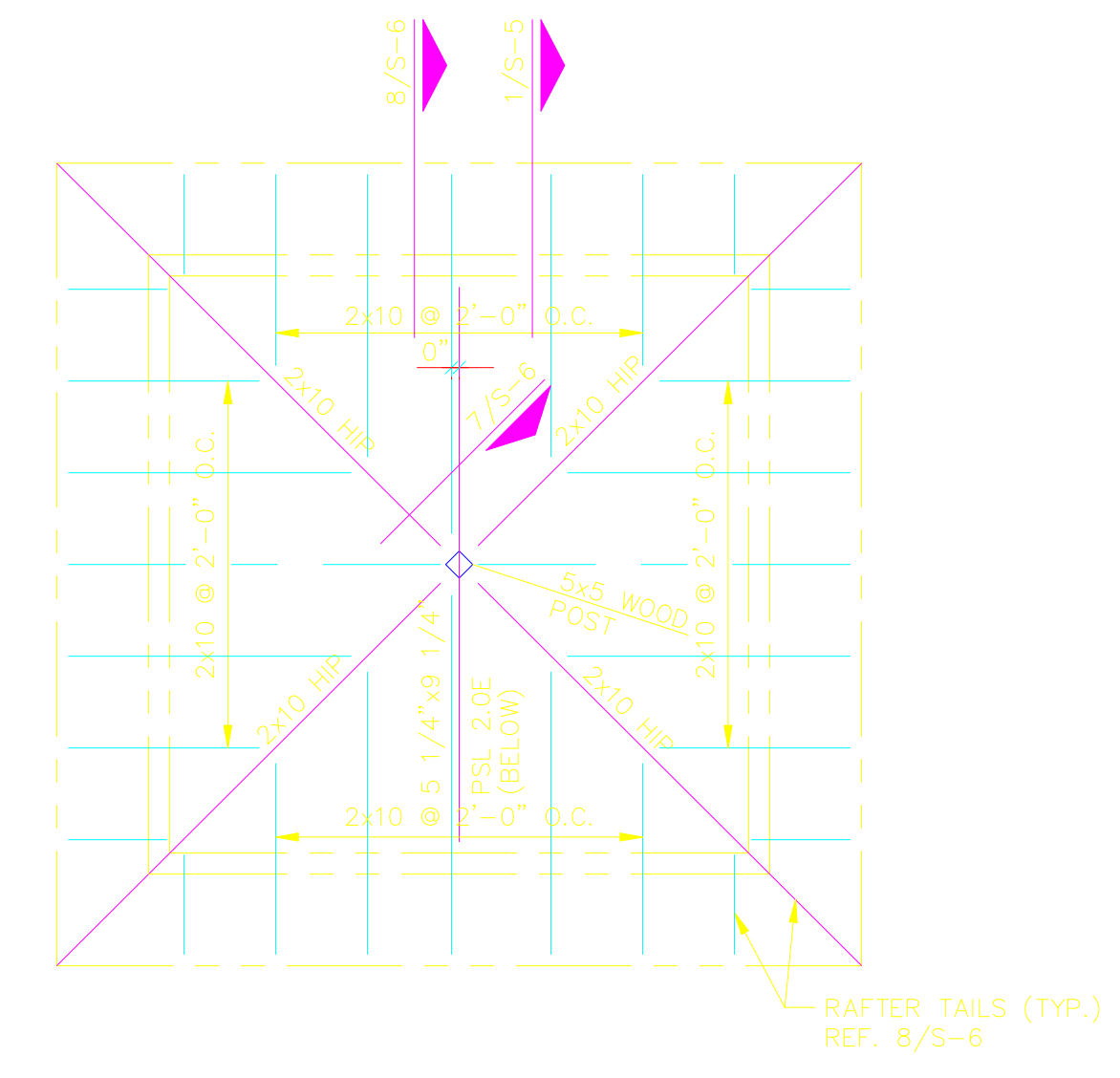
RAFTER TAILS (TYP.)  
REF. 8/S-6



**1 SHEAR WALL ELEVATION**  
 NOT TO SCALE

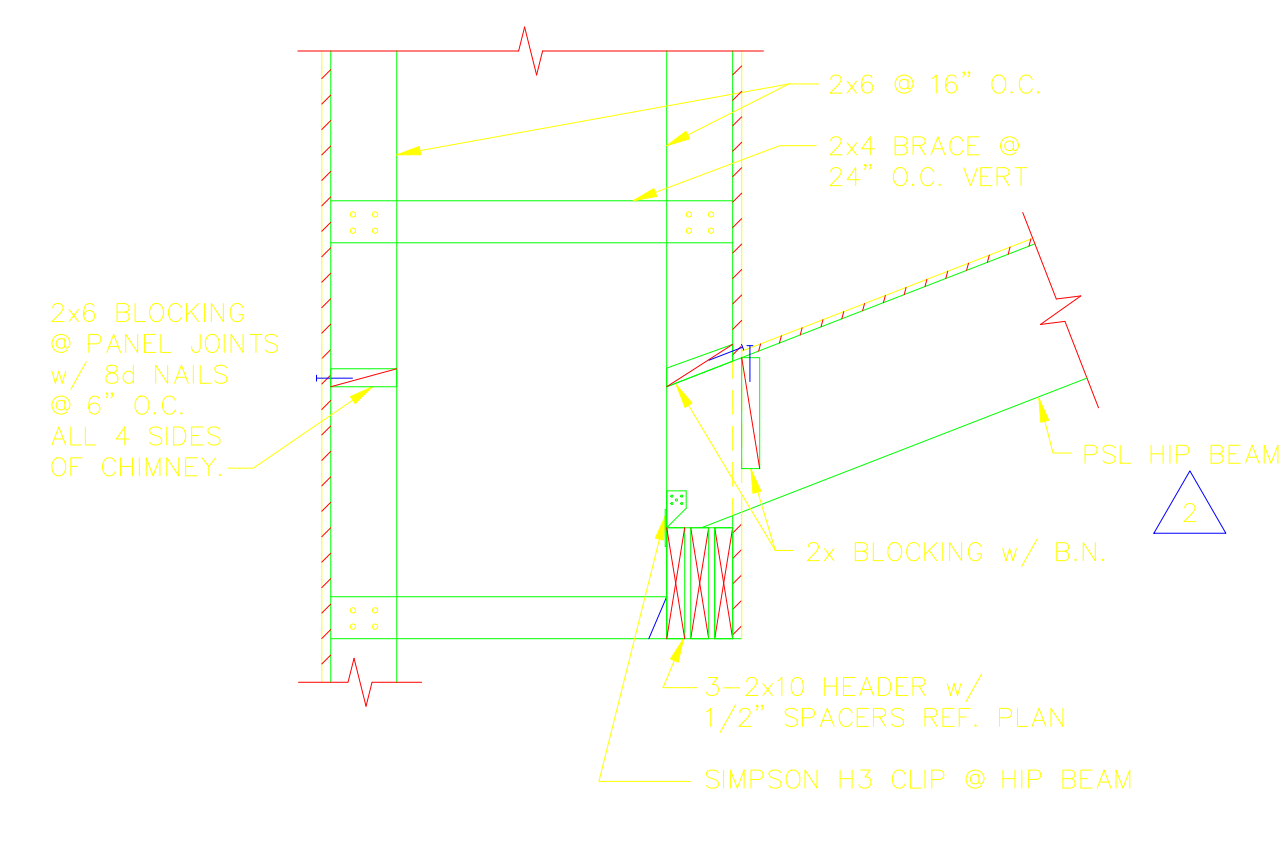
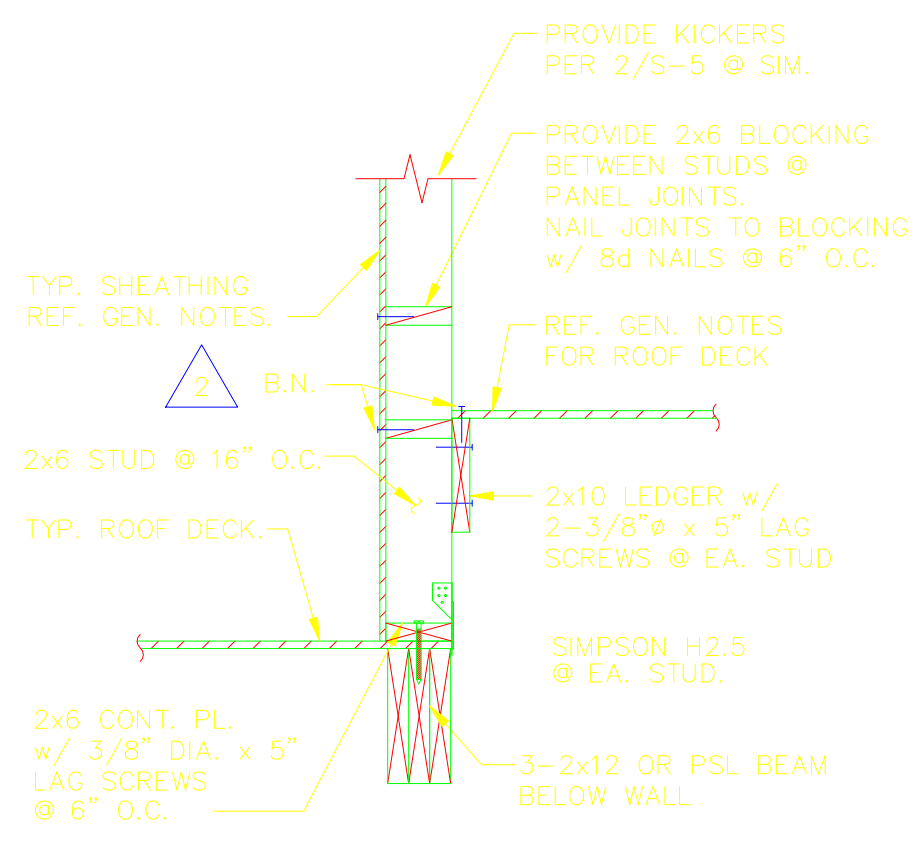
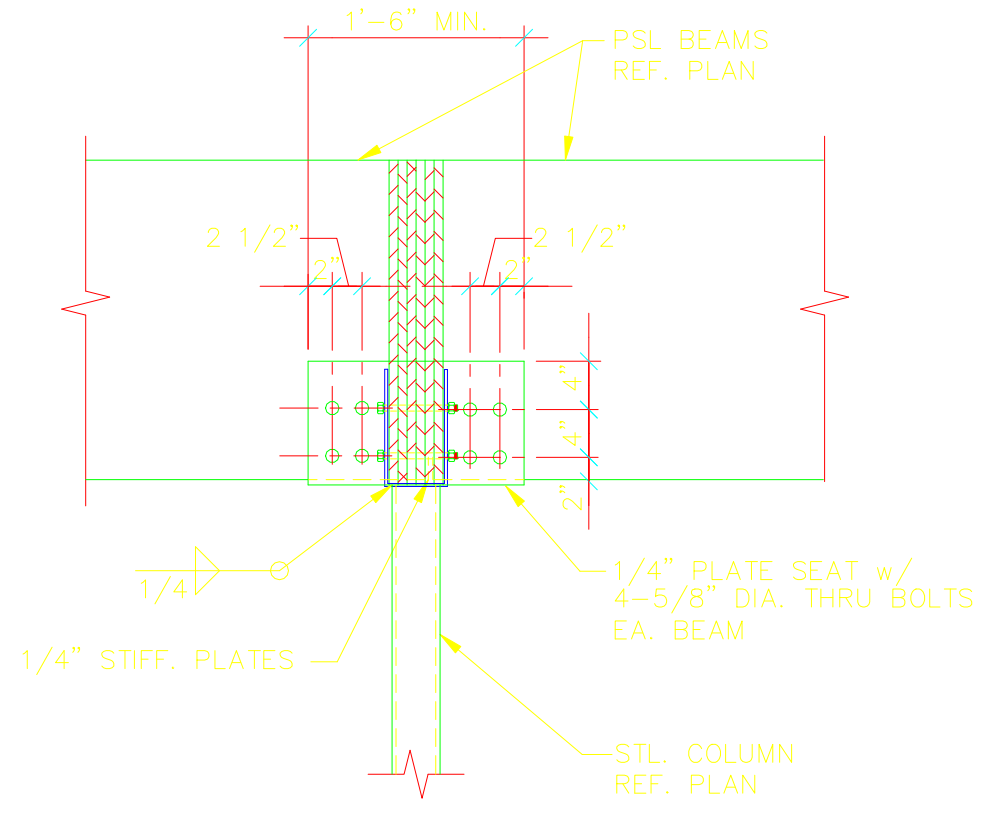
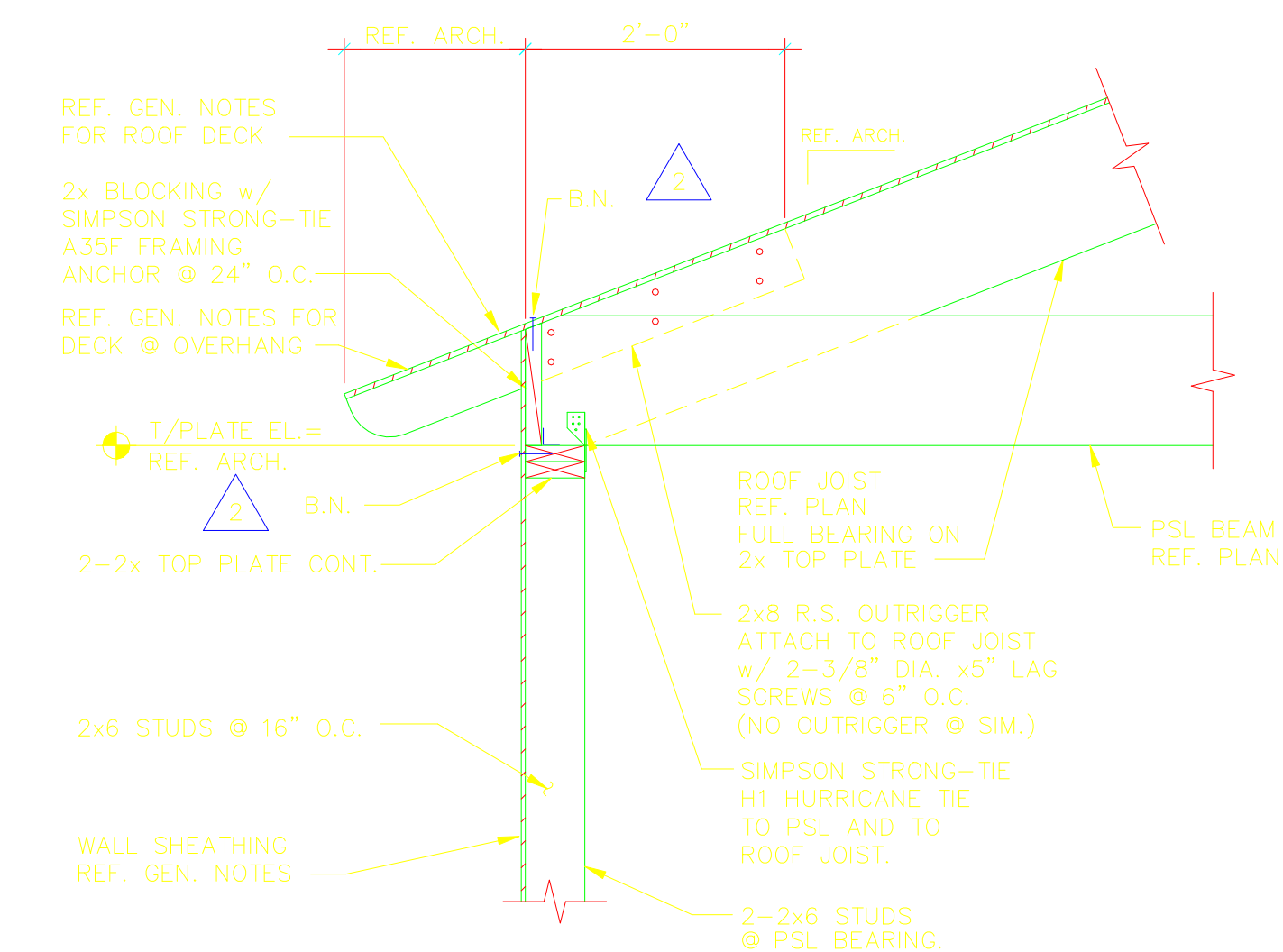
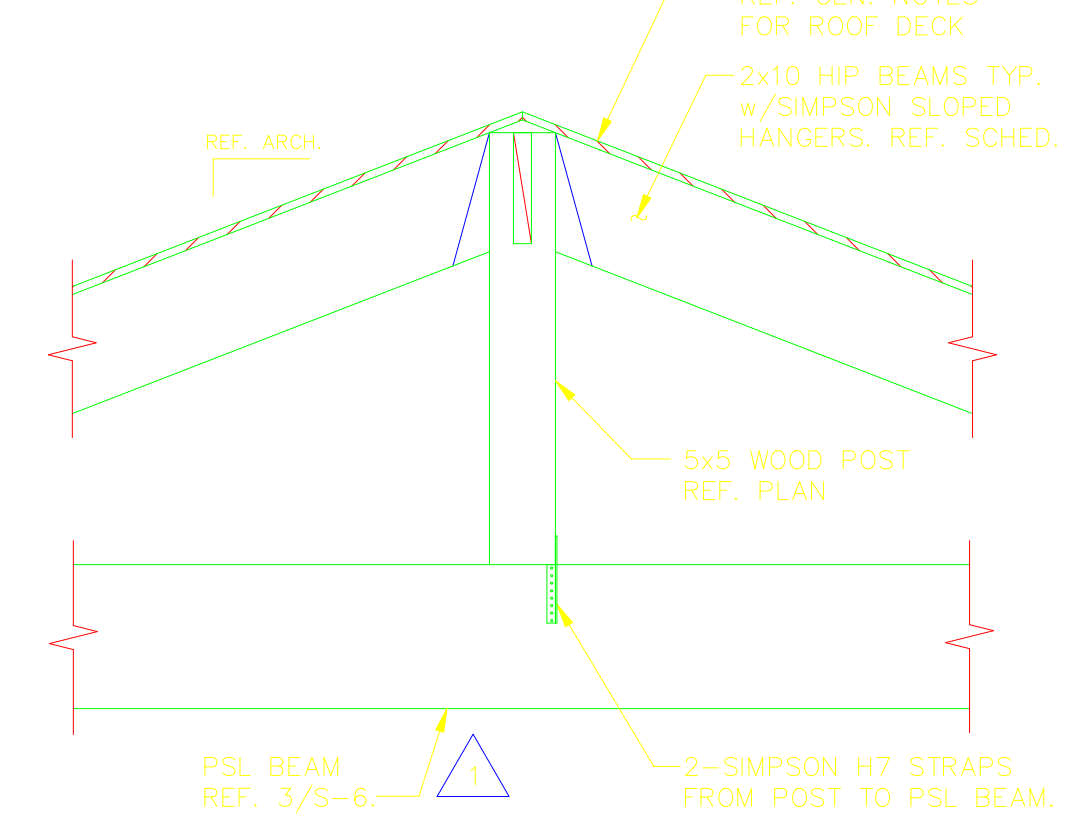
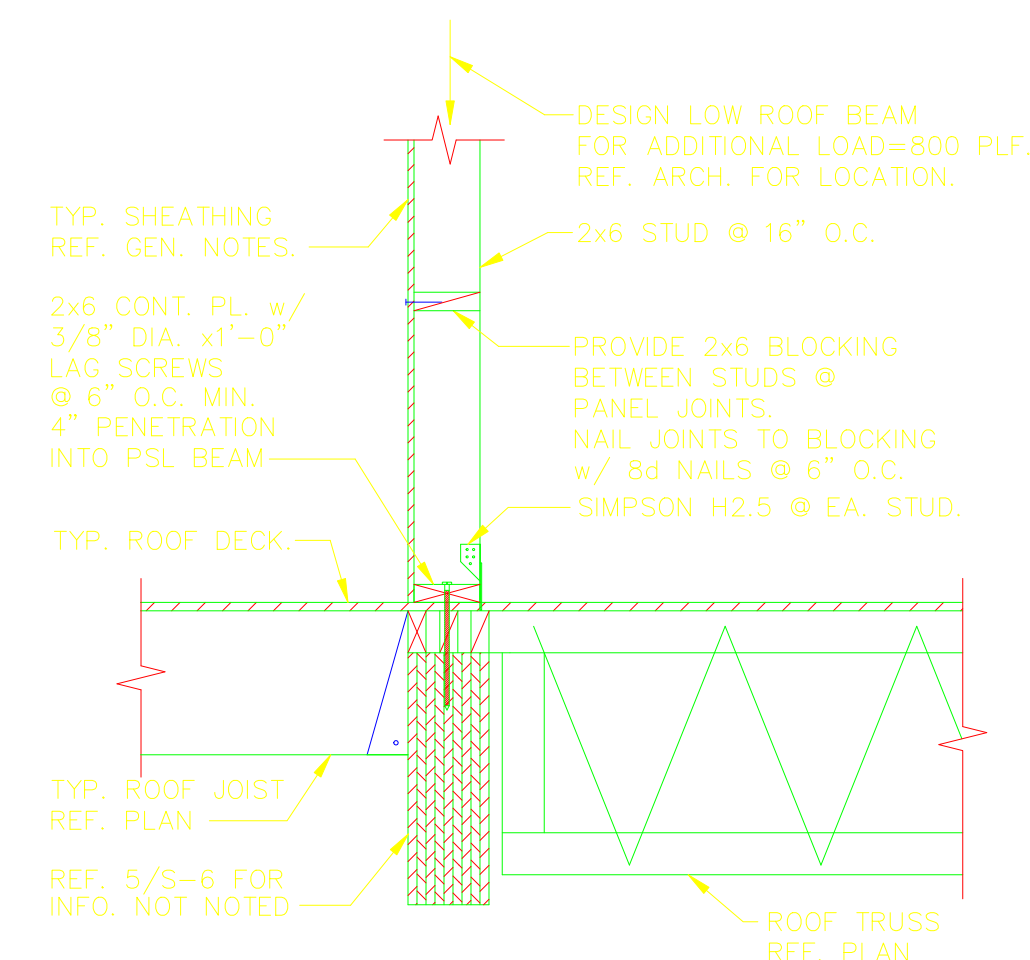
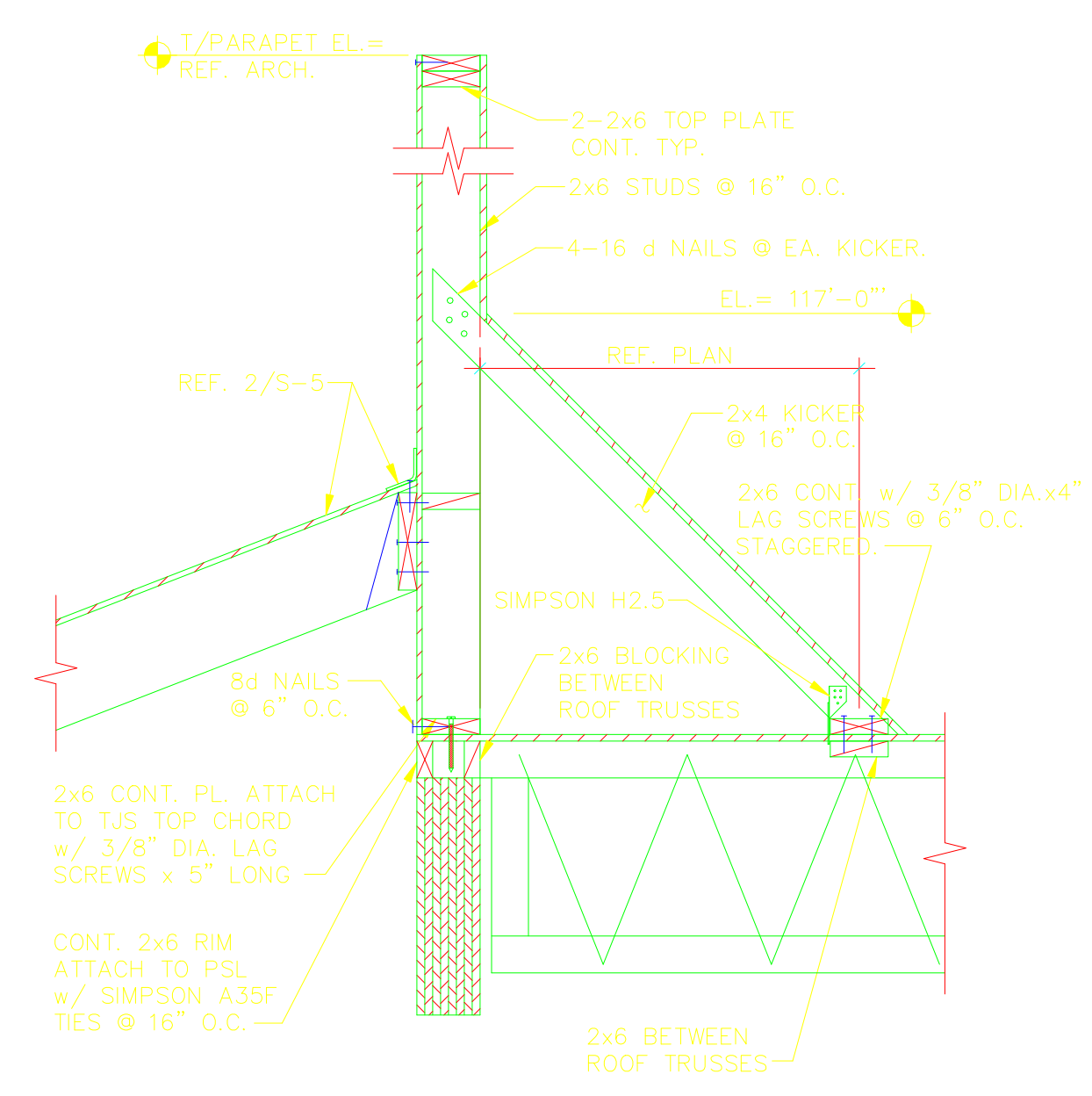


**2 T1 TRUSS PROFILE**  
 NOT TO SCALE



**SIMPSON HOLD-DOWN SCHEDULE**

SHEARWALL MARK	SIMPSON HOLD-DOWN	ANCHOR BOLT	MIN. EMBED
S1	PHDS	SSTB 24	1'-8 5/8"
S2	HD10A	SSTB 34	2'-4 7/8"



PERMIT ISSUE:  
 22 SEPT. 2003

REVISIONS:  
 28 OCT. 2003 ADDENDUM #1  
 12 NOV. 2003 CITY COMMENTS  
 18 DEC. 2003 CITY COMMENTS  
 28 JAN. 2004 OWNER COMMENTS

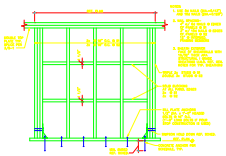
PROTO 90  
 HWY 287 AT PEARL RD.  
 CASA GRANDE, AZ



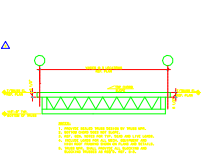
nca Architecture  
 6500 Greenville Ave  
 Suite 307  
 Dallas, TX 75206  
 214.361.9901  
 Fax 361.9906

**HGA** Hart, Gaugler & Associates, Inc.  
 Structural Engineering Services  
 12801 N. Central Expressway, #1400  
 Dallas, TX 75243  
 972-239-5111 / 972-239-5055 (Fax)

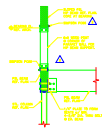
SHEET NO. S-6  
 DRAWN: JLL DESIGN: JEK HGA JOB # 203180



1 SHEAR WALL ELEVATION



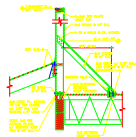
2 T1 TRUSS PROFILE



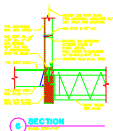
4 SECTION

**SIMPSON HOLD-DOWN SCHEDULE**

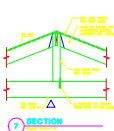
Anchor	Wind Speed (mph)	Height (ft)	Zone
SD-18	100	10	1
SD-24	100	10	2
SD-36	100	10	3
SD-48	100	10	4
SD-60	100	10	5
SD-72	100	10	6
SD-96	100	10	7
SD-120	100	10	8
SD-144	100	10	9
SD-180	100	10	10
SD-216	100	10	11
SD-240	100	10	12
SD-300	100	10	13
SD-360	100	10	14
SD-420	100	10	15
SD-480	100	10	16
SD-540	100	10	17
SD-600	100	10	18
SD-720	100	10	19
SD-840	100	10	20
SD-960	100	10	21
SD-1080	100	10	22
SD-1200	100	10	23
SD-1320	100	10	24
SD-1440	100	10	25
SD-1560	100	10	26
SD-1680	100	10	27
SD-1800	100	10	28
SD-1920	100	10	29
SD-2040	100	10	30
SD-2160	100	10	31
SD-2280	100	10	32
SD-2400	100	10	33
SD-2520	100	10	34
SD-2640	100	10	35
SD-2760	100	10	36
SD-2880	100	10	37
SD-3000	100	10	38
SD-3120	100	10	39
SD-3240	100	10	40
SD-3360	100	10	41
SD-3480	100	10	42
SD-3600	100	10	43
SD-3720	100	10	44
SD-3840	100	10	45
SD-3960	100	10	46
SD-4080	100	10	47
SD-4200	100	10	48
SD-4320	100	10	49
SD-4440	100	10	50
SD-4560	100	10	51
SD-4680	100	10	52
SD-4800	100	10	53
SD-4920	100	10	54
SD-5040	100	10	55
SD-5160	100	10	56
SD-5280	100	10	57
SD-5400	100	10	58
SD-5520	100	10	59
SD-5640	100	10	60
SD-5760	100	10	61
SD-5880	100	10	62
SD-6000	100	10	63
SD-6120	100	10	64
SD-6240	100	10	65
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SD-6480	100	10	67
SD-6600	100	10	68
SD-6720	100	10	69
SD-6840	100	10	70
SD-6960	100	10	71
SD-7080	100	10	72
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SD-7320	100	10	74
SD-7440	100	10	75
SD-7560	100	10	76
SD-7680	100	10	77
SD-7800	100	10	78
SD-7920	100	10	79
SD-8040	100	10	80
SD-8160	100	10	81
SD-8280	100	10	82
SD-8400	100	10	83
SD-8520	100	10	84
SD-8640	100	10	85
SD-8760	100	10	86
SD-8880	100	10	87
SD-9000	100	10	88
SD-9120	100	10	89
SD-9240	100	10	90
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SD-10440	100	10	100



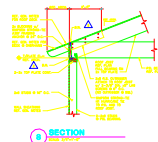
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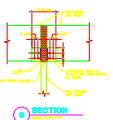
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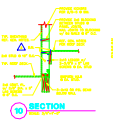
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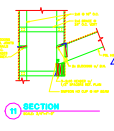
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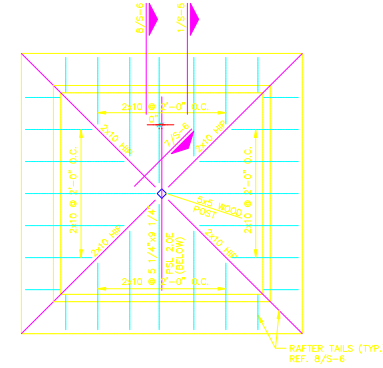
9 SECTION



10 SECTION

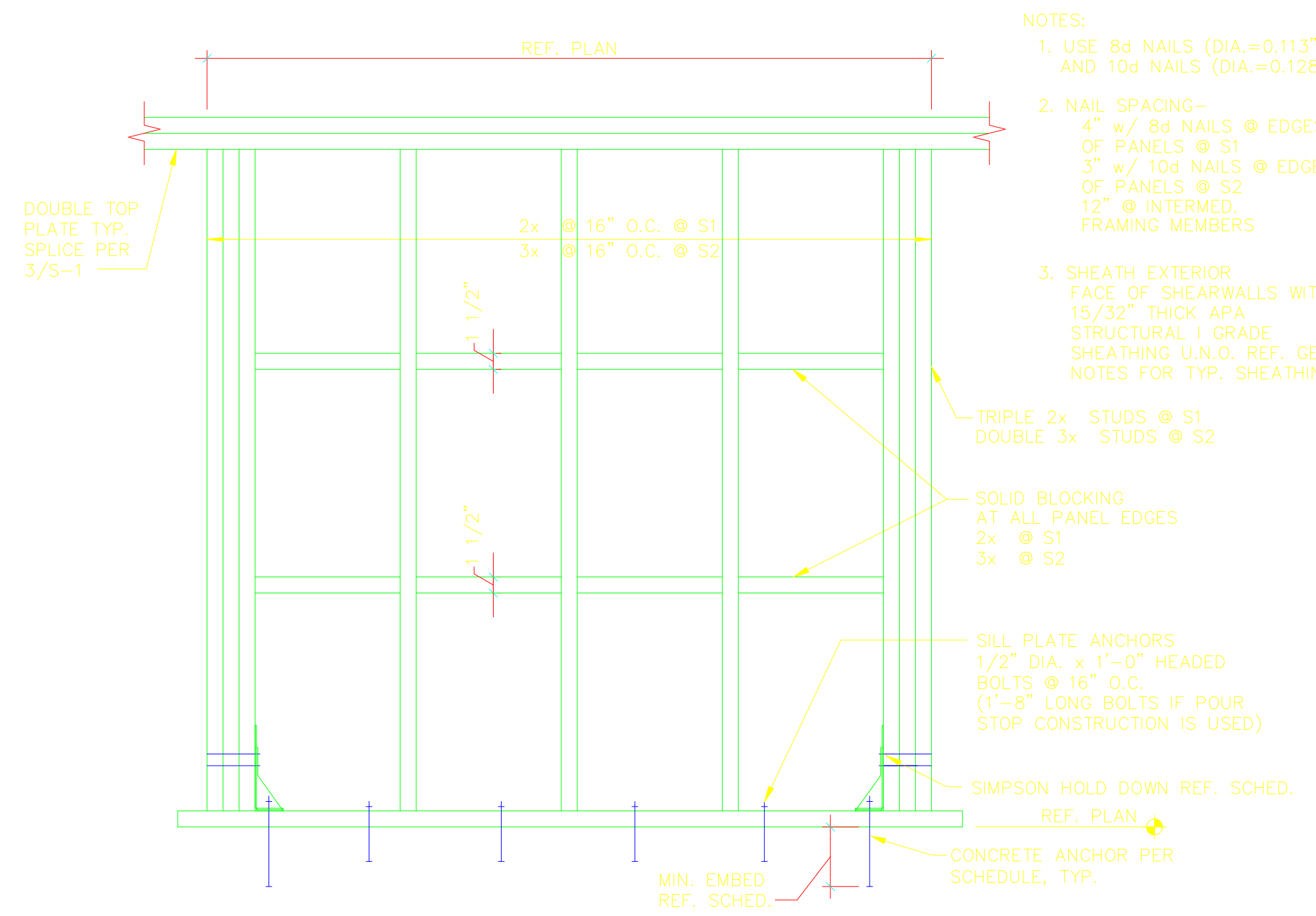


11 SECTION



3 PARTIAL PLAN @ HIGH ROOF FRAMING  
SCALE: 1/4"=1'-0"

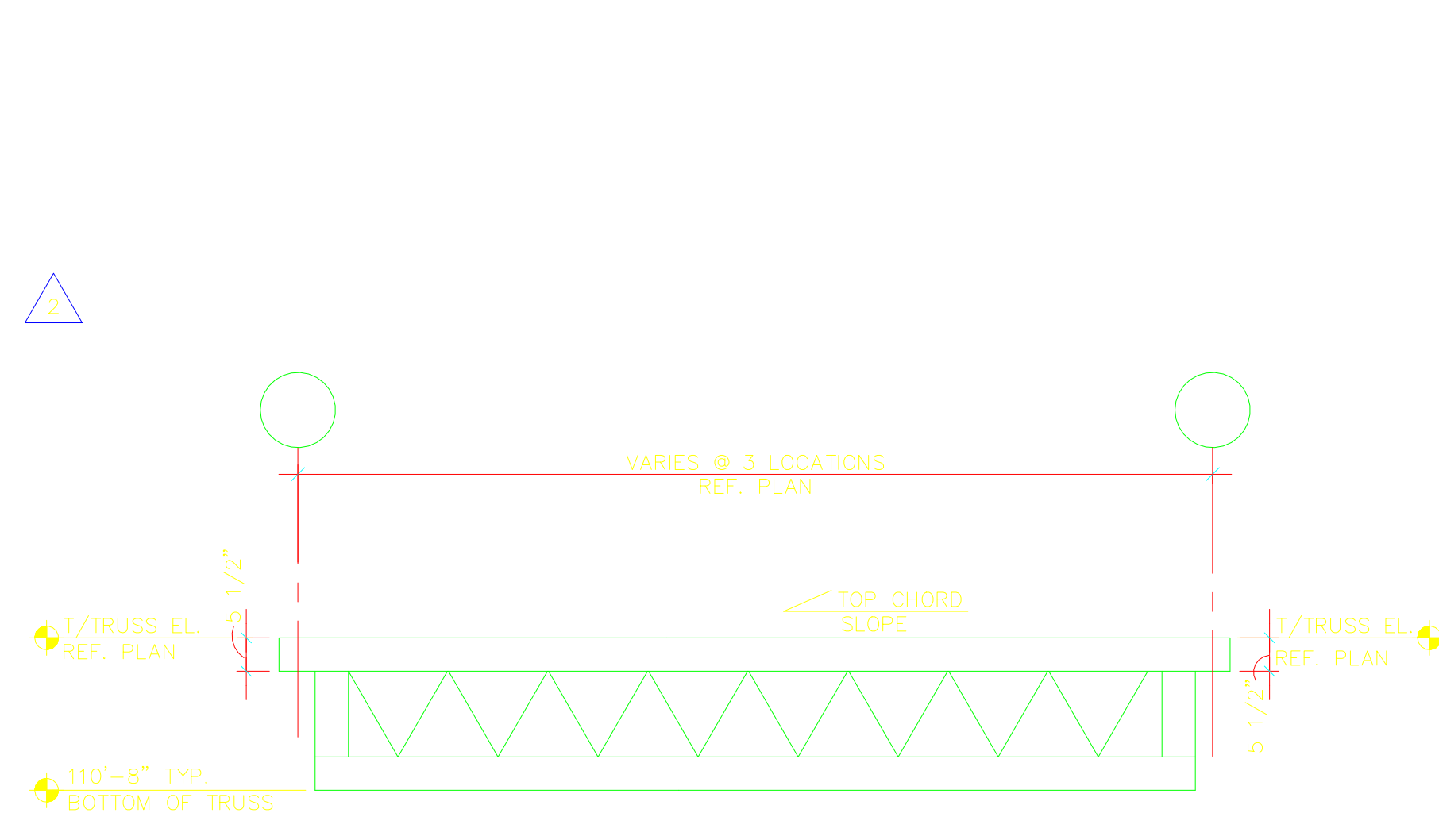




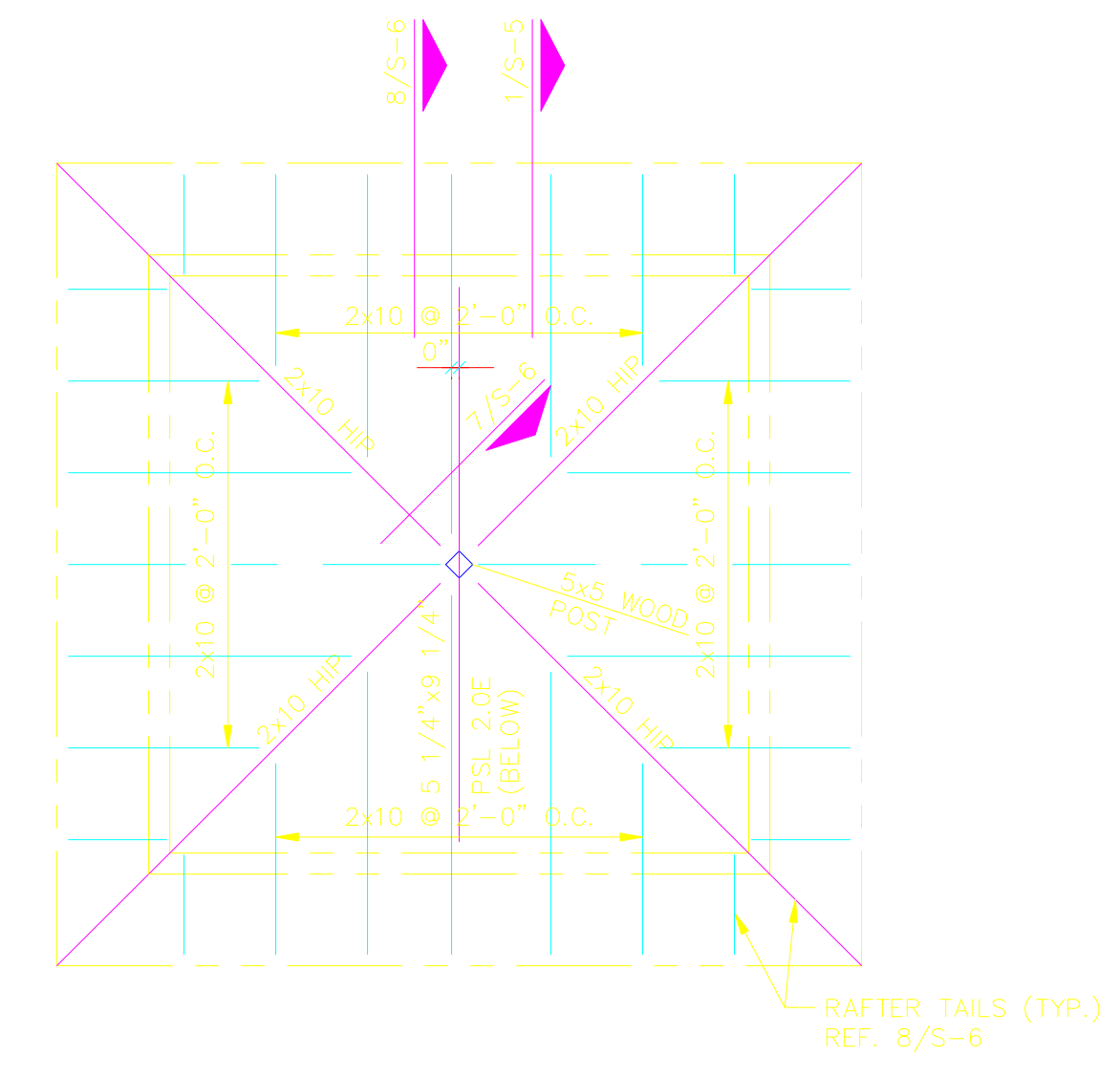
**1 SHEAR WALL ELEVATION**  
NOT TO SCALE

**SIMPSON HOLD-DOWN SCHEDULE**

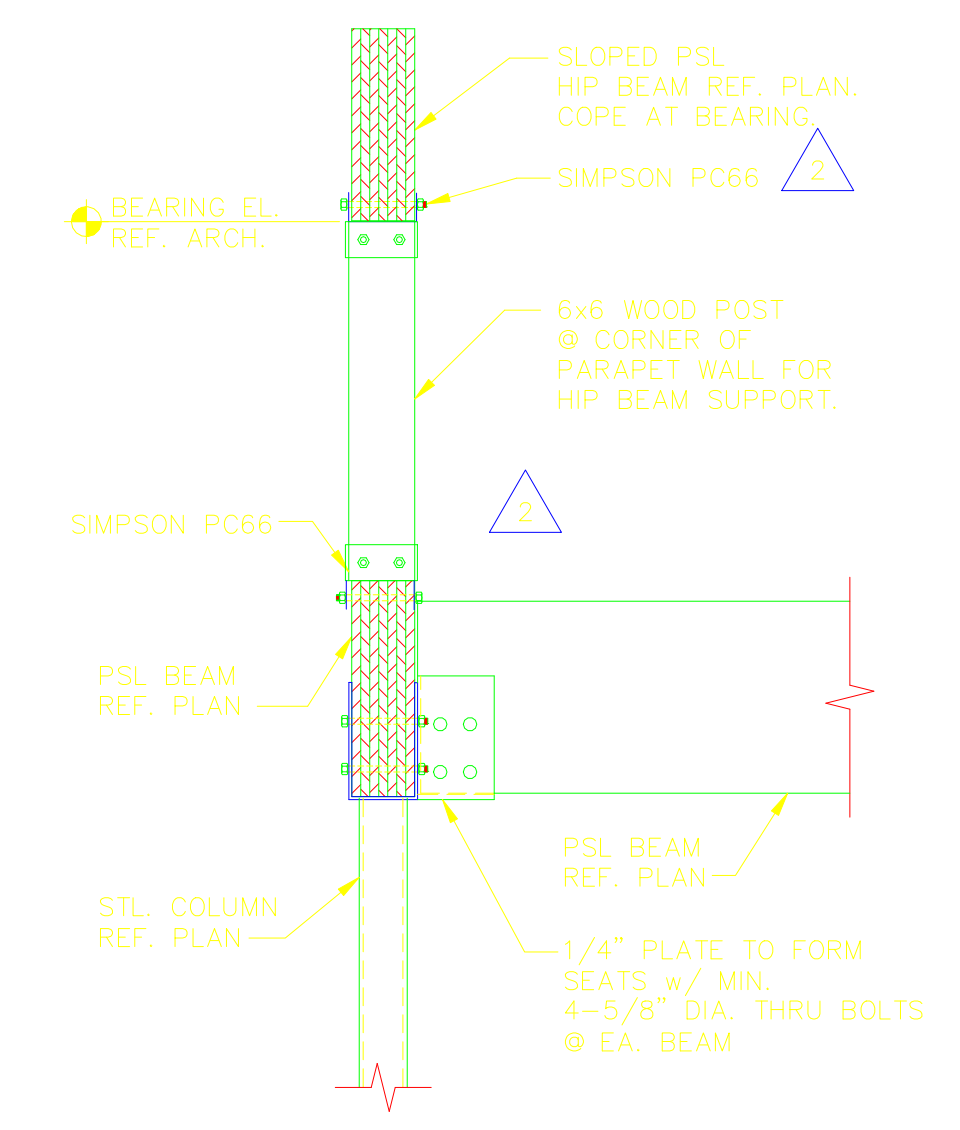
SHEARWALL MARK	SIMPSON HOLD-DOWN	ANCHOR BOLT	MIN. EMBED
S1	PHDS	SSTB 24	1'-8 5/8"
S2	HD10A	SSTB 34	2'-4 7/8"



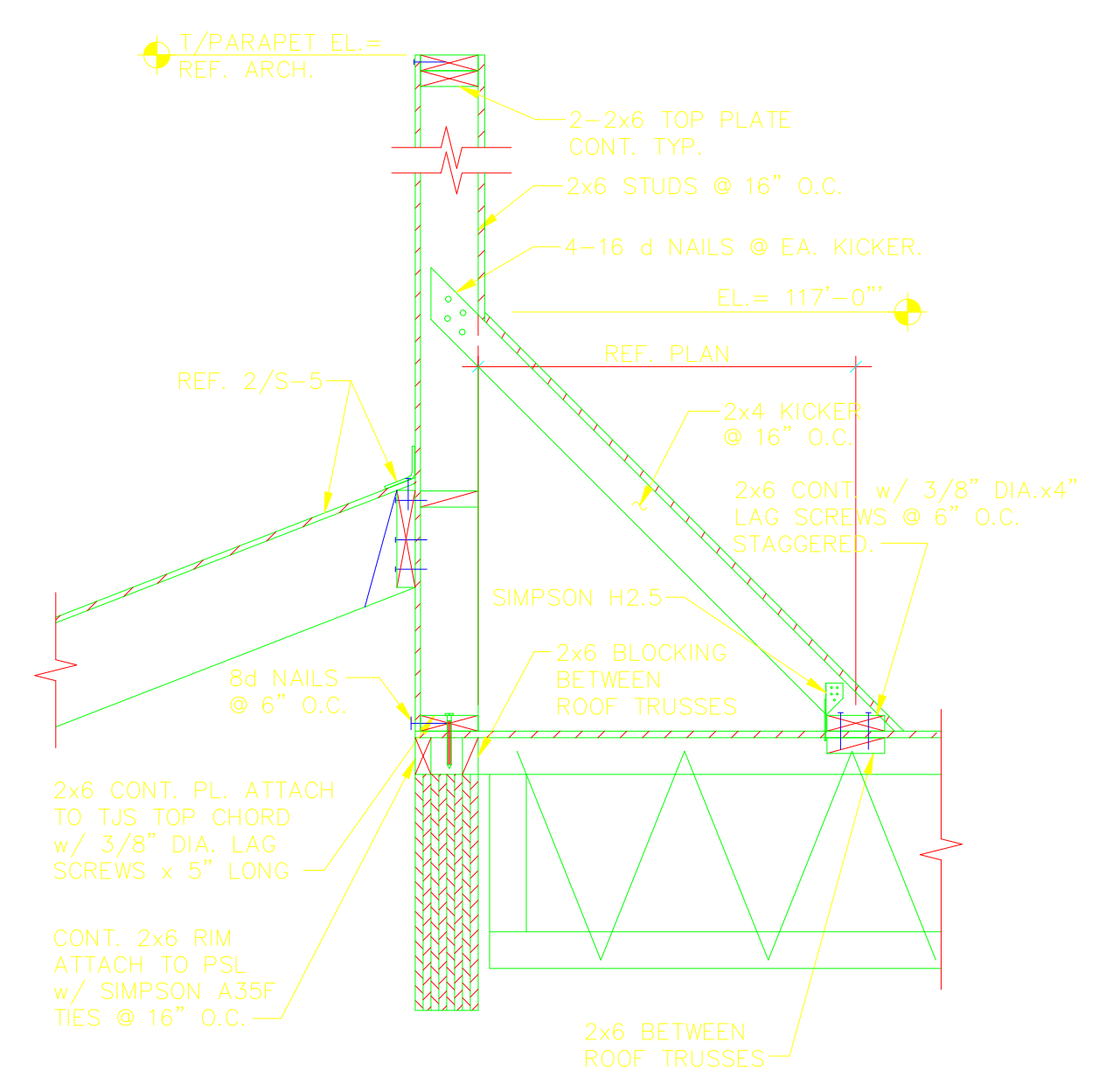
**2 T1 TRUSS PROFILE**  
NOT TO SCALE



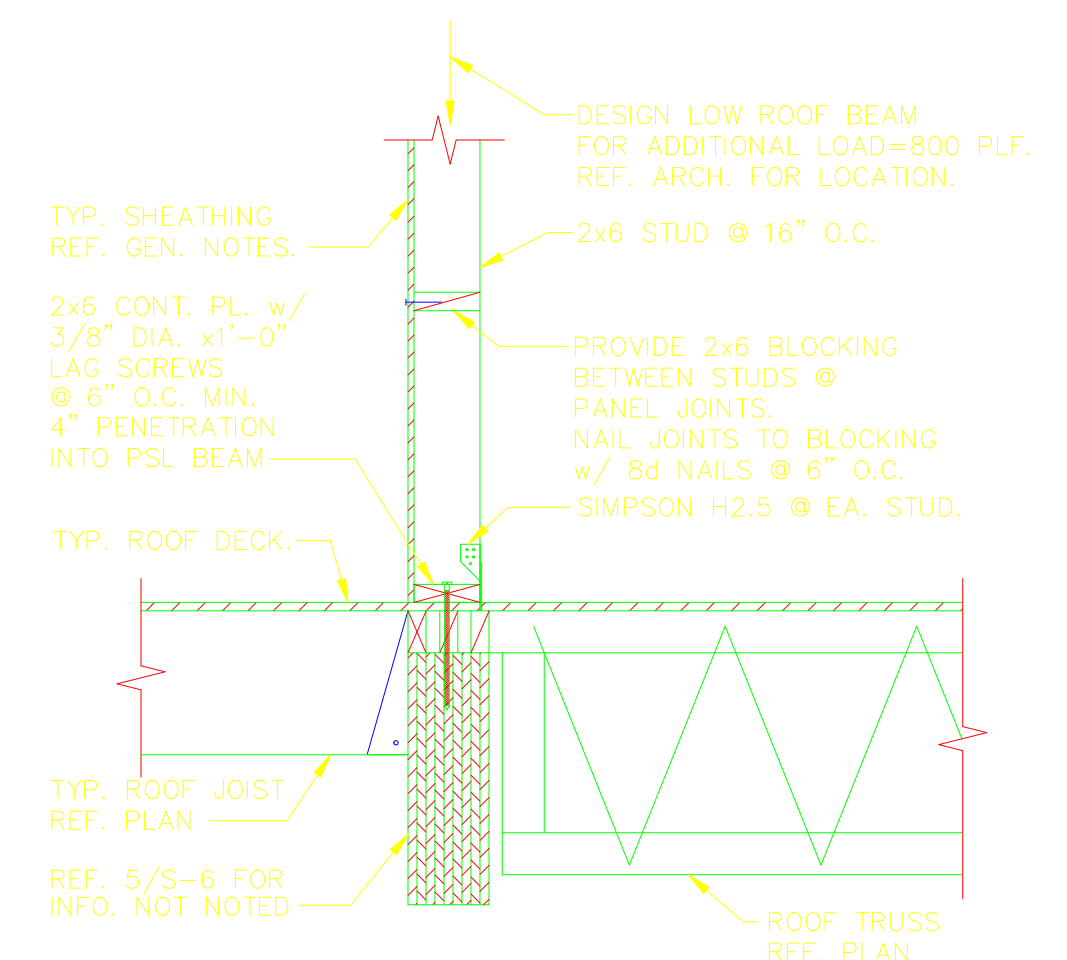
**3 PARTIAL PLAN @ HIGH ROOF FRAMING**  
SCALE: 1/4"=1'-0"



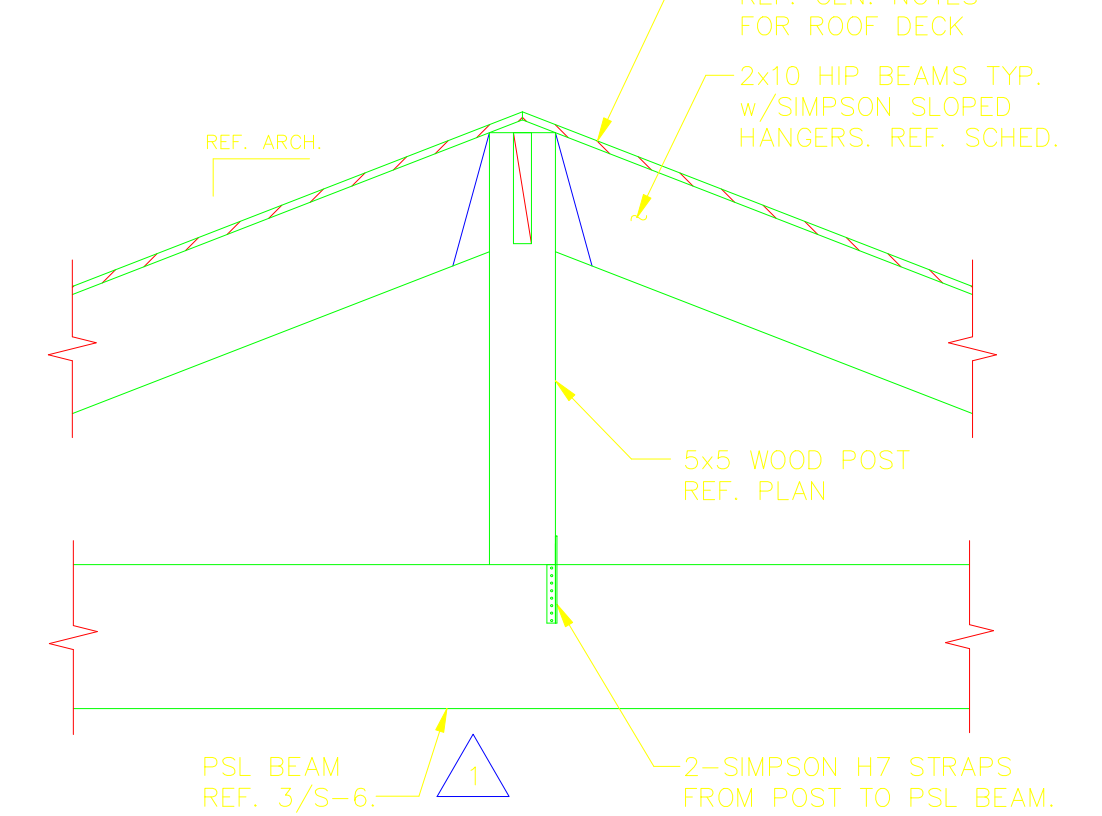
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SCALE: 3/4"=1'-0"



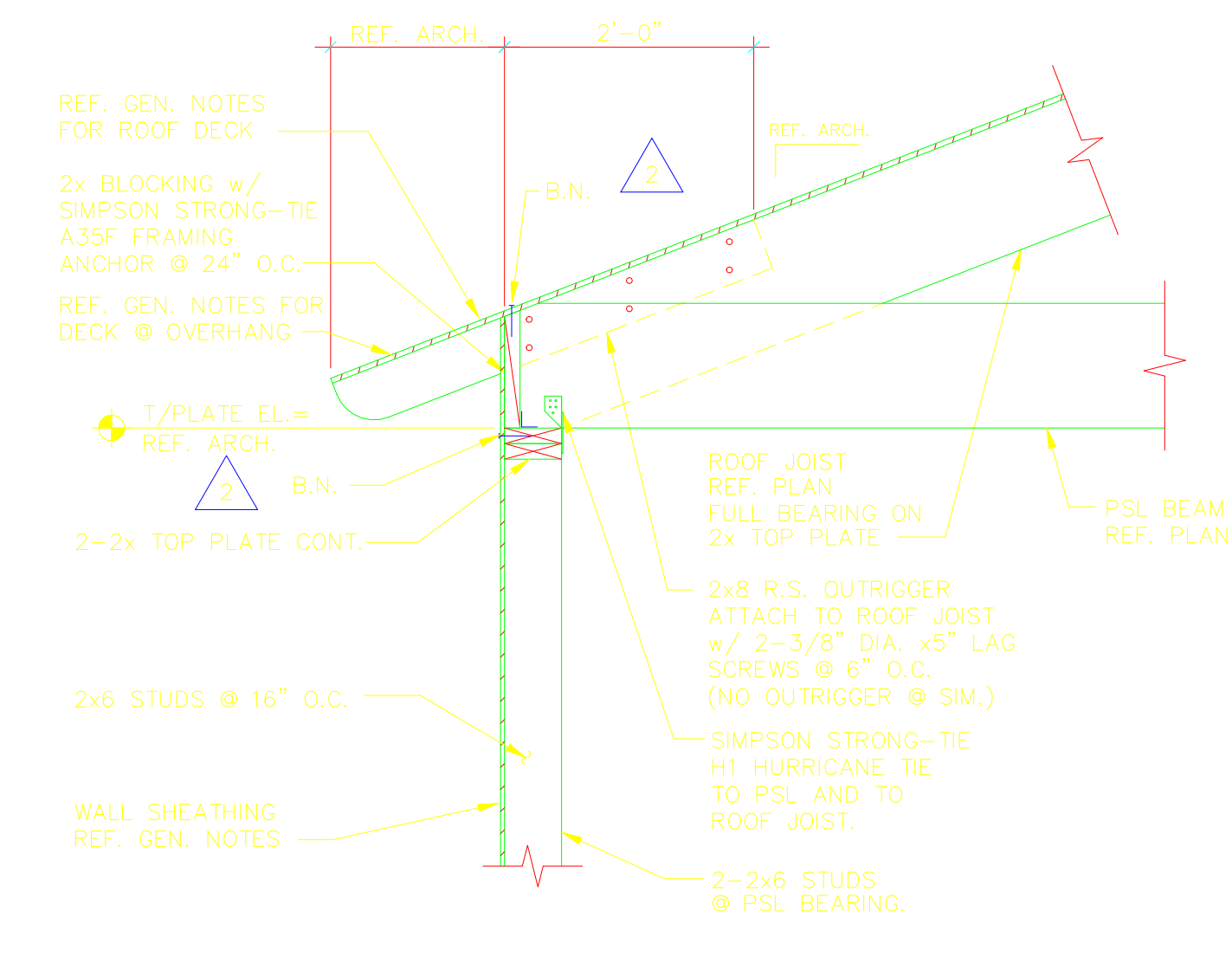
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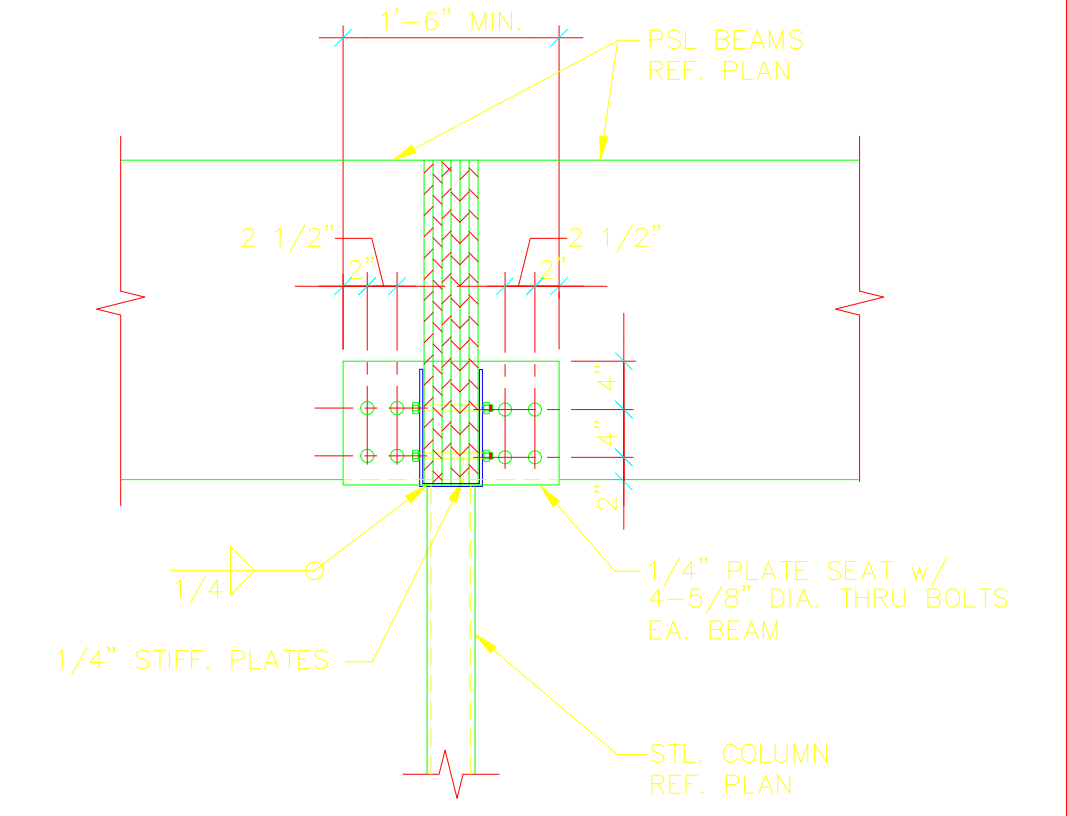
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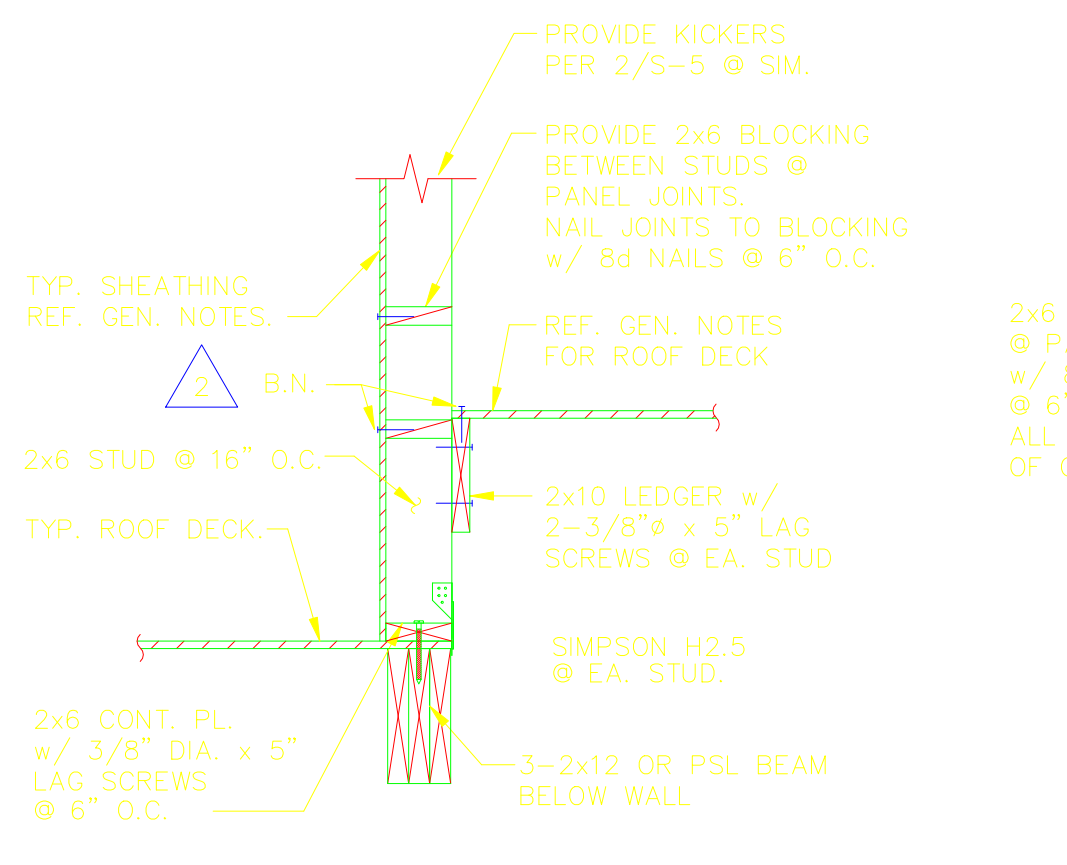
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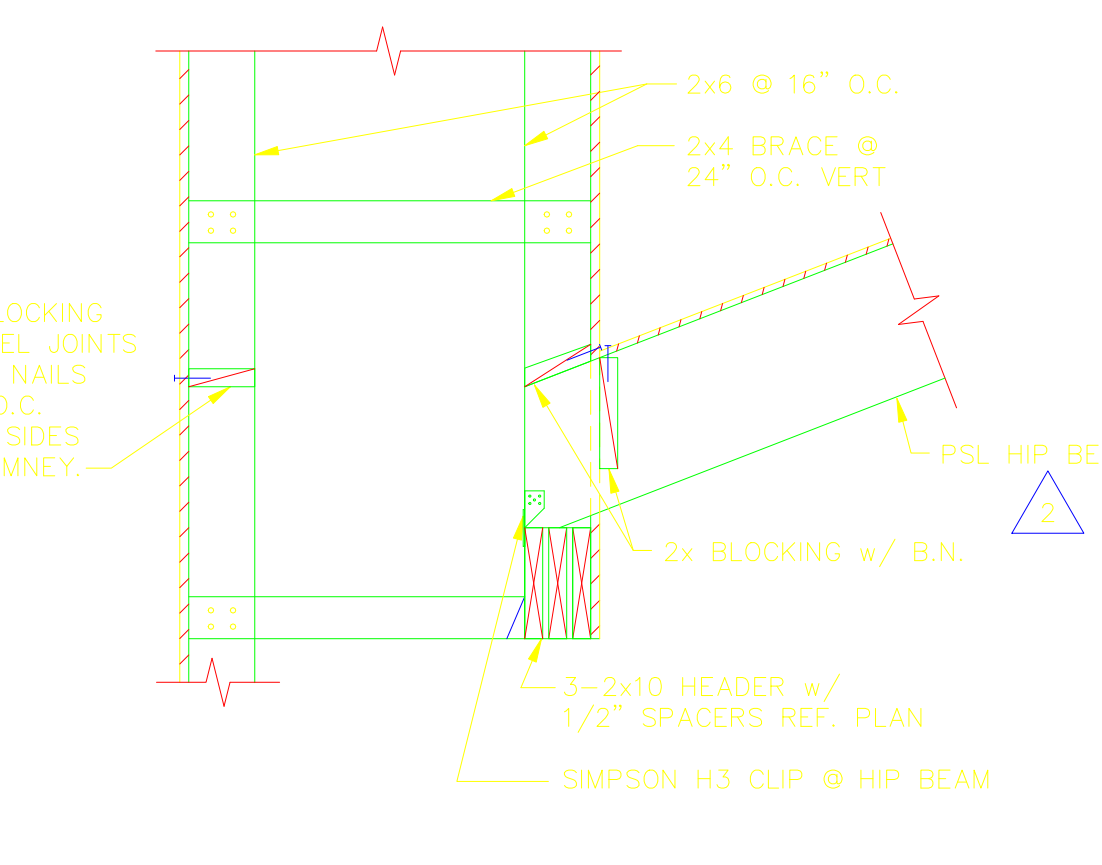
**8 SECTION**  
SCALE: 3/4"=1'-0"



**9 SECTION**  
SCALE: 3/4"=1'-0"



**10 SECTION**  
SCALE: 3/4"=1'-0"



**11 SECTION**  
SCALE: 3/4"=1'-0"

PERMIT ISSUE:

22 SEPT. 2003

REVISIONS:

28 OCT. 2003	ADDENDUM #1
12 NOV. 2003	CITY COMMENTS
18 DEC. 2003	CITY COMMENTS
28 JAN. 2004	OWNER COMMENTS

PROTO 90  
 HWY 287 AT PEARL RD.  
 CASA GRANDE, AZ

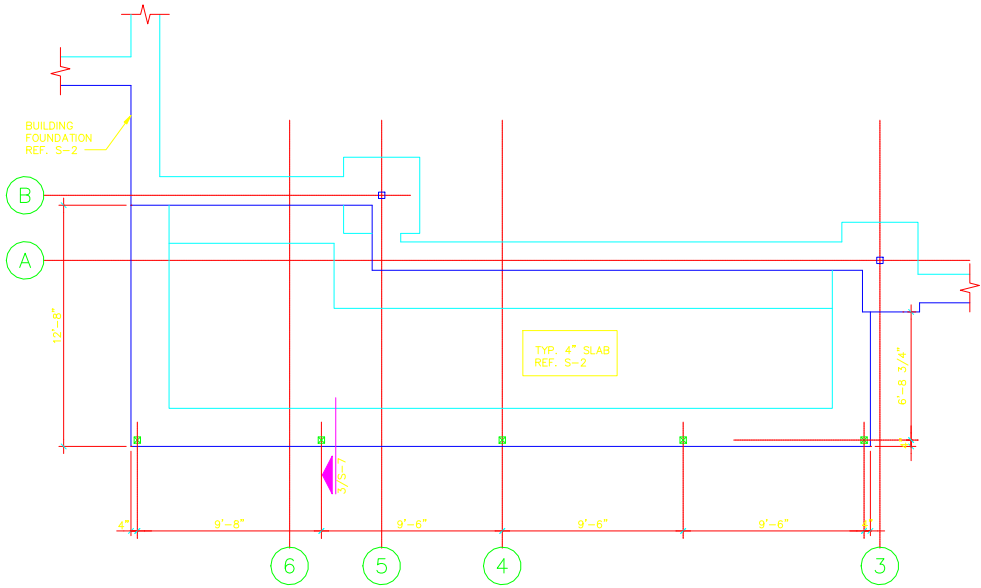


nca Architecture  
 8500 Greenville Ave  
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BRAWN JLL DESIGN JEK HGA JOB # 203180

SHEET NO: S-6

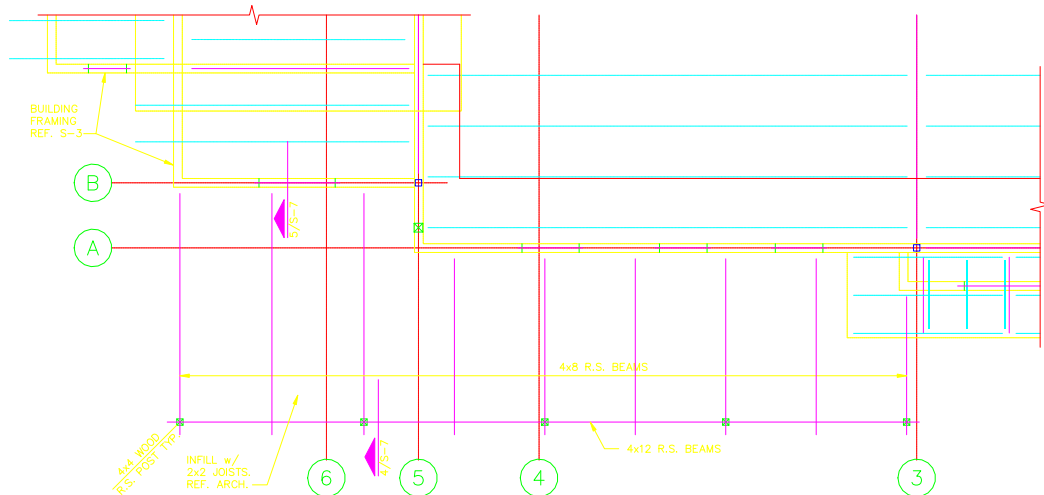


# 1 FOUNDATION PLAN @ PATIO

SCALE: 1/4"=1'-0"

FOUNDATION PLAN NOTES:

1. REF. CIVIL FOR ACTUAL FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
2. REF. GEN. NOTES FOR SUBGRADE PREPARATIONS.

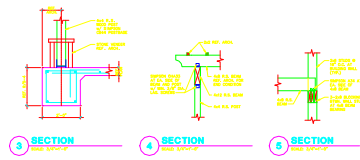


# 2 ROOF FRAMING PLAN @ PATIO

SCALE: 1/4"=1'-0"

ROOF FRAMING PLAN NOTES:

1. DATUM EL. = 100'-0". REF. CIVIL FOR ACTUAL F.F. EL.
2. ROOF JOISTS NOTED R.S. ARE ROUGH SAWN FIR, REF. ARCH.

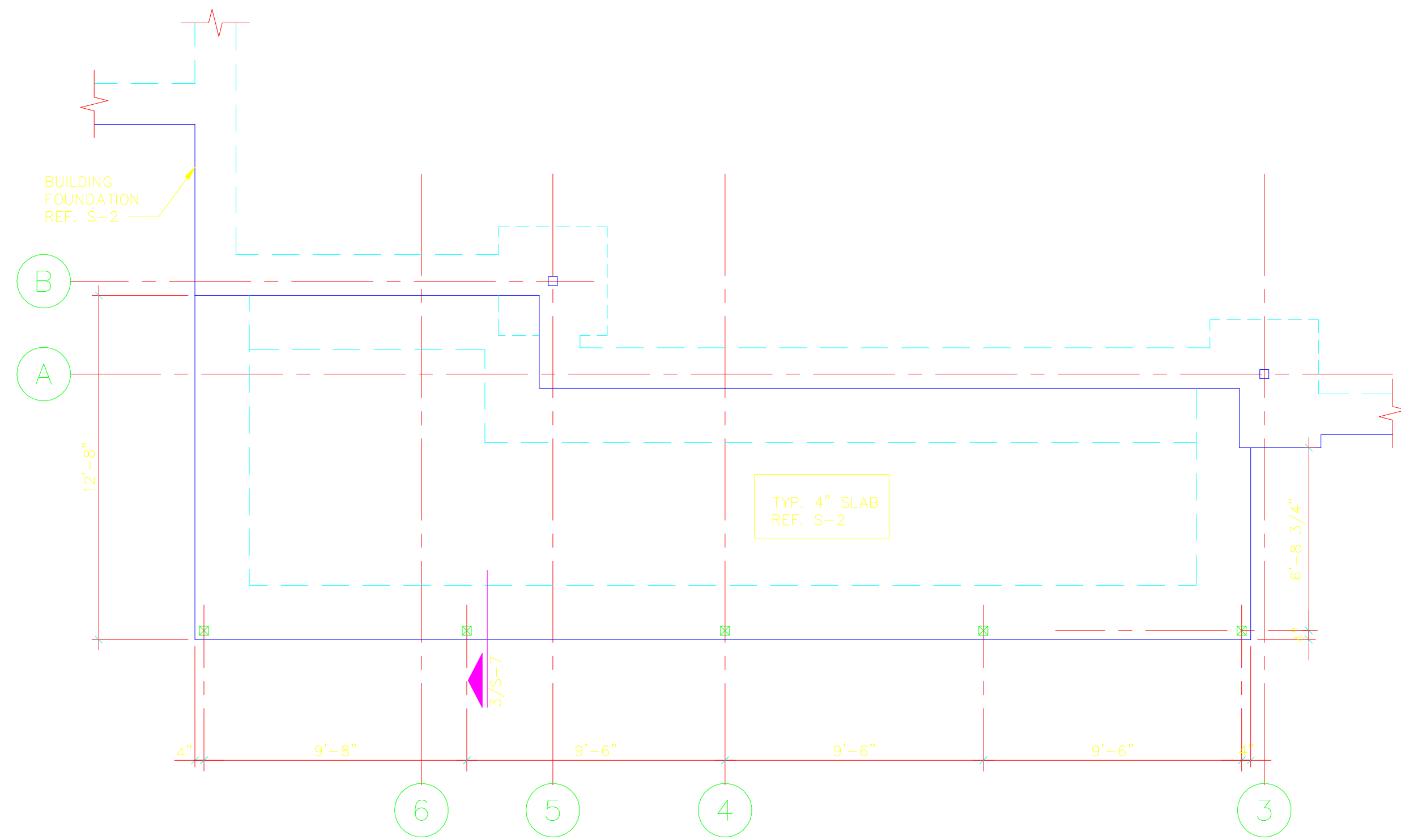


**SPECIAL REVISIONS**

1. REVISED FOR PERMITS AND TO ADD FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
2. REVISED FOR PERMITS AND TO ADD FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
3. REVISED FOR PERMITS AND TO ADD FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
4. REVISED FOR PERMITS AND TO ADD FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
5. REVISED FOR PERMITS AND TO ADD FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
6. REVISED FOR PERMITS AND TO ADD FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
7. REVISED FOR PERMITS AND TO ADD FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
8. REVISED FOR PERMITS AND TO ADD FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
9. REVISED FOR PERMITS AND TO ADD FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
10. REVISED FOR PERMITS AND TO ADD FINISH FLOOR ELEVATION AND SLOPE AT PATIO.

**SPECIAL REVISIONS**

1. REVISED FOR PERMITS AND TO ADD FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
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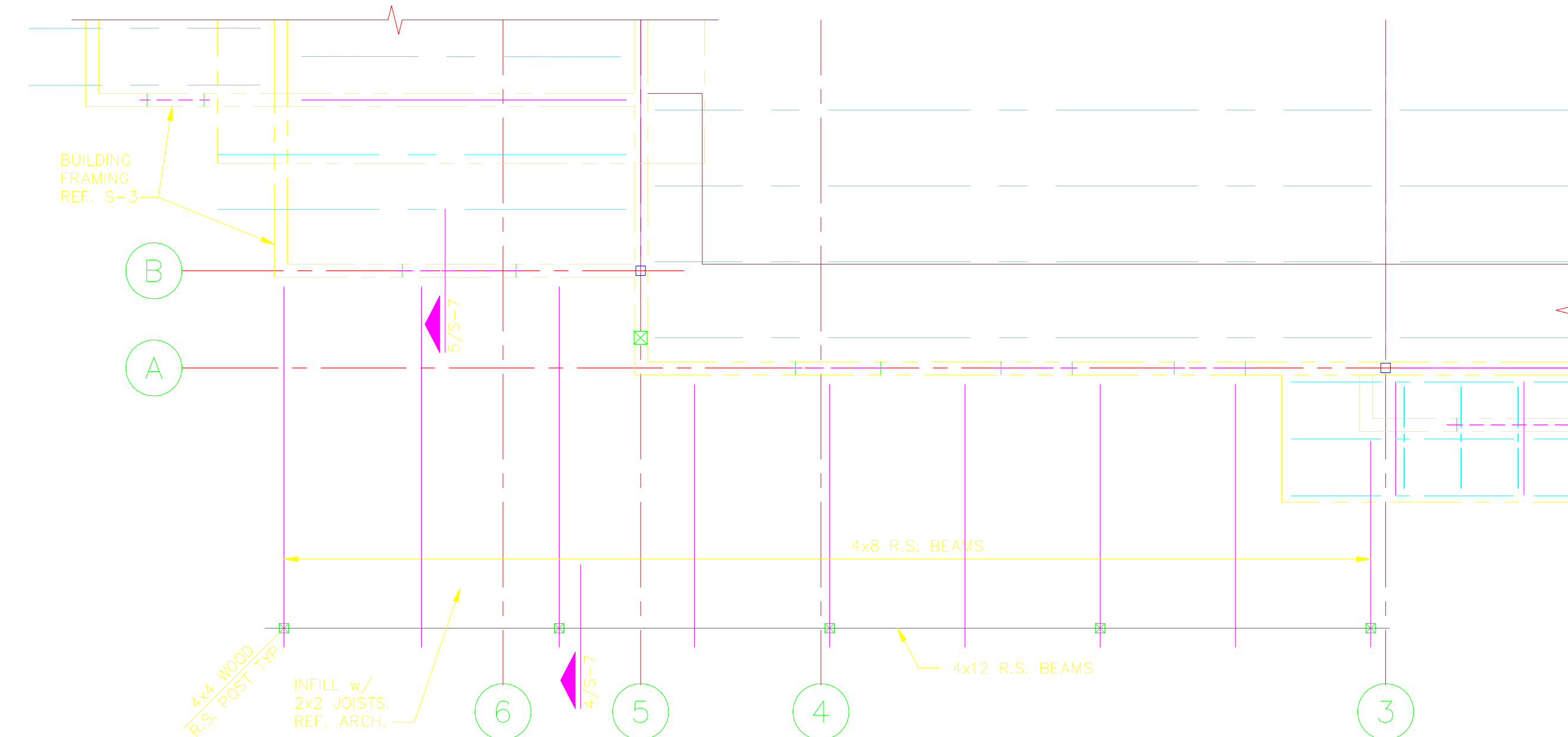


### 1 FOUNDATION PLAN @ PATIO

SCALE: 1/4"=1'-0"

#### FOUNDATION PLAN NOTES:

1. REF. CIVIL FOR ACTUAL FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
2. REF. GEN. NOTES FOR SUBGRADE PREPARATIONS.

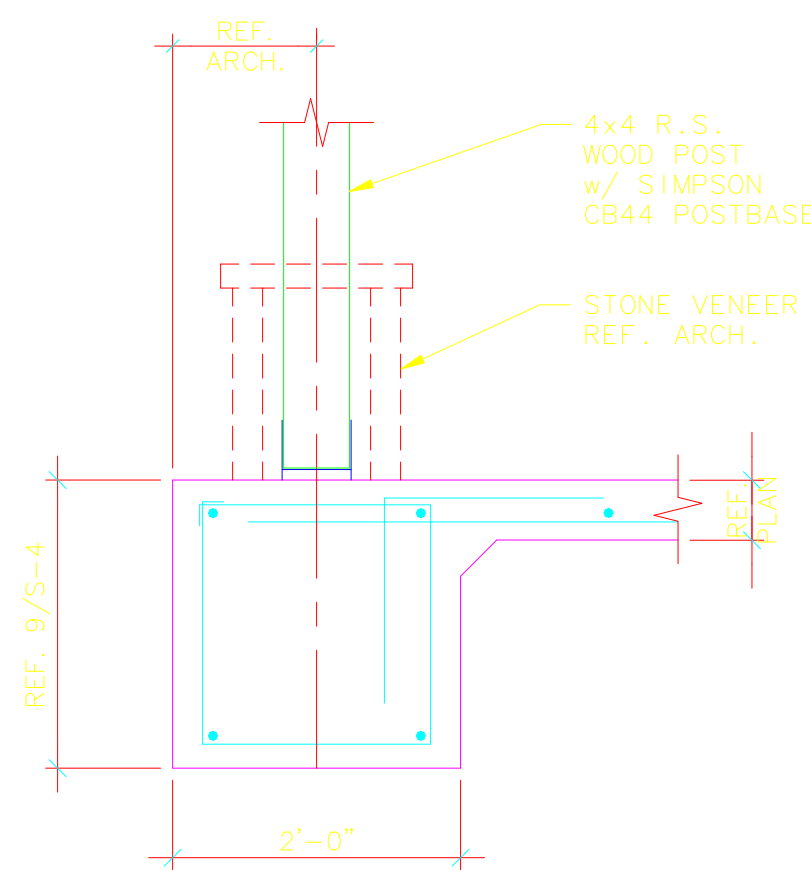


### 2 ROOF FRAMING PLAN @ PATIO

SCALE: 1/4"=1'-0"

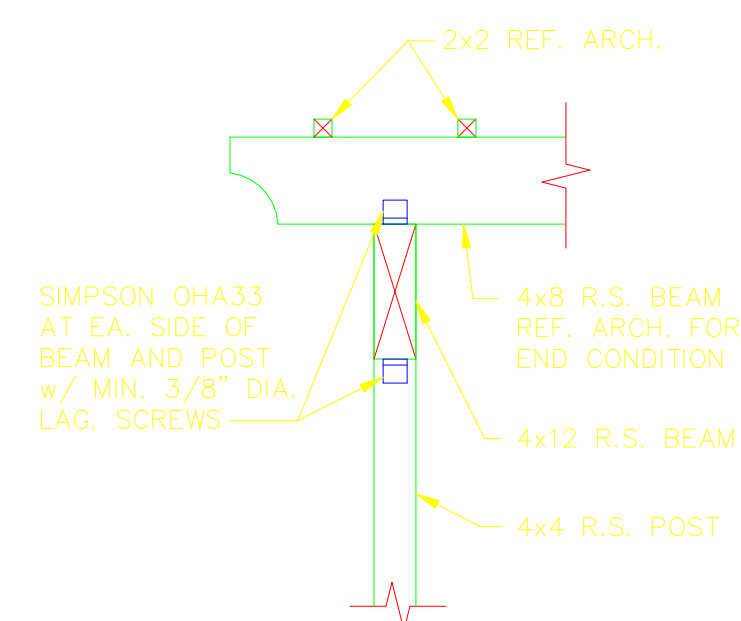
#### ROOF FRAMING PLAN NOTES:

1. DATUM EL. = 100'-0". REF. CIVIL FOR ACTUAL F.F. EL.
2. ROOF JOISTS NOTED R.S. ARE ROUGH SAWN FIR. REF. ARCH.



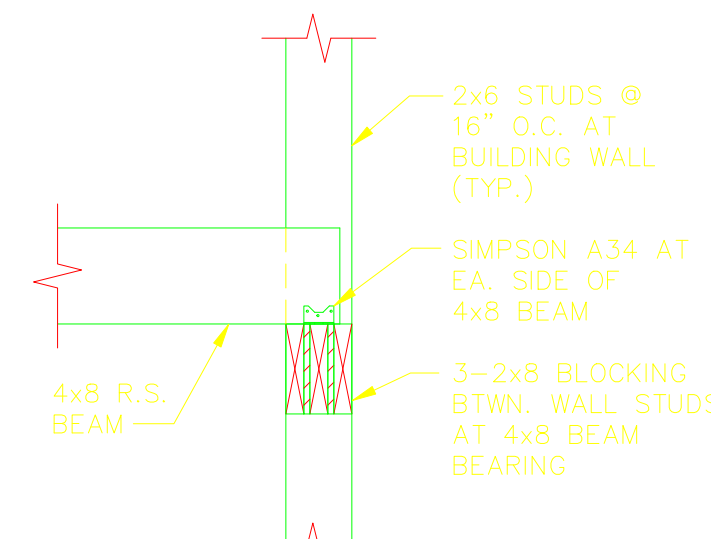
### 3 SECTION

SCALE: 3/4"=1'-0"



### 4 SECTION

SCALE: 3/4"=1'-0"



### 5 SECTION

SCALE: 3/4"=1'-0"

#### SPECIAL INSPECTIONS

1. A SPECIAL INSPECTOR SHALL BE ENGAGED TO MAKE SPECIAL INSPECTIONS OF THE CONSTRUCTION WORK AS OUTLINED BELOW IN CONFORMANCE WITH THE PROVISIONS OF CHAPTER 17 OF THE UNIFORM BUILDING CODE.
2. THE SPECIAL INSPECTOR SHALL BE APPROVED BY THE LOCAL BUILDING OFFICIAL AND SUBMIT INSPECTION REPORTS PER SECTION 1701.3.
3. THE FOLLOWING WORK SHALL BE INSPECTED BY THE SPECIAL INSPECTOR:
  - A. CONCRETE:
    1. PLACEMENT OF REINFORCEMENT - SPECIAL INSPECTION NEED NOT BE CONTINUOUS BUT MUST BE DONE ONCE PLACEMENT IS COMPLETED AND PRIOR TO CLOSING THE FORMS. ITEMS TO BE CHECKED INCLUDE BAR SIZE, GRADE, QUANTITIES, SPACING, LAP SPLICE LOCATIONS AND LENGTH. BARS SHALL BE CLEAN AND FREE OF MUD AND EXCESSIVE RUST AND MUST HAVE PROPER CONCRETE COVERAGE.
    2. CONCRETE PLACEMENT - THE SPECIAL INSPECTOR SHALL BE PRESENT DURING CONCRETE PLACEMENT OPERATIONS. ITEMS TO BE CHECKED INCLUDE MANNER OF PLACEMENT, CONSOLIDATION BY MECHANICAL VIBRATION, PREVENTION OF COLD JOINTS, PROPER LOCATION AND CONFIGURATION OF VERTICAL CONSTRUCTION JOINTS AND PROPER CURING PROCEDURES FOR COLD OR HOT WEATHER CONCRETING.
    3. EMBEDDED ANCHOR BOLTS & STUDS - SPECIAL INSPECTION SHALL BE DONE ON A CONTINUOUS BASIS. ITEMS TO BE CHECKED ARE PROPER ANCHOR BOLT DIAMETERS, EMBEDMENT LENGTHS, SPACING BETWEEN ANCHORS, EDGE DISTANCES AND CONSOLIDATION AROUND ANCHORS.
  - B. STRUCTURAL WELDING:
    1. ALL WELDING SHALL RECEIVE CONTINUOUS SPECIAL INSPECTION UNLESS COVERED BY ITEM 2.
    2. THE FOLLOWINGS ITEMS NEED NOT BE CONTINUOUSLY INSPECTED BUT SHALL BE VISUALLY INSPECTED AFTER COMPLETION OF THE WORK:
      - a. SINGLE-PASS FILLET WELDS NOT EXCEEDING 5/16 INCH IN SIZE.
      - b. WELDING OF STAIRS AND RAILING SYSTEMS.
      - c. WELDING DONE IN AN APPROVED FABRICATOR'S SHOP IN ACCORDANCE WITH SECTION 1701.7.
    3. ITEMS TO BE CHECKED ARE THE CERTIFICATION OF THE WELDERS, WELDING PROCEDURES AND MATERIALS, VISUAL INSPECTION OF WELD SIZE, LENGTH, QUALITY, QUANTITY AND SPACING.
  - C. STRUCTURAL MASONRY:
    1. PERIODIC SPECIAL INSPECTION SHALL BE MADE DURING THE PREPARATION AND TAKING OF PRISMS AND TEST SPECIMENS, AT THE START OF LAYING UNITS, AFTER THE PLACEMENT OF REINFORCING STEEL, GROUT SPACE PRIOR TO EACH GROUTING OPERATION.
    2. CONTINUOUS SPECIAL INSPECTION SHALL BE DONE DURING ALL GROUTING OPERATIONS.
    3. ITEMS TO BE CHECKED ARE MORTAR MIXES, PROPER SETTING OF BED AND HEAD JOINTS, PROPER CLEANING OF GROUT CELLS, REINFORCEMENT SIZE, GRADE, SPACING, AND SUPPORT, GROUTING PROCEDURES AND CONSOLIDATION.
  - D. POST INSTALLED ANCHORS:
    1. THE INSTALLATION OF CHEMICAL - TYPE ANCHORS IN CONCRETE SHALL BE CONTINUOUSLY INSPECTED.
    2. ITEMS TO BE CHECKED ARE PROPER BOLT DIAMETERS, EMBEDMENT LENGTHS, SPACING BETWEEN ANCHORS, EDGE DISTANCES AND INSTALLATION PROCEDURE COMPLIANCE WITH THE APPROPRIATE EVALUATION REPORT.
  - E. WOOD FRAMING:
    1. THE FOLLOWING MATERIALS SHALL BE CHECKED PRIOR TO THEIR ERECTION: LOAD BEARING STUD SPECIES AND GRADE; WALL SHEATHING PANEL GRADE, SPAN RATING AND NAILS; SHEAR WALL SHEATHING PANEL GRADE, SPAN RATING AND NAILS; ROOF SHEATHING PANEL GRADE, SPAN RATING AND NAILS; GLUED LAMINATED BEAM SPECIES COMBINATION.
    2. THE FOLLOWING ITEMS SHALL BE CHECKED ON A PERIODIC BASIS DURING CONSTRUCTION: NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN THE LATERAL LOAD RESISTING SYSTEM INCLUDING DRAG STRUTS, COLLECTORS AND HOLD-DOWNS.
  - F. GRADING, EXCAVATIONS AND COMPACTED FILL:
    1. DURING EARTHWORK EXCAVATIONS, GRADING AND FILLING OPERATIONS INSPECTION TO SATISFY REQUIREMENTS OF CHAPTER 18 AND APPENDIX CHAPTER 33.

PERMIT ISSUE:

22 SEPT. 2003

#### REVISIONS:

- |             |                |
|-------------|----------------|
| 28 OCT 2003 | ADDENDUM #1    |
| 12 NOV 2003 | CITY COMMENTS  |
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PROTO 90  
HWY 287 AT PEARL RD.  
CASA GRANDE, AZ

Johnny  
**Carino's**  
COUNTRY ITALIAN

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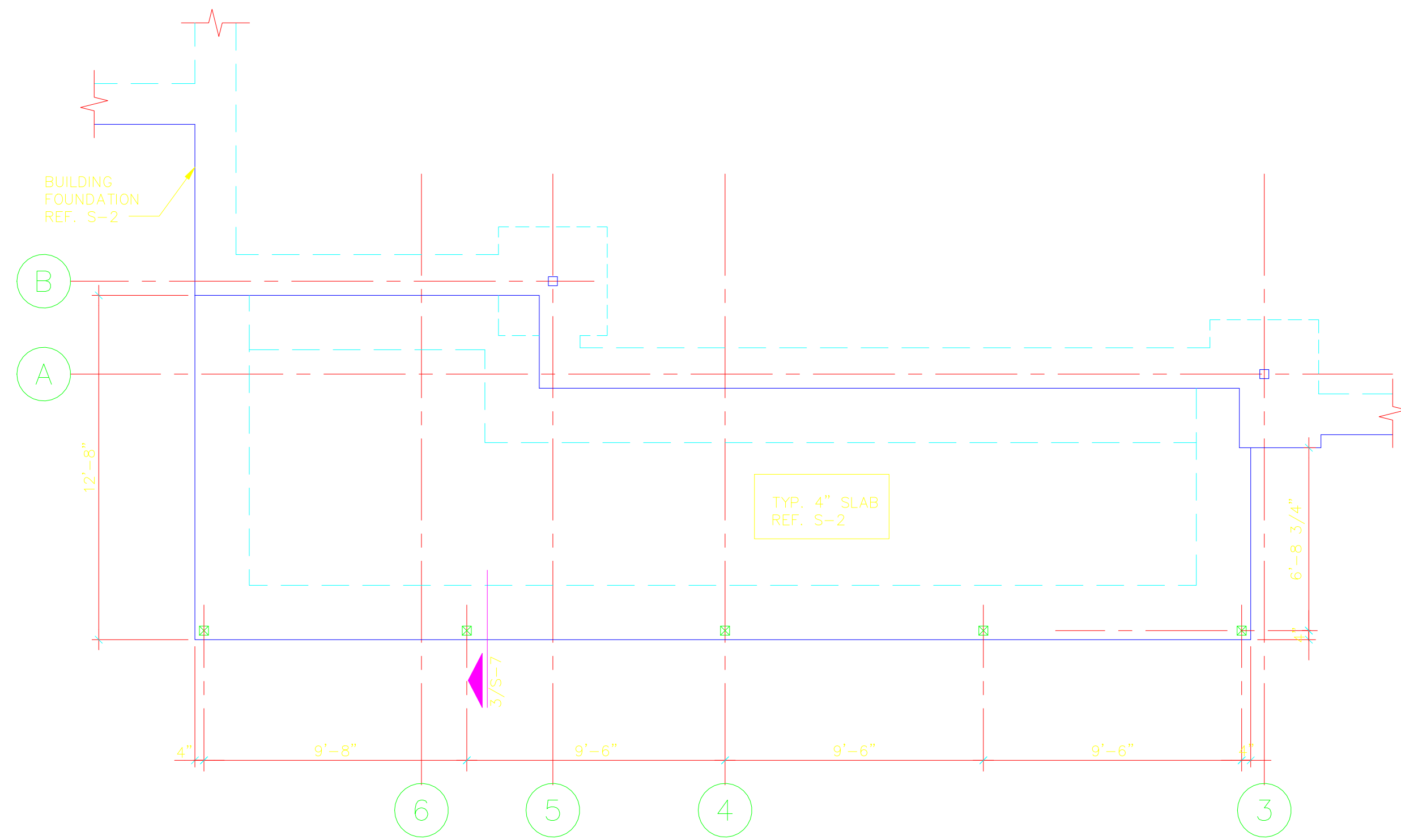
DRAWN: JLL DESIGN: JEK HGA JOB # 203180

SHEET NO:

S-7

SCALE: 3/4"=1'-0"



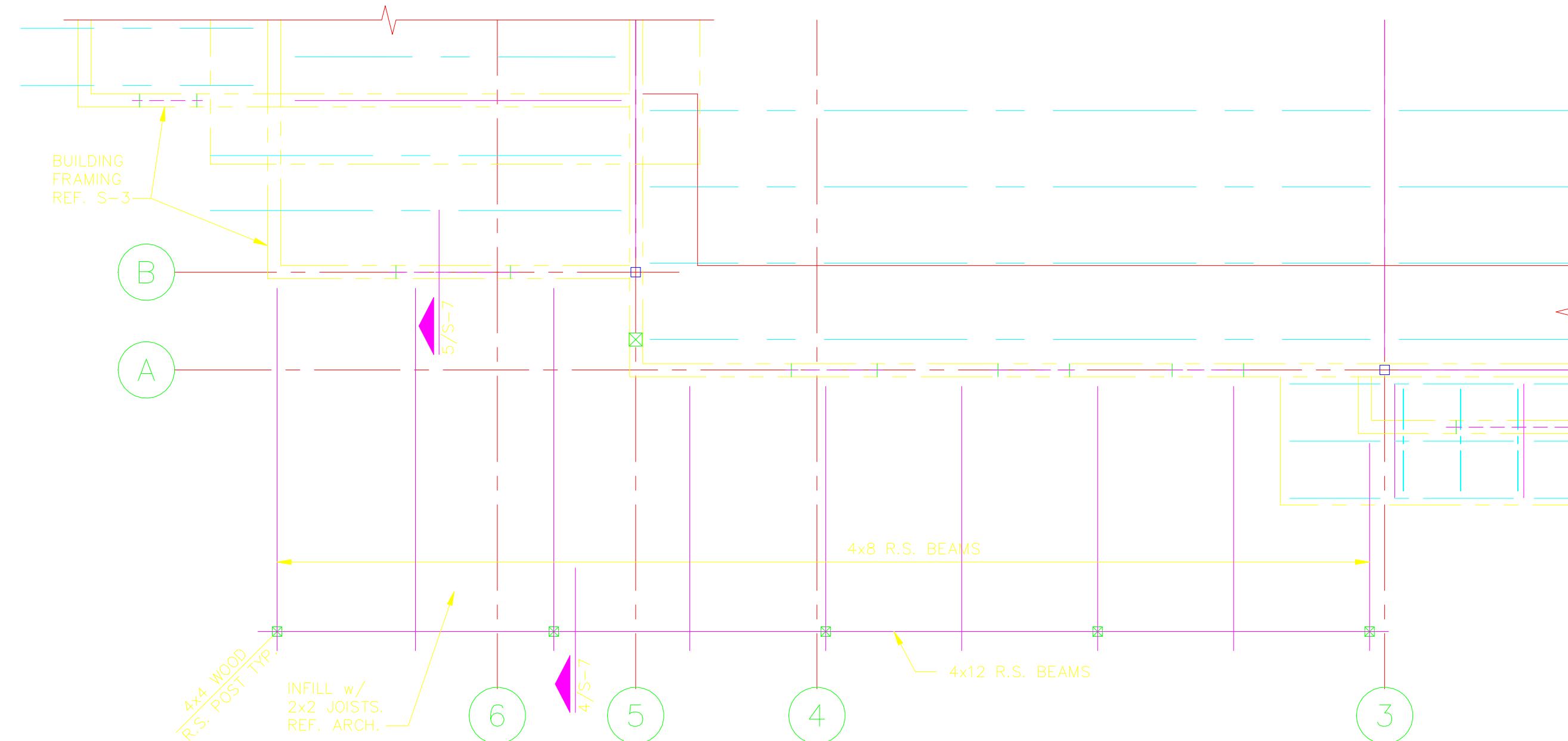


### 1 FOUNDATION PLAN @ PATIO

SCALE: 1/4"=1'-0"

#### FOUNDATION PLAN NOTES:

1. REF. CIVIL FOR ACTUAL FINISH FLOOR ELEVATION AND SLOPE AT PATIO.
2. REF. GEN. NOTES FOR SUBGRADE PREPARATIONS.

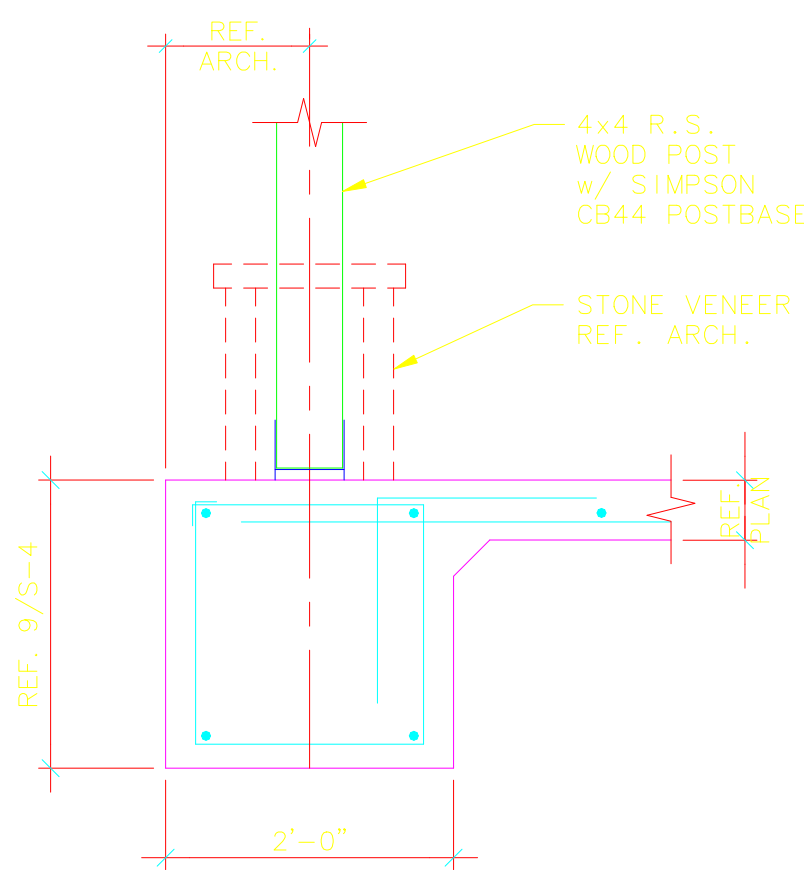


### 2 ROOF FRAMING PLAN @ PATIO

SCALE: 1/4"=1'-0"

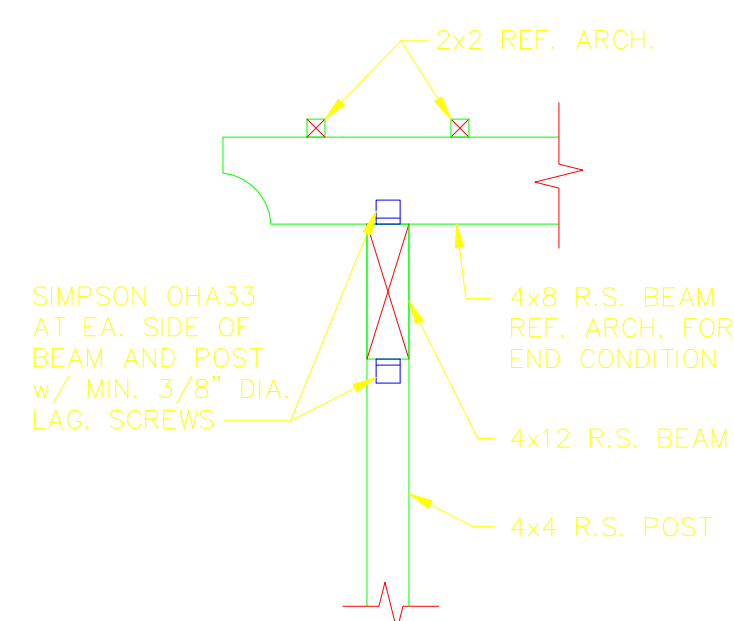
#### ROOF FRAMING PLAN NOTES:

1. DATUM EL. = 100'-0". REF. CIVIL FOR ACTUAL F.F. EL.
2. ROOF JOISTS NOTED R.S. ARE ROUGH SAWN FIR. REF. ARCH.



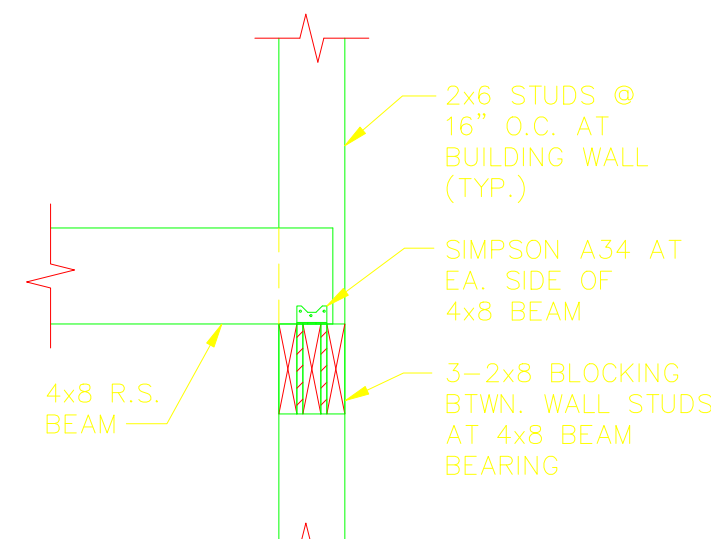
### 3 SECTION

SCALE: 3/4"=1'-0"



### 4 SECTION

SCALE: 3/4"=1'-0"



### 5 SECTION

SCALE: 3/4"=1'-0"

#### SPECIAL INSPECTIONS

1. A SPECIAL INSPECTOR SHALL BE ENGAGED TO MAKE SPECIAL INSPECTIONS OF THE CONSTRUCTION WORK AS OUTLINED BELOW IN CONFORMANCE WITH THE PROVISIONS OF CHAPTER 17 OF THE UNIFORM BUILDING CODE.
2. THE SPECIAL INSPECTOR SHALL BE APPROVED BY THE LOCAL BUILDING OFFICIAL AND SUBMIT INSPECTION REPORTS PER SECTION 1701.3.
3. THE FOLLOWING WORK SHALL BE INSPECTED BY THE SPECIAL INSPECTOR:
  - A. CONCRETE:
    1. PLACEMENT OF REINFORCEMENT - SPECIAL INSPECTION NEED NOT BE CONTINUOUS BUT MUST BE DONE ONCE PLACEMENT IS COMPLETED AND PRIOR TO CLOSING THE FORMS. ITEMS TO BE CHECKED INCLUDE BAR SIZE, GRADE, QUANTITIES, SPACING, LAP SPLICE LOCATIONS AND LENGTH. BARS SHALL BE CLEAN AND FREE OF MUD AND EXCESSIVE RUST AND MUST HAVE PROPER CONCRETE COVERAGE.
    2. CONCRETE PLACEMENT - THE SPECIAL INSPECTOR SHALL BE PRESENT DURING CONCRETE PLACEMENT OPERATIONS. ITEMS TO BE CHECKED INCLUDE MANNER OF PLACEMENT, CONSOLIDATION BY MECHANICAL VIBRATION, PREVENTION OF COLD JOINTS, PROPER LOCATION AND CONFIGURATION OF VERTICAL CONSTRUCTION JOINTS AND PROPER CURING PROCEDURES FOR COLD OR HOT WEATHER CONCRETING.
    3. EMBEDDED ANCHOR BOLTS & STUDS - SPECIAL INSPECTION SHALL BE DONE ON A CONTINUOUS BASIS. ITEMS TO BE CHECKED ARE PROPER ANCHOR BOLT DIAMETERS, EMBEDMENT LENGTHS, SPACING BETWEEN ANCHORS, EDGE DISTANCES AND CONSOLIDATION AROUND ANCHORS.
  - B. STRUCTURAL WELDING:
    1. ALL WELDING SHALL RECEIVE CONTINUOUS SPECIAL INSPECTION UNLESS COVERED BY ITEM 2.
    2. THE FOLLOWINGS ITEMS NEED NOT BE CONTINUOUSLY INSPECTED BUT SHALL BE VISUALLY INSPECTED AFTER COMPLETION OF THE WORK:
      - a. SINGLE-PASS FILLET WELDS NOT EXCEEDING 5/16 INCH IN SIZE.
      - b. WELDING OF STAIRS AND RAILING SYSTEMS.
      - c. WELDING DONE IN AN APPROVED FABRICATOR'S SHOP IN ACCORDANCE WITH SECTION 1701.7.
    3. ITEMS TO BE CHECKED ARE THE CERTIFICATION OF THE WELDERS, WELDING PROCEDURES AND MATERIALS, VISUAL INSPECTION OF WELD SIZE, LENGTH, QUALITY, QUANTITY AND SPACING.
  - C. STRUCTURAL MASONRY:
    1. PERIODIC SPECIAL INSPECTION SHALL BE MADE DURING THE PREPARATION AND TAKING OF PRISMS AND TEST SPECIMENS, AT THE START OF LAYING UNITS, AFTER THE PLACEMENT OF REINFORCING STEEL, GROUT SPACE PRIOR TO EACH GROUTING OPERATION.
    2. CONTINUOUS SPECIAL INSPECTION SHALL BE DONE DURING ALL GROUTING OPERATIONS.
    3. ITEMS TO BE CHECKED ARE MORTAR MIXES, PROPER SETTING OF BED AND HEAD JOINTS, PROPER CLEANING OF GROUT CELLS, REINFORCEMENT SIZE, GRADE, SPACING, AND SUPPORT, GROUTING PROCEDURES AND CONSOLIDATION.
  - D. POST INSTALLED ANCHORS:
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SCALE: JOB # 203180

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OWNER COMMENTS

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SEE SUBJECT NO.

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www.hga.com (214) 752-2200 (toll-free)

DESIGNER: JEL  
DRAWN: JEL  
CHECKED: JEL  
DATE: 09/22/03



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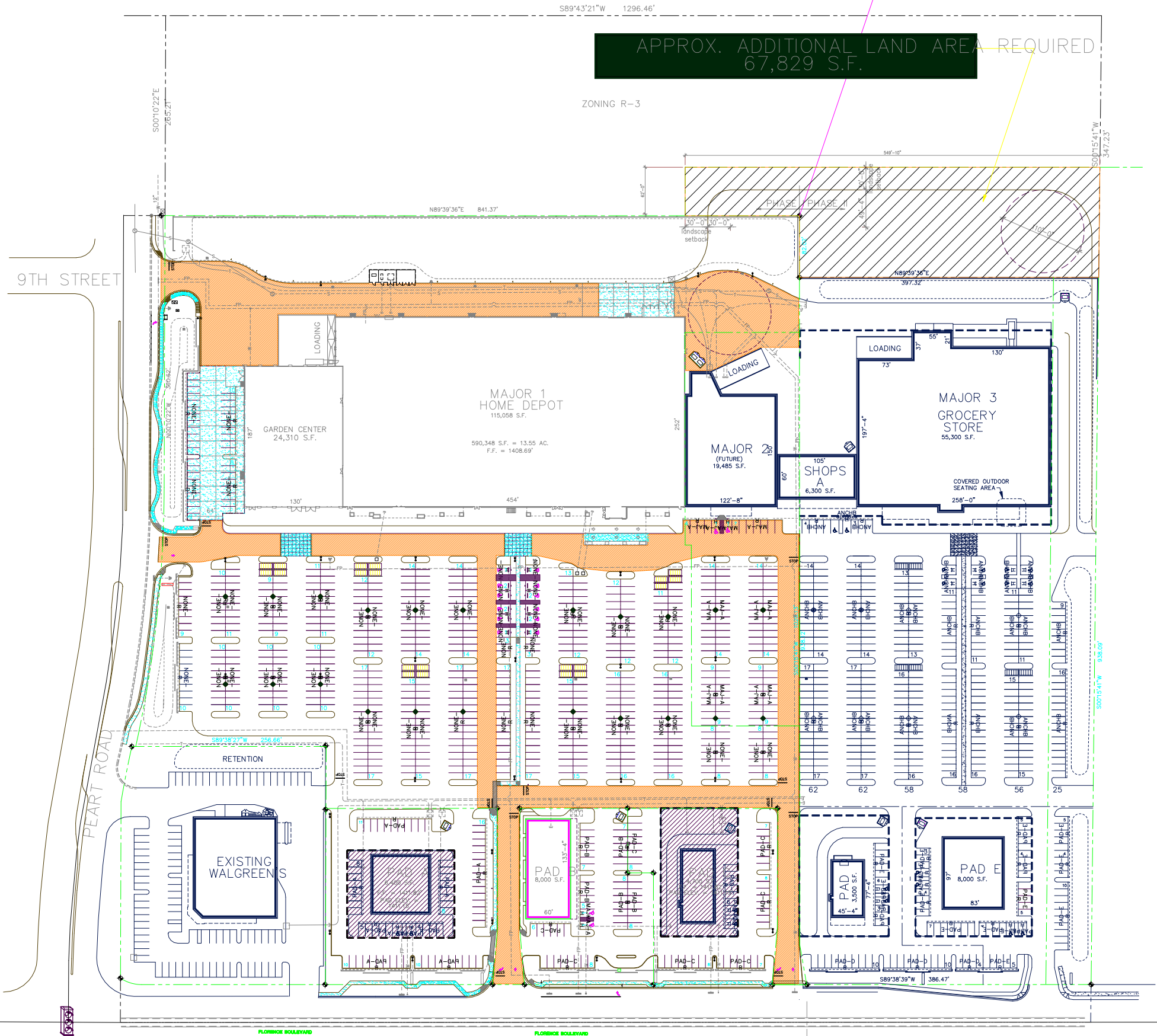
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 www.hga.com (214) 752-2000 (toll free)

DESIGNER: JEL  
 CHECKER: JEL  
 DATE: 09/22/03

# ROTATE CIVIL DRAWINGS TO MATCH PROPERTY LINES WHEN IMPORTING



APPROX. ADDITIONAL LAND AREA REQUIRED  
67,829 S.F.

ZONING R-3

9TH STREET

PEART ROAD

EXISTING WALGREENS

RETENTION

MAJOR 1 HOME DEPOT

GARDEN CENTER

MAJOR 2 (FUTURE)

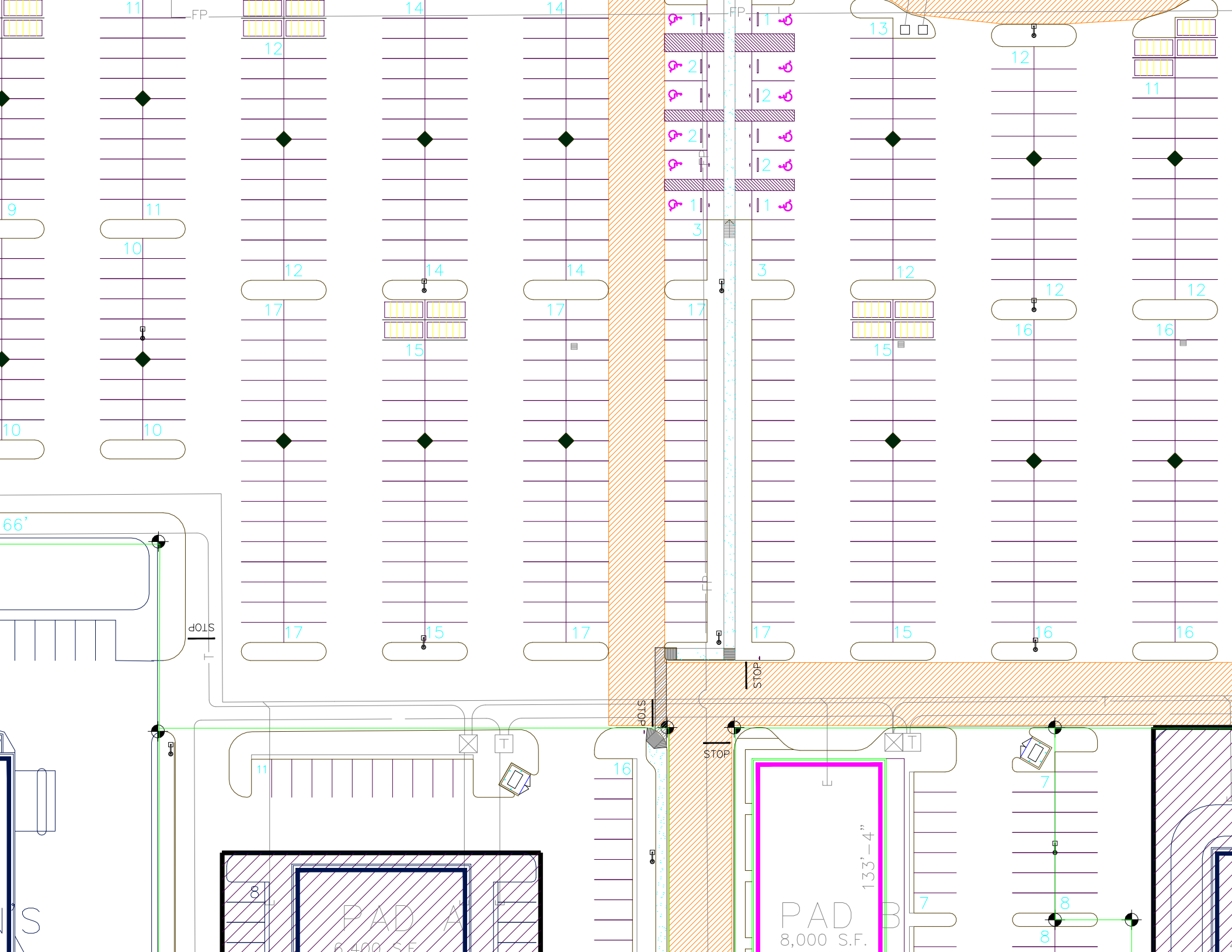
SHOPS A

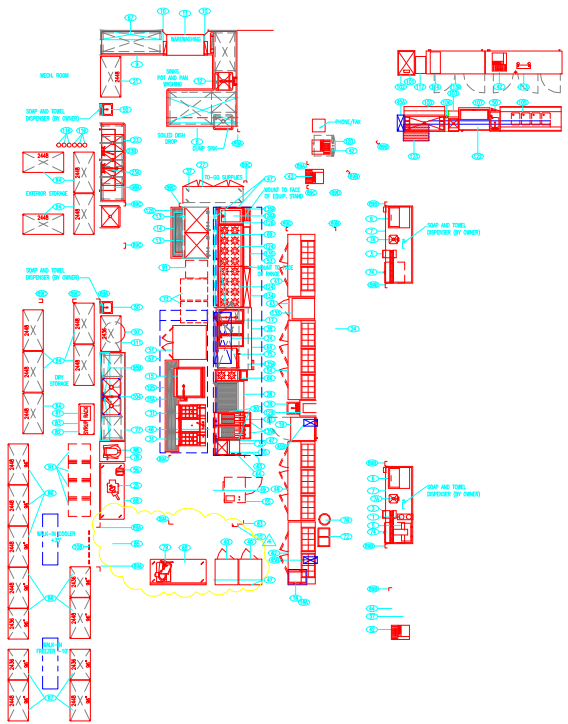
MAJOR 3 GROCERY STORE

COVERED OUTDOOR SEATING AREA

PHASE I PHASE II  
630'-0" to walgreens property line







EQUIPMENT SCHEDULE			
ITEM	QTY	DESCRIPTION	REMARKS
1	1	COFFEE MAKER	(CALC. BY OWNER)
2	1	COFFEE MAKER - NOT SHOWN	(CALC. BY OWNER)
3	1	LOAF BREAD SLICER	(CALC. BY OWNER)
4	1	LOAF BREAD SLICER - NOT SHOWN	(CALC. BY OWNER)
5	1	TVA MAKER	
6	2	TOAST BREADER	
7	2	WINDING TENSION	
7A	2	TOAST FACETS	
8	1	TOASTER OVEN	
9	1	GLASS DISPENSER	
10	1	WINE COOLER	
11	1	WINE COOLER	
12	1	SPRAY PAINT FACET	(CALC. BY OWNER)
13	2	ICE MAKERS	
14	1	ICE STORAGE BIN	
15	1	FILTRATION UNIT	
16	1	TRAY FOR WINE SHELF	
17	1	WINE REFRIGERATOR	NOT SHOWN
17A	1	FULL FACED OPEN SHOWN	
18	1	WINE RACK	
19	2	WINE RACKS	(CALC. BY OWNER)
20	1	WINE RACK	
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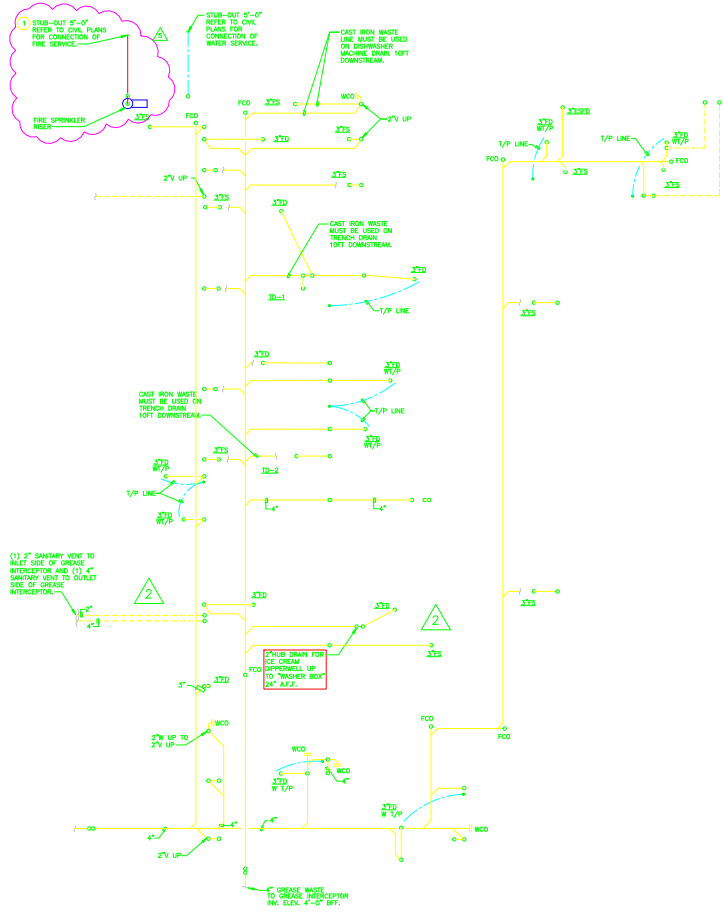
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OWNER COMMENTS

FOR  
REFERENCE  
ONLY

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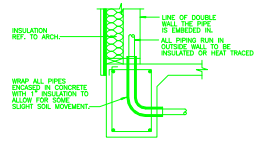






**KEYED NOTES:**

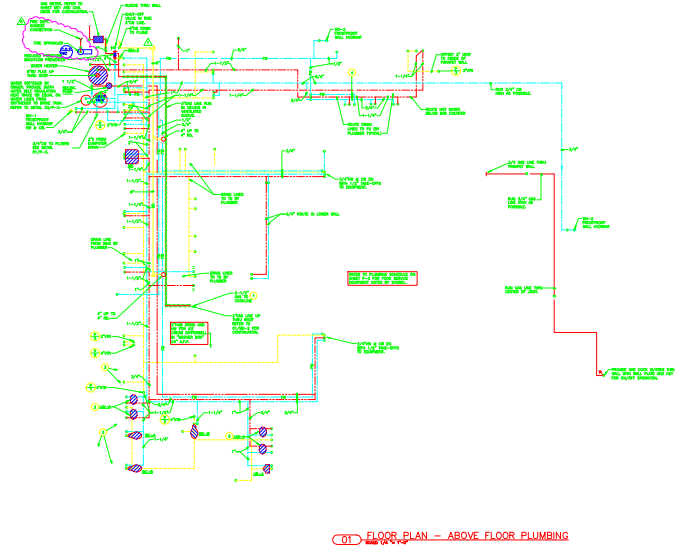
- 1. FIRE LINE TO BE SIZED BY SPRINKLER CONTRACTOR HYDRAULIC CALCULATIONS. BACKFLOW PREVENTER TO BE SIZED PER FIRE LINE DESIGN. (USE 6" SIZE FOR BIDDING PURPOSES ONLY.)



**STANDARD GRADE BEAM PENETRATION DETAIL**  
 (02) NO SCALE

- NOTE:
1. DO NOT OMIT CONTINUOUS REINFORCEMENT UNLESS A LAP SPlice OF 30 BAR DIAMETER EACH WAY IS USED. (LAP BAR SAME DIAMETER AS MAIN REINFORCEMENT)
  2. IF FOOTING MUST BE REINFORCED TO MEET THESE PARAMETERS, LENGTH IS 2'-0" MINIMUM FOR PROTECTION.
  3. PIPE DIMENSIONAL LOCATION WITHIN GRADE BEAM REFER TO STRUCTURAL PLANS.

**(01) FLOOR PLAN – UNDER FLOOR PLUMBING**  
 SCALE: 1/4" = 1'-0"



02 FLOOR PLAN - ABOVE FLOOR PLUMBING



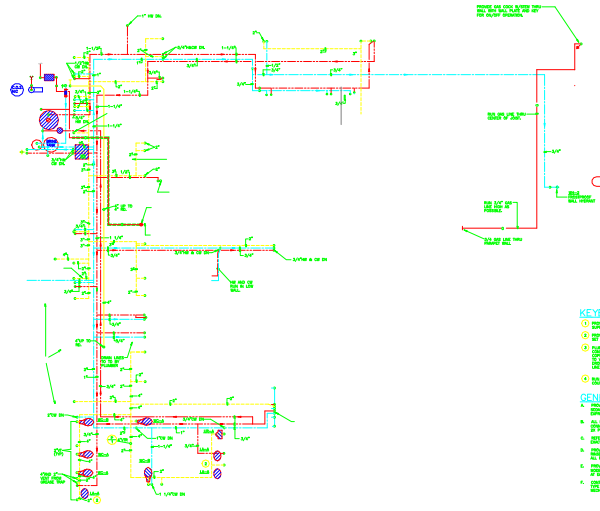
02 TRAP PRIMER DETAIL

**KEYED NOTES:**

- 1. PROVIDE SCHEDULE 40, 1/2" DIA. GALV. STEEL PIPE FOR ALL PLUMBING AND
- 2. PROVIDE SCHEDULE 40, 1/2" DIA. GALV. STEEL PIPE FOR ALL PLUMBING AND
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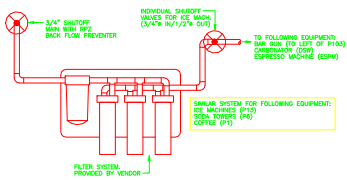
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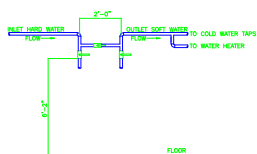
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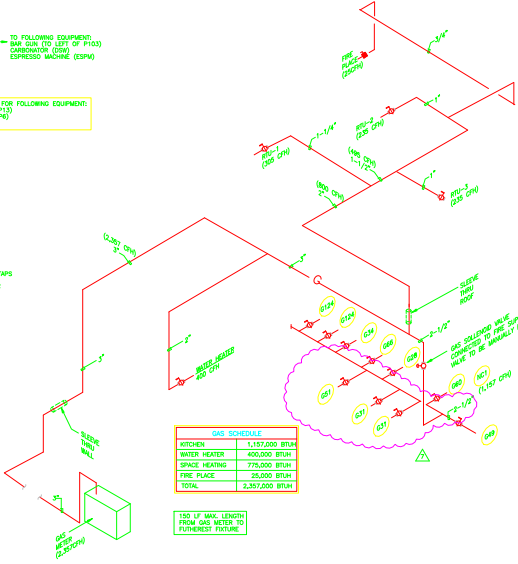
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- H. PROVIDE SCHEDULE 40, 1/2" DIA. GALV. STEEL PIPE FOR ALL PLUMBING AND
- I. PROVIDE SCHEDULE 40, 1/2" DIA. GALV. STEEL PIPE FOR ALL PLUMBING AND
- J. PROVIDE SCHEDULE 40, 1/2" DIA. GALV. STEEL PIPE FOR ALL PLUMBING AND



**01 FILTER SYSTEM**  
NO SCALE



**02 WATER SOFTENER BYPASS VALVE**  
NO SCALE

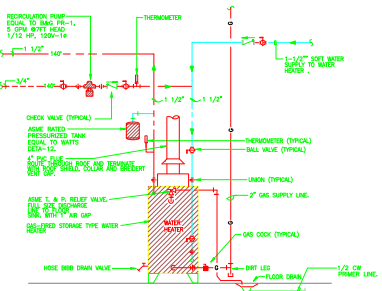
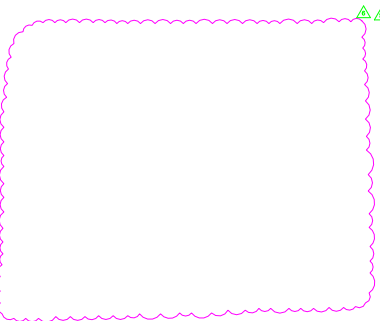


**GAS SCHEDULE**

KITCHEN	1,157,000 BTUH
WATER HEATER	400,000 BTUH
SPACE HEATING	775,000 BTUH
FIRE PLACE	35,000 BTUH
TOTAL	2,357,000 BTUH

100 LF MAX. LENGTH FROM GAS METER TO FURNITURE FIXTURE

**03 GAS RISER DIAGRAM**  
NO SCALE



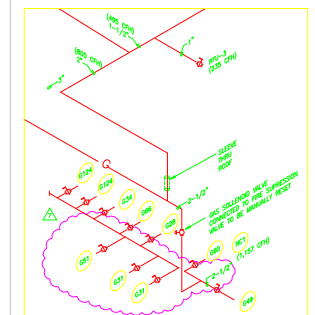
**05 GAS WATER HEATER PIPING DIAGRAM**  
NO SCALE

**PLUMBING FIXTURE SCHEDULE**

TYPE	DESCRIPTION
WC-A	AMERICAN STANDARD #2243A15, 1.50L FLUSH, ELONGATED, COMPLETE WITH #481310-100 BOLT CAPS. CURBNET. PPS OPEN FRONT. SEAT LESS COVER. SLOAN #111 FLUSH VALVE.
WC-B	AMERICAN STANDARD #2243A102, 1.5 0A/FLUSH, ELONGATED, COMPLETE WITH #481310-100 BOLT CAPS. CURBNET. PPS OPEN FRONT. SEAT LESS COVER. SLOAN #111 FLUSH VALVE. ADA COMPLIANT.
UR-A	AMERICAN STANDARD #2441132 MOUNT RM 17\"/>

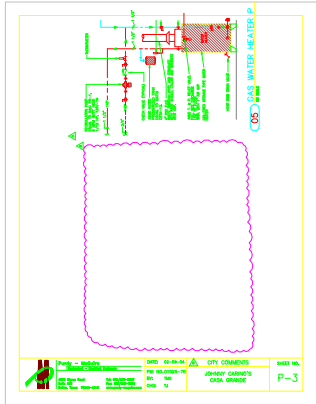
**PLUMBING SCHEDULE**

P1	1/2\"/>
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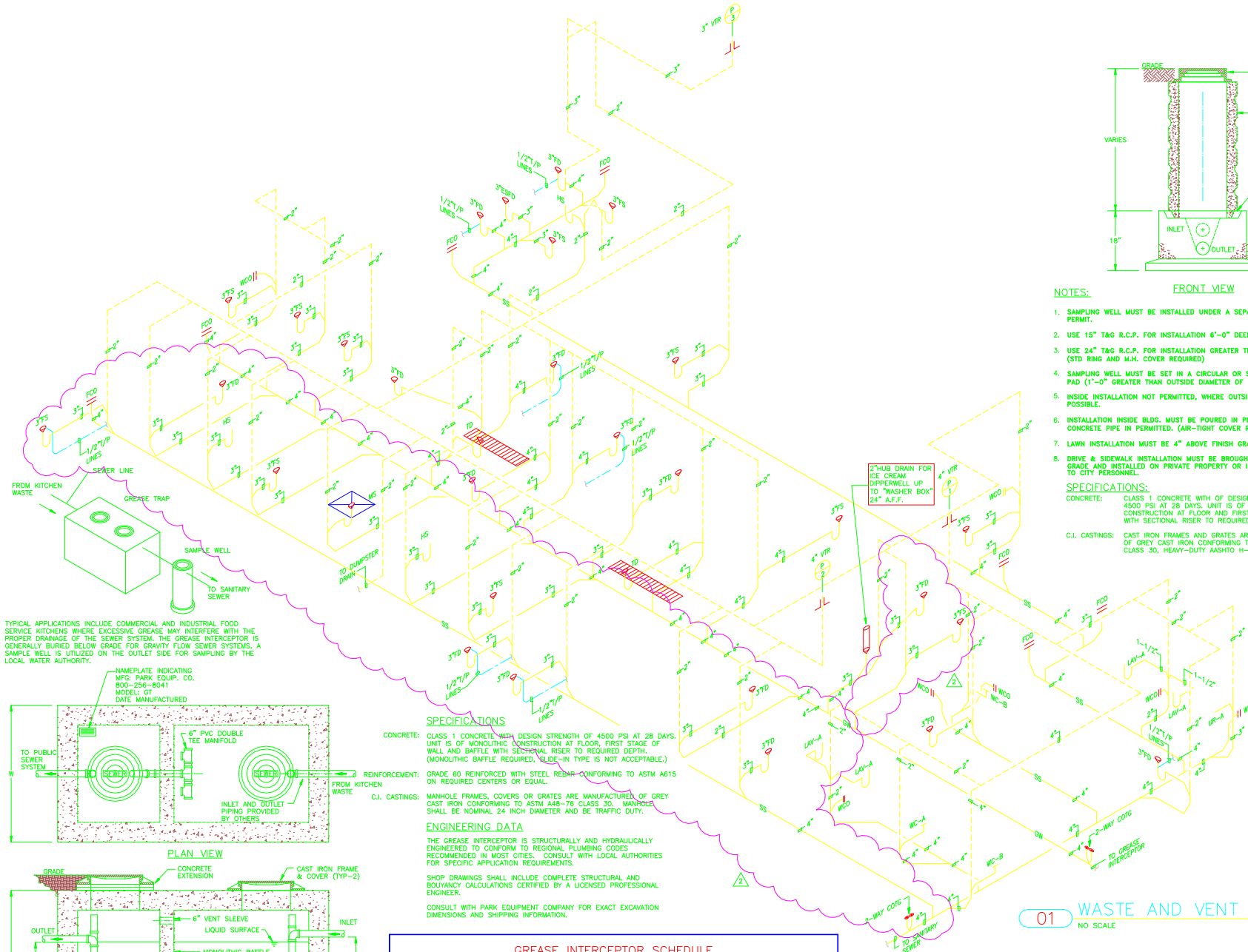
**03 GAS RISER DIAGRAM**  
NO SCALE

Project: **JOHNSON COMPANY'S GAS OFFICE** CITY COMMENTS: **JOHNSON COMPANY'S GAS OFFICE** SHEET NO. **P-3**  
 Date: **01/11/2024** DWG. NO.: **01** CITY COMMENTS: **JOHNSON COMPANY'S GAS OFFICE**  
 Scale: **AS SHOWN** DWG. DATE: **01/11/2024** CITY COMMENTS: **JOHNSON COMPANY'S GAS OFFICE**

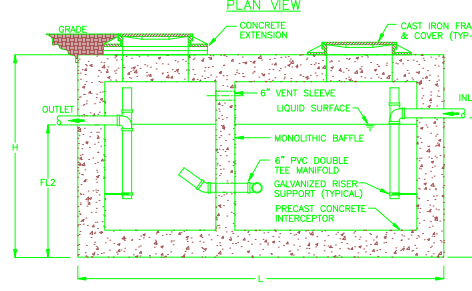
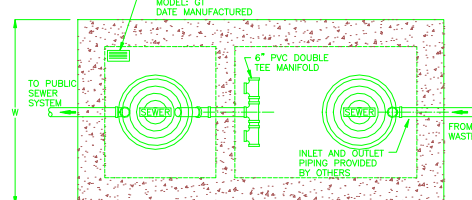


Project: **JOHNSON COMPANY'S GAS OFFICE** CITY COMMENTS: **JOHNSON COMPANY'S GAS OFFICE** SHEET NO. **P-3**  
 Date: **01/11/2024** DWG. NO.: **01** CITY COMMENTS: **JOHNSON COMPANY'S GAS OFFICE**  
 Scale: **AS SHOWN** DWG. DATE: **01/11/2024** CITY COMMENTS: **JOHNSON COMPANY'S GAS OFFICE**





TYPICAL APPLICATIONS INCLUDE COMMERCIAL AND INDUSTRIAL FOOD SERVICE KITCHENS WHERE EXCESSIVE GREASE MAY INTERFERE WITH THE PROPER DRAINAGE OF THE SEWER SYSTEM. THE GREASE INTERCEPTOR IS GENERALLY BURIED BELOW GRADE FOR GRAVITY FLOW SEWER SYSTEMS. A SAMPLE WELL IS UTILIZED ON THE OUTLET SIDE FOR SAMPLING BY THE LOCAL WATER AUTHORITY.



**02 GREASE INTERCEPTOR DETAIL**  
NO SCALE

**SPECIFICATIONS**

CONCRETE: CLASS 1 CONCRETE WITH DESIGN STRENGTH OF 4500 PSI AT 28 DAYS. UNIT IS OF MONOLITHIC CONSTRUCTION AT FLOOR, FIRST STAGE OF WALL AND BAFFLE WITH SECTIONAL RISER TO REQUIRED DEPTH. (MONOLITHIC BAFFLE REQUIRED, SLIDE-IN TYPE IS NOT ACCEPTABLE.)

REINFORCEMENT: GRADE 60 REINFORCED WITH STEEL REBAR CONFORMING TO ASTM A615 ON REQUIRED CENTERS OR EQUAL.

C.I. CASTINGS: MANHOLE FRAMES, COVERS OR GRATES ARE MANUFACTURED OF GREY CAST IRON CONFORMING TO ASTM A48-76 CLASS 30. MANHOLE SHALL BE NOMINAL 24 INCH DIAMETER AND BE TRAFFIC DUTY.

**ENGINEERING DATA**

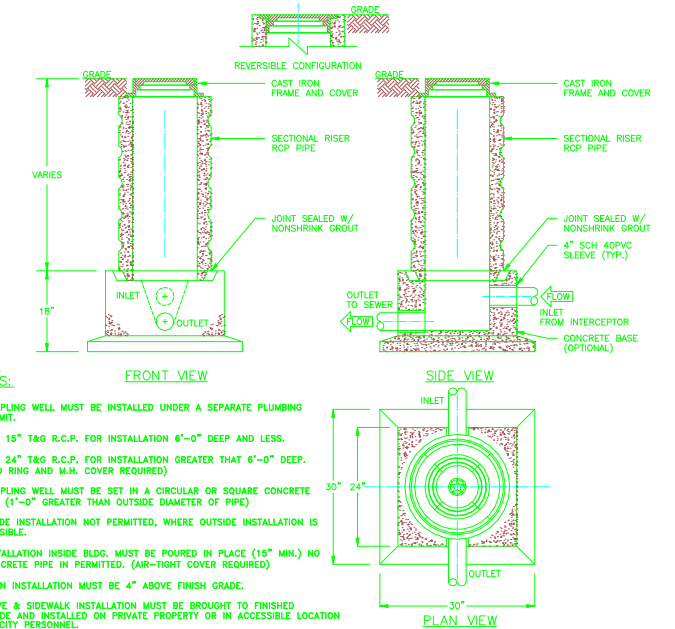
THE GREASE INTERCEPTOR IS STRUCTURALLY AND HYDRAULICALLY ENGINEERED TO CONFORM TO REGIONAL PLUMBING CODES RECOMMENDED IN MOST CITIES. CONSULT WITH LOCAL AUTHORITIES FOR SPECIFIC APPLICATION REQUIREMENTS.

SHOP DRAWINGS SHALL INCLUDE COMPLETE STRUCTURAL AND BOUYANCY CALCULATIONS CERTIFIED BY A LICENSED PROFESSIONAL ENGINEER.

CONSULT WITH PARK EQUIPMENT COMPANY FOR EXACT EXCAVATION DIMENSIONS AND SHIPPING INFORMATION.

GREASE INTERCEPTOR SCHEDULE									
MODEL NO.	CAPACITY US GAL	GREASE CAP. (LBS)	EMPTY WT. (LBS)	LENGTH	WIDTH	HEIGHT	INLET FL1	OUTLET FL2	
GT-500	500	1,200	10,000	7'-10"	4'-4"	4'-6"	4'-6"	3'-0"	
GT-750	750	1,700	10,000	7'-10"	4'-4"	6'-0"	6'-0"	4'-2"	
GT-1000	1,000	2,300	13,000	8'-8"	5'-0"	6'-0"	6'-0"	4'-6"	
GT-1500	1,500	3,500	20,000	9'-2"	5'-8"	7'-0"	7'-0"	3'-6"	
GT-2000	2,000	4,600	24,000	13'-0"	7'-0"	6'-0"	6'-0"	4'-6"	
GT-2500	2,500	5,700	27,000	13'-0"	7'-0"	7'-0"	7'-0"	5'-6"	
GT-3000	3,000	6,900	30,000	13'-0"	7'-0"	8'-0"	8'-0"	6'-6"	
GT-3500	3,500	8,000	33,000	13'-0"	7'-0"	8'-6"	8'-6"	7'-3"	
GT-4000	4,000	9,240	36,000	13'-0"	7'-0"	9'-6"	9'-6"	8'-3"	

NOTE: OTHER SIZES ARE AVAILABLE. CONTACT US FOR MORE INFORMATION.



**NOTES:**

- SAMPLING WELL MUST BE INSTALLED UNDER A SEPARATE PLUMBING PERMIT.
- USE 15" T&G R.C.P. FOR INSTALLATION 6'-0" DEEP AND LESS.
- USE 24" T&G R.C.P. FOR INSTALLATION GREATER THAN 6'-0" DEEP. (STD RING AND M.H. COVER REQUIRED)
- SAMPLING WELL MUST BE SET IN A CIRCULAR OR SQUARE CONCRETE PAD (1'-0" GREATER THAN OUTSIDE DIAMETER OF PIPE)
- INSIDE INSTALLATION NOT PERMITTED, WHERE OUTSIDE INSTALLATION IS POSSIBLE.
- INSTALLATION INSIDE BLDG. MUST BE POURED IN PLACE (15" MIN.) NO CONCRETE PIPE IS PERMITTED. (AIR-TIGHT COVER REQUIRED)
- LAWN INSTALLATION MUST BE 4" ABOVE FINISH GRADE.
- DRIVE & SIDEWALK INSTALLATION MUST BE BROUGHT TO FINISHED GRADE AND INSTALLED ON PRIVATE PROPERTY OR IN ACCESSIBLE LOCATION TO CITY PERSONNEL.

**SPECIFICATIONS:**

CONCRETE: CLASS 1 CONCRETE WITH OF DESIGN STRENGTH OF 4500 PSI AT 28 DAYS. UNIT IS OF MONOLITHIC CONSTRUCTION AT FLOOR AND FIRST STAGE OF WALL WITH SECTIONAL RISER TO REQUIRED DEPTH.

C.I. CASTINGS: CAST IRON FRAMES AND GRATES ARE MANUFACTURED OF GREY CAST IRON CONFORMING TO ASTM A48-76 CLASS 30, HEAVY-DUTY AASHTO H-20

**03 SAMPLE WELL DETAIL**  
NO SCALE

**GREASE INTERCEPTOR CALCS:**

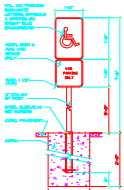
FLOOR SINKS:  
(7)FS X (4)FU = 28(FU)

FLOOR DRAINS:  
(8)FD X (2)FU = 16(FU)

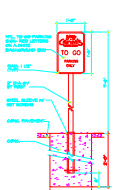
FLOOR TROUGH:  
(2)FT X (8)FU = 16(FU)

28(FU) + 16(FU) + 16(FU) = 60(FU)

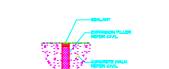
**01 WASTE AND VENT RISER DIAGRAM**  
NO SCALE



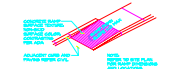
03 ARCHITECTURE & FINISHES SPACE BEAM



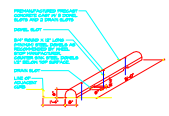
02 TO-GO FINISHES BEAM



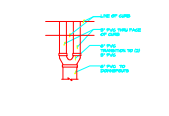
05 WALK EXPANSION JOINT



04 RAMP DETAIL



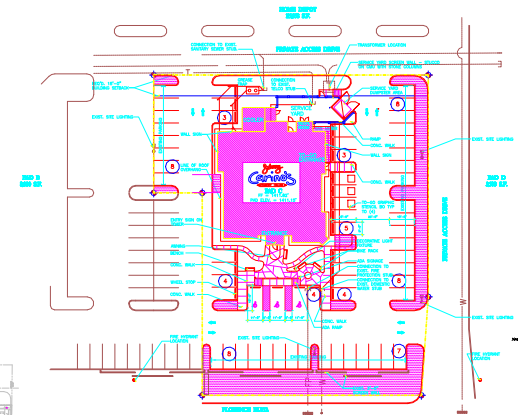
07 WHEELCHAIR STEP



06 CURB FINISHED DETAIL

**OWNER/OWNER CODE INFORMATION**  
 WM GRACE DEVELOPMENT CO.  
 7575 N. 16TH ST.  
 SUITE 1  
 PHOENIX, AZ 85020  
 602.955.8234

**RESTAURANT CONTACT**  
 SANDA FOOD GROUP  
 8801 JEFFERSON STREET  
 SUITE 500  
 ALBUQUERQUE, NM 87109  
 505.855.3600



01 FLOOR PLAN

**PROJECT DESCRIPTION**  
 JOHNNY CARINO'S COUNTRY ITALIAN A CASUAL DINING COMMERCIAL RESTAURANT ON PAD 'C' APPROXIMATELY 40,450 S.F. OF THE CASA GRANDE MARKETPLACE DEVELOPMENT.

THIS PARCEL IS DELIVERED PAD READY. ALL UTILITIES ARE DELIVERED TO THE SITE AS NOTED ON THIS PLAN. THE PAD SITE HAS BEEN ROUGH GRADDED. DRAINAGE AND RETENTION IS COVERED UNDER SEPARATE DOCUMENTS. PARKING AS NOTED ON THE PLAN IS EXISTING.

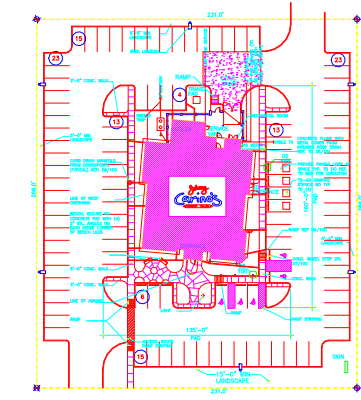
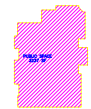
**TERRAIN DESCRIPTION**  
 PAD 'C' - CASA GRANDE MARKET PLACE

**PROJECT DATA**

ZONING	PAD	COVERAGE
LAND AREA	± 40,450 S.F.	100%
LANDSCAPE AREA	± 8,000 S.F.	24%
BUILDING/PAVING AREA	± 30,848 S.F.	76%
BUILDING AREA	6069 S.F.	
COOLING/ FREEZER	283 S.F.	
STORAGE SHED	80 S.F.	
TOTAL	6432 SQ. FT.	
DINING SEATS	234 SEATS	
BUILDING HEIGHT	26'-4" AT PEAK	

**PARKING COEFFICIENT**

PARKING BASED ON 1/50 SF OF USABLE PUBLIC AREA  
 3035/50 = 61 REQUIRED



01 SITE PLAN

**PROJECT DATA**

LAND AREA	61,446 S.F.
LAND AREA MAY INCREASE FOR LOCAL ZONING AND LANDSCAPE REQUIREMENTS	
PAD AREA	21,800 S.F.
BUILDING AREA	6069 S.F.
COOLING/ FREEZER	283 S.F.
STORAGE SHED	80 S.F.
TOTAL	6432 SQ. FT.
DINING SEATS	234 SEATS
BUILDING HEIGHT	26'-4" AT PEAK

**PARKING COEFFICIENT**

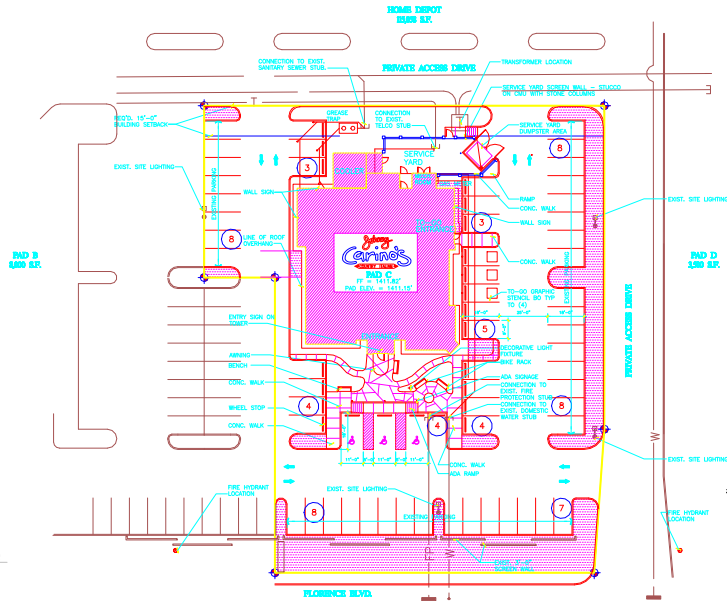
107 SPACES
5 ACCESSIBLE SPACES
112 TOTAL SPACES

**OWNER/DEVELOPER INFORMATION**

WM. GRACE DEVELOPMENT CO.  
7575 N. 16TH ST.  
SUITE 1  
PHOENIX, AZ 85020  
602.956.8254

**RESTAURANT OPERATOR**

SANDIA FOOD GROUP  
6801 JEFFERSON STREET  
SUITE 500  
ALBUQUERQUE, NM 87109  
505.855.9500



**SITE PLAN**  
11.25.03



**PROJECT DESCRIPTION**

JOHNNY CARINO'S COUNTRY ITALIAN A CASUAL DINING COMMERCIAL RESTAURANT ON PAD C, APPROXIMATELY 40,450 S.F. OF THE CASA GRANDE MARKETPLACE DEVELOPMENT.

THIS PARCEL IS DELIVERED PAD READY. ALL UTILITIES ARE DELIVERED TO THE SITE AS NOTED ON THIS PLAN. THE PAD SITE HAS BEEN ROUGH GRADED. DRAINAGE AND RETENTION IS COVERED UNDER SEPARATE DOCUMENTS. PARKING AS NOTED ON THE PLAN IS EXISTING.

**LEGAL DESCRIPTION**

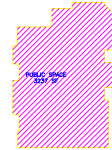
PAD 'C' - CASA GRANDE MARKET PLACE

**PROJECT DATA**

ZONING	PAD	COVERAGE
LAND AREA	± 40,450 S.F.	100%
LANDSCAPE AREA	± 9,602 S.F.	24%
BUILDING/PAVING AREA	± 30,848 S.F.	76%
BUILDING AREA	6069 S.F.	
COOLER / FREEZER	283 S.F.	
STORAGE SHED	80 S.F.	
TOTAL	6432 SQ. FT.	
DINING SEATS	234 SEATS	
BUILDING HEIGHT	26'-4" AT PEAK	

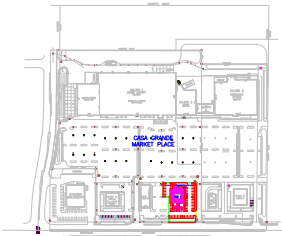
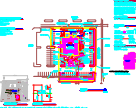
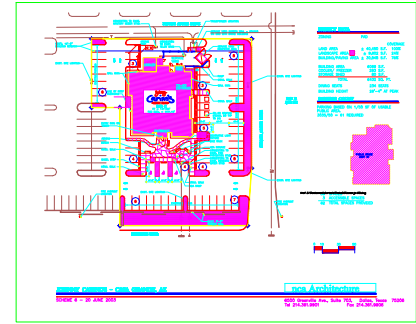
**PARKING COUNT**

PARKING BASED ON 1/50 SF OF USABLE PUBLIC AREA.  
3035/50 = 61 REQUIRED

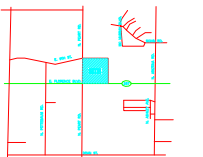


2. Restaurant/Carino's/Pad C/Space 36-80-8

3 ACCESSIBLE SPACES  
62 TOTAL SPACES PROVIDED



**MARKETPLACE MAP**

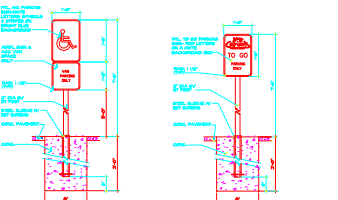


**LOCATION MAP**



SANDIA FOOD GROUP - PROTO 8.3  
CASA GRANDE MARKETPLACE - CASA GRANDE, AZ

17 APRIL 2003  
6500 Greenbriar Ave.  
Dallas, TX 75206  
Suite 207  
Ph: 214.361.9999  
Fax: 214.361.9900



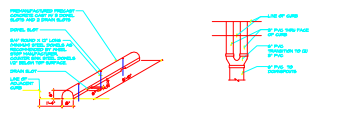
**03 ACCESSIBLE PARKING SPACE**  
SIDE VIEW

**02 TO-GO PARKING SPACE**  
SIDE VIEW



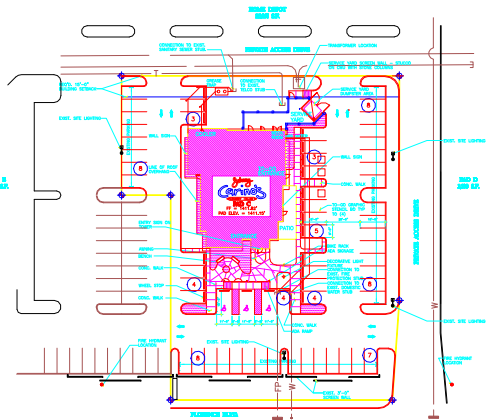
**05 WALK EXPANSION JOINT**  
SIDE VIEW

**04 RAMP DETAIL**  
SIDE VIEW



**07 CURB STEP**  
TOP VIEW

**06 CURB FINISHED DETAIL**  
TOP VIEW



**FLOOR PLAN**  
1:1/8" = 1'-0"

**OWNER/DEVELOPER INFORMATION**

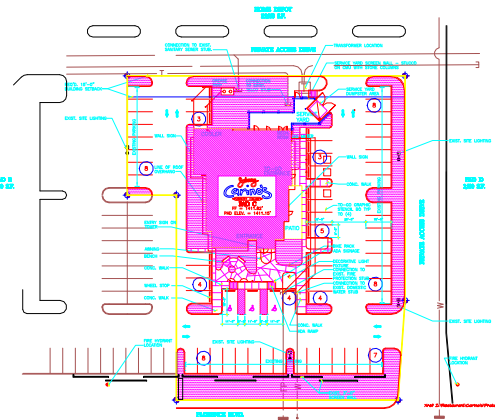
WM. GRACE DEVELOPMENT CO.  
7375 N. 16TH ST.  
SUITE 1  
PHOENIX, AZ 85020  
602.956.8254

**RESTAURANT OPERATOR**

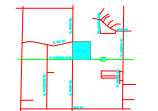
SAHARA FOOD GROUP  
6801 JEFFERSON STREET  
SUITE 500  
ALBUQUERQUE, NM 87109  
505.850.9500



**MARKETPLACE MAP**



**SETH PLAN**  
1:1/8" = 1'-0"



**LOCKERS MAP**

**PROPOSED PAVEMENT PLAN**

JOHNNY CARINO'S COUNTRY ITALIAN A CASUAL DINING COMMERCIAL RESTAURANT ON PAD C. APPROXIMATELY 40,400 S.F. OF THE CASA GRANDE MARKETPLACE DEVELOPMENT.

THIS PARCEL IS DELIVERED PAD READY. ALL UTILITIES ARE DELIVERED TO THE SITE AS NOTED ON THIS PLAN. THE PAD SITE HAS BEEN ROUGH GRADED. DRAINAGE AND RETENTION IS COVERED UNDER SEPARATE DOCUMENTS. PARKING AS NOTED ON THE PLAN IS EXISTING.

**LEGAL DESCRIPTION**

PAD 'C' - CASA GRANDE MARKET PLACE

**PROPERTY DATA**

ZONING	PAD	COVERAGE
LAND AREA	± 40,400 S.F.	100%
LANDSCAPE AREA	± 9,800 S.F.	24%
BUILDING/PAVING AREA	± 30,848 S.F.	76%
BUILDING AREA	4878 S.F.	
COOLER/ FREEZER	250 S.F.	
PATIO	400 S.F.	
TOTAL	5528 SQ. FT.	
DINING SEATS	193 SEATS	
BUILDING HEIGHT	26'-4" AT PEAK	
<b>PARKING COUNT</b>		
PARKING BASED ON 1/50 SF OF USABLE PUBLIC AREA.		
2287/50 = 44 REQUIRED		



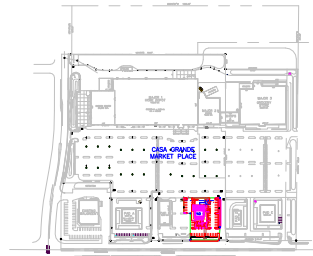
58 SPACES  
3 ACCESSIBLE SPACES  
62 TOTAL SPACES PROVIDED

**OWNER/DEVELOPER INFORMATION**

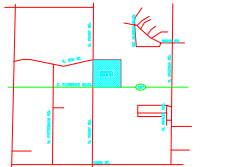
WM GRACE DEVELOPMENT CO.  
7575 N. 16TH ST.  
SUITE 1  
PHOENIX, AZ 85020  
602.856.8254

**RESTAURANT OPERATOR**

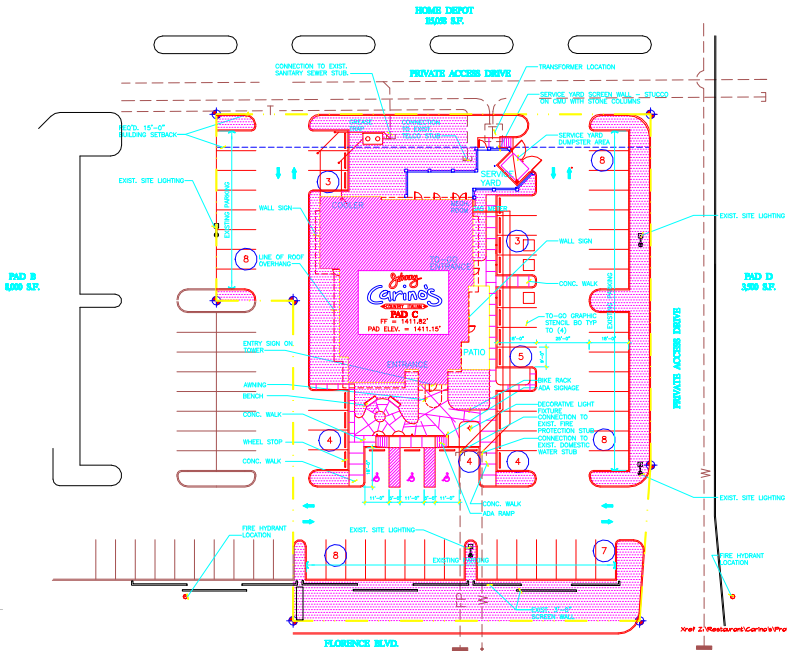
SANDIA FOOD GROUP  
6801 JEFFERSON STREET  
SUITE 500  
ALBUQUERQUE, NM 87109  
505.856.9500



**MARKETPLACE MAP**



**LOCATION MAP**



**SITE PLAN**

1" = 20'-0"



NORICH

**PROJECT DESCRIPTION**

JOHNNY CARINO'S COUNTRY ITALIAN A CASUAL DINING COMMERCIAL RESTAURANT ON PAD C APPROXIMATELY 40,450 S.F. OF THE CASA GRANDE MARKETPLACE DEVELOPMENT.

THIS PARCEL IS DELIVERED PAD READY. ALL UTILITIES ARE DELIVERED TO THE SITE AS NOTED ON THIS PLAN. THE PAD SITE HAS BEEN ROUGH GRADED. DRAINAGE AND RETENTION IS COVERED UNDER SEPARATE DOCUMENTS. PARKING AS NOTED ON THE PLAN IS EXISTING.

**LEGAL DESCRIPTION**

PAD "C" - CASA GRANDE MARKET PLACE

**PROJECT DATA**

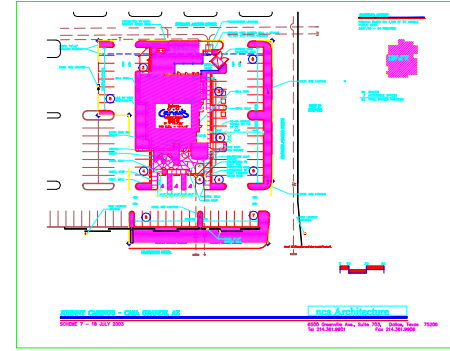
ZONING	PAD	COVERAGE
LAND AREA	± 40,450 S.F.	100%
LANDSCAPE AREA	± 9,602 S.F.	24%
BUILDING/PAVING AREA	± 30,848 S.F.	76%
BUILDING AREA	4878 S.F.	
COOLER/ FREEZER	250 S.F.	
PATIO	400 S.F.	
TOTAL	5528 SQ. FT.	
DINING SEATS	193 SEATS	
BUILDING HEIGHT	26'-4" AT PEAK	

**PARKING COUNT**

PARKING BASED ON 1/50 SF OF USABLE PUBLIC AREA.  
2267/50 = 44 REQUIRED



59 SPACES  
3 ACCESSIBLE SPACES  
62 TOTAL SPACES PROVIDED



SANDIA FOOD GROUP - PROTO 9.0  
CASA GRANDE MARKETPLACE - CASA GRANDE, AZ

**Wm & Vicki Anderson**

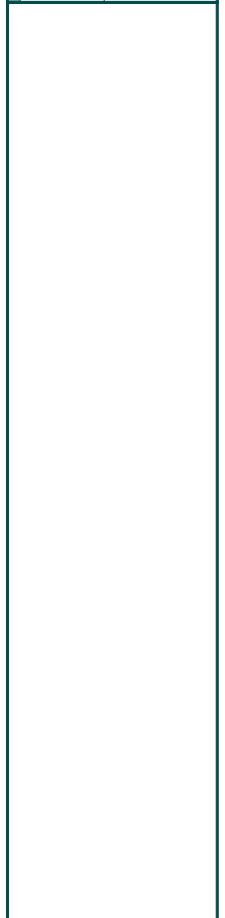
31 JULY 2003

6500 Greenbelt Ave.  
Dallas, TX 75246  
Suite 307  
Tel: 214.361.9900  
Fax: 214.361.9906



KURT D. REED ASSOCIATES, INC.  
 7400 EAST McDONALD DRIVE, SUITE 101 / SCOTTSDALE, AZ 85260  
 Phone (480) 941-1140 Fax (480) 948-1050 www.kdra.com

REV.	DATE	REVISIONS



ARIZONA REVISED STATUTES 32-1128.01.

**NOTICE OF EXTENDED CERTIFICATION AND APPROVAL PERIOD:**  
 The contractor shall be liable to certify and approve drawings and estimates within 30 days after the drawings and estimates are received from the contractor.

**NOTICE OF EXTENDED PAYMENT PROVISION:**  
 The contractor shall be liable to make payments within 30 days after certification and approval of drawings and estimates.

This drawing is an instrument of service and the property of KDR, Inc. and shall remain the confidential property of KDR, Inc. This drawing shall be restricted to the original use for which it is prepared and shall not be reproduced or otherwise used without the express written consent of KDR, Inc.



PROPOSED RETAIL CENTER  
 HWY 287 - FLORENCE BLVD.  
 AND PEART ROAD  
 CASA GRANDE, AZ  
 W.M. GRACE DEVELOPMENT CO.  
 PHOENIX, AZ

Project No: 991363 Sheet No:  
 Date: 29 JUNE 01  
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 Checked: KDR

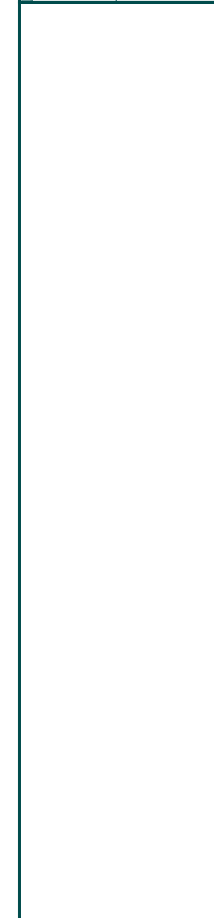
XREF

XREF



KURT D. REED ASSOCIATES, INC.  
 7400 EAST McDONALD DRIVE, SUITE 101 / SCOTTSDALE, AZ 85260  
 Phone (480) 941-1140 Fax (480) 948-1025 www.kdra.com

REV.	DATE	REVISIONS



ARIZONA REVISED STATUTES 32-1128.01.

**NOTICE OF EXTENDED CERTIFICATION AND APPROVAL PERIOD:**  
 The contractor shall be liable to certify and approve drawings and estimates within 30 days after the drawings and estimates are received from the contractor.

**NOTICE OF EXTENDED PAYMENT PROVISION:**  
 The contractor shall be liable to make payments within 30 days after certification and approval of drawings and estimates.

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 HWY 287 - FLORENCE BLVD.  
 AND PEART ROAD  
 CASA GRANDE, AZ  
 W.M. GRACE DEVELOPMENT CO.  
 PHOENIX, AZ

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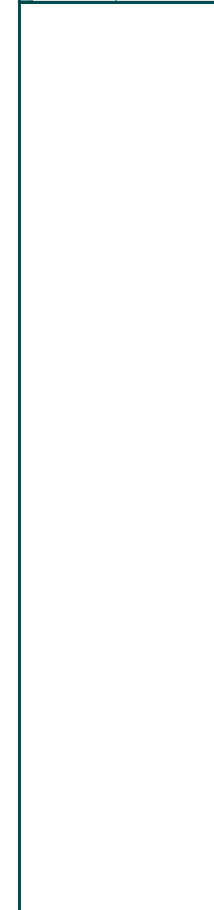
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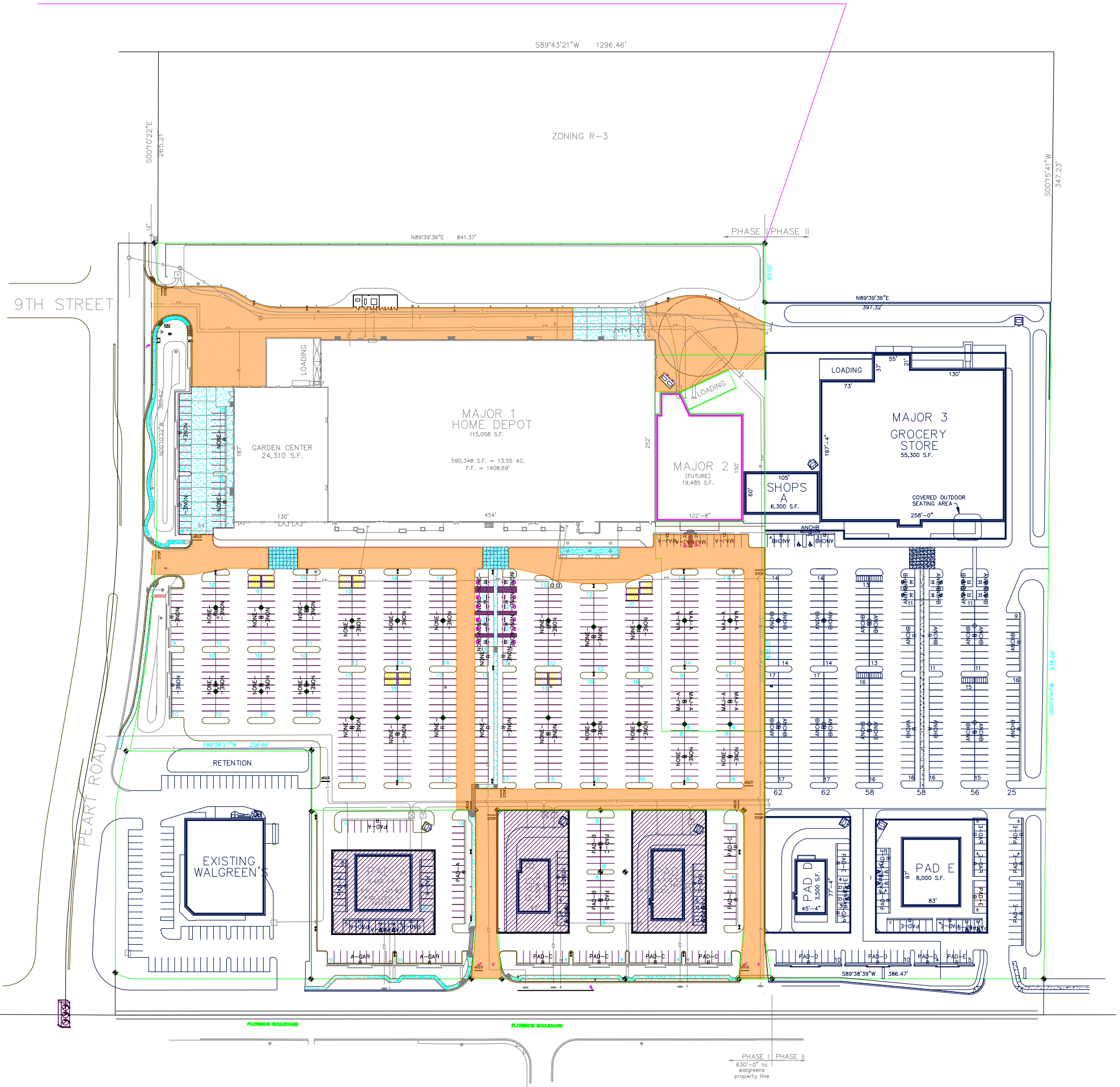
PROPOSED RETAIL CENTER  
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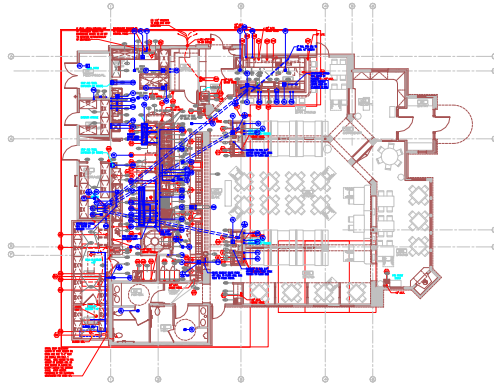
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# ROTATE CIVIL DRAWINGS TO MATCH PROPERTY LINES WHEN IMPORTING







**ELECTRICAL LEGEND**

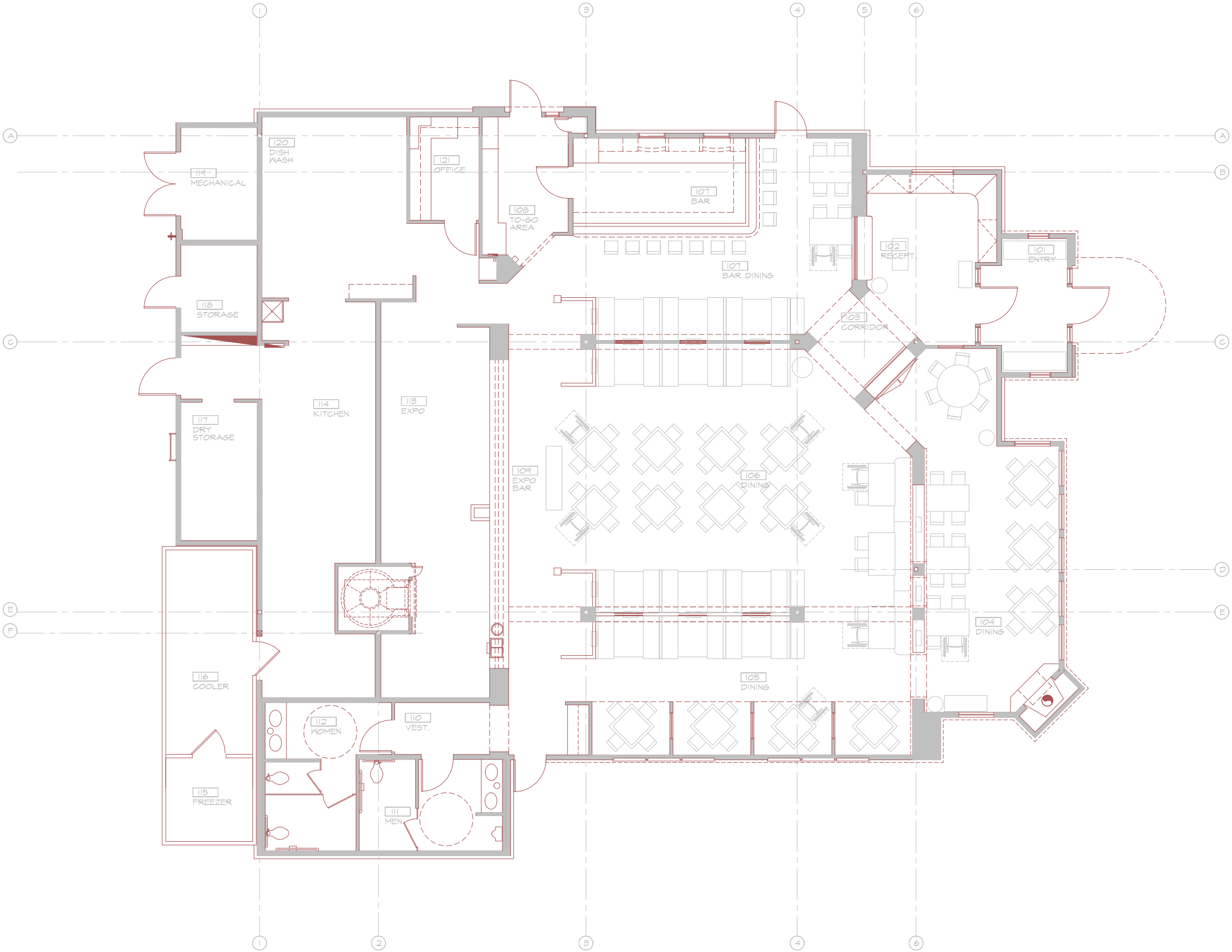
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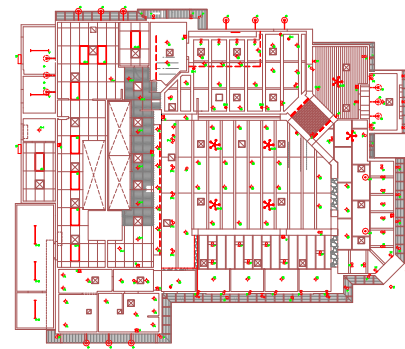
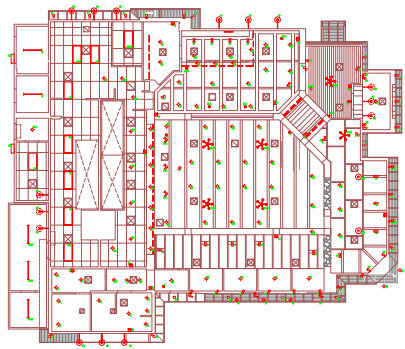
**EQUIPMENT SCHEDULE**

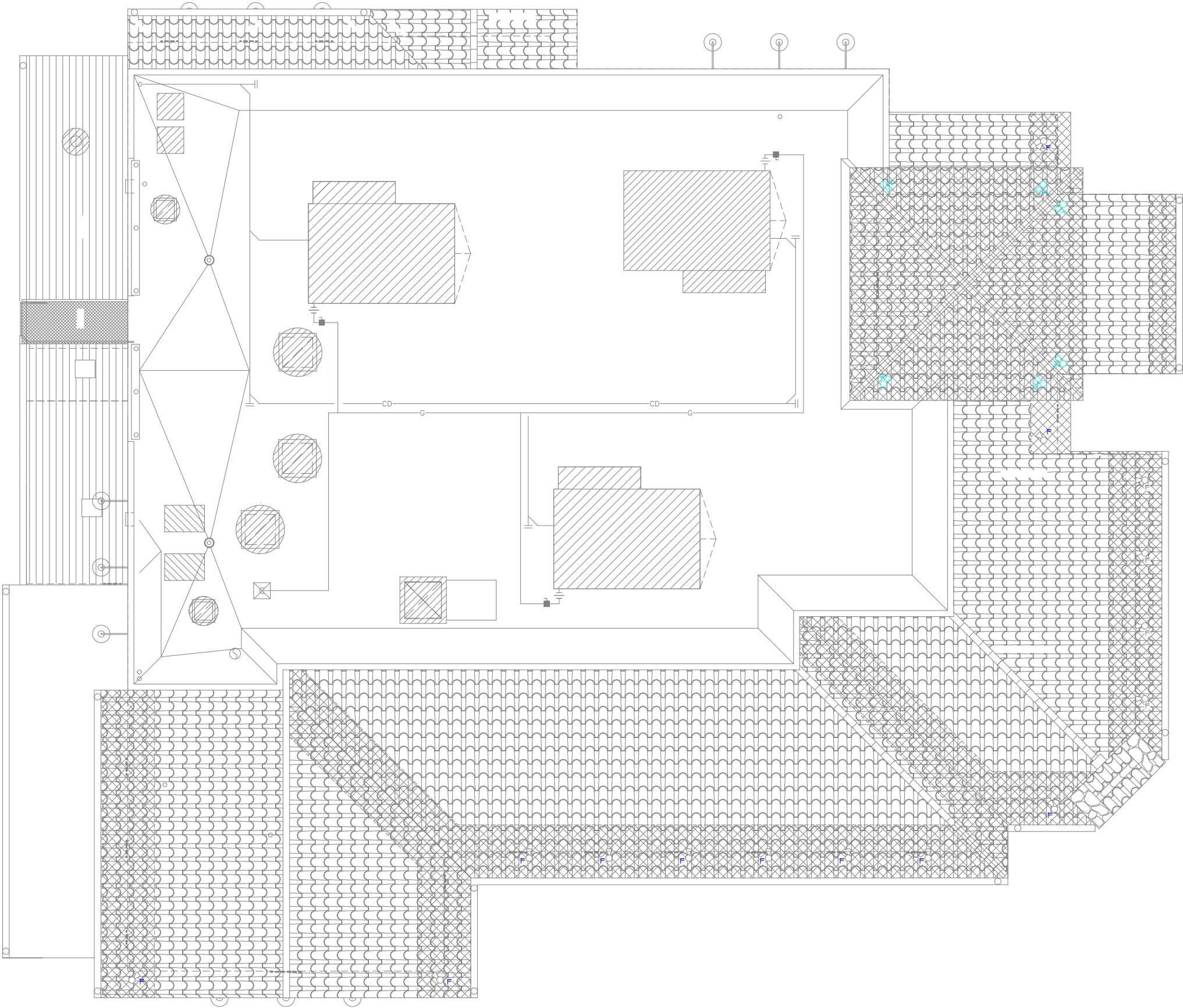
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**PLUMBING SCHEDULE**

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6500 Greenville Ave.  
Suite 307  
Dallas, TX 75206  
214.361.9901  
Fax 361.9906

AREA OF BUILDING:

JOB NO.

DATE:

C.D. REF:

C.L. REF:

PROJECT:

0103

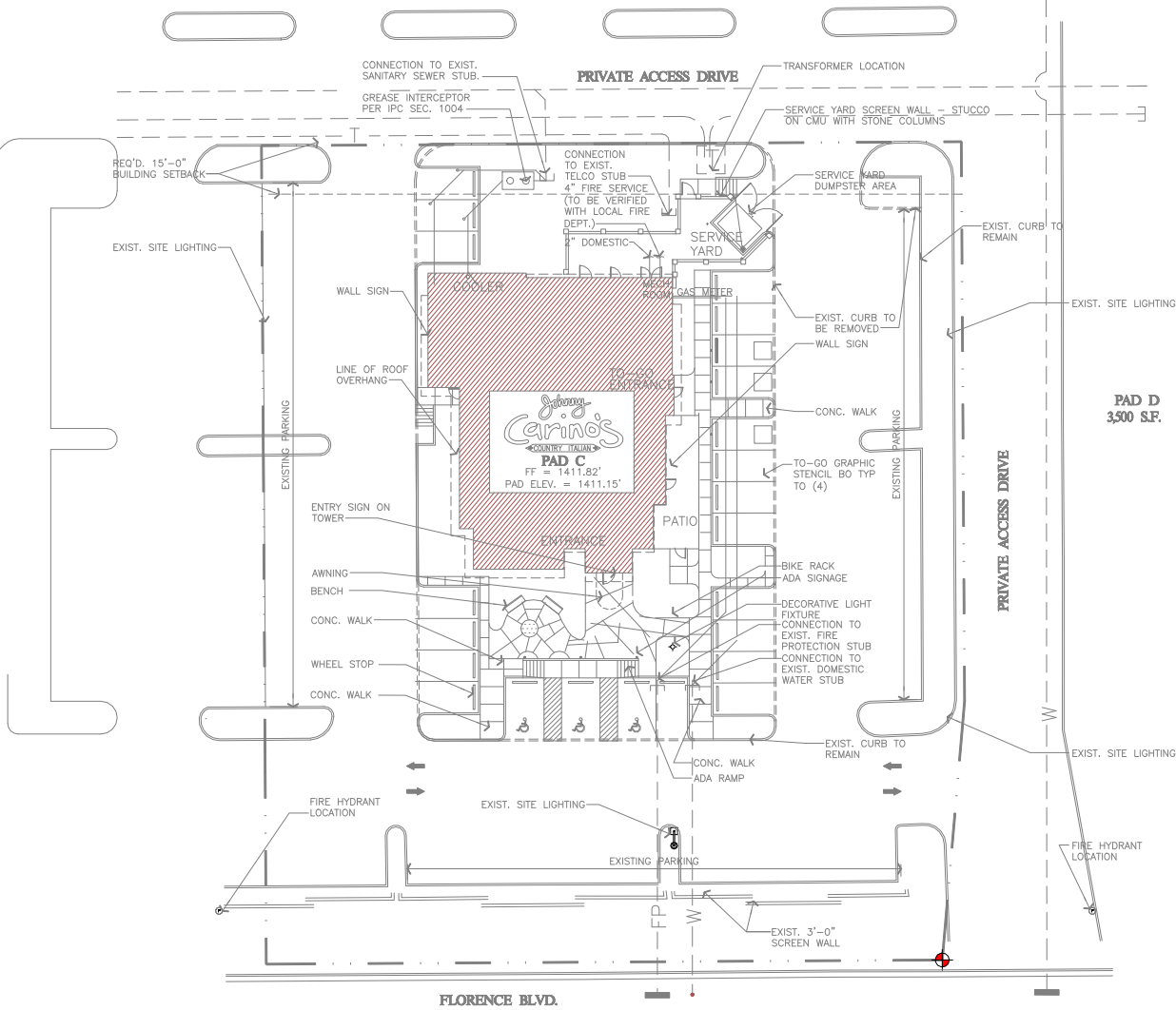
SCALE:

SHEET:

CARINO'S PROTO

8.0 - UPDATE

HOME DEPOT  
15,088 S.F.



*Carino's*  
COUNTRY ITALIAN  
**PAD C**  
FF = 1411.82'  
PAD ELEV. = 1411.15'

**PAD D**  
3,500 S.F.

FLORENCE BLVD.



PROTO ISSUE:

22 SEPT. 2003


**REVISIONS:**

▲	10/28/03	SCHEMATIC #1
▲	11/12/03	CITY COMMENTS
▲	01/28/04	CITY COMMENTS
▲	01/28/04	CITY COMMENTS
▲	01/28/04	CITY COMMENTS
▲	02/09/04	CITY COMMENTS
▲	6	CITY COMMENTS

PROTO 90  
HWY 287 AT PEARL RD.  
CASA GRANDE, AZ



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SHEET NO.

PRINT DATE - 28 JAN 2004

NCA JOB # - XXXX

