

GENERAL NOTES

GENERAL

- THE STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE UNIFORM BUILDING CODE, 1994 EDITION, AND ALL APPLICABLE REGULATIONS OF THE CITY OF CASA GRANDE, ARIZONA.
- THE LOADS THAT HAVE BEEN USED IN THE STRUCTURAL DESIGN INCLUDE THE FOLLOWING:
 DEAD LOADS AT ROOF:
 ROOF JOISTS 3 PSF
 ROOF DECK AND INSULATION 4 PSF
 ROOFING 5 PSF TYP. (10 PSF @ CLAY TILE ROOF)
 CEILING 2 PSF
 MEP (INCLUDING SPRINKLERS) 2 PSF
 TOTAL = 20 PSF TYP. (25 PSF @ CLAY TILE ROOF)
 LIVE LOAD AT ROOF 20 PSF
 WIND LOADS:
 BASIC WIND SPEED 75 MPH
 EXPOSURE C
 NET UPLIFT FIELD = 10 PSF
 PERIMETER = 10 PSF
 SEISMIC LOADS:
 ZONE 1
 SOIL TYPE SD
 ALL LIVE LOADS HAVE BEEN REDUCED BASED ON TRIBUTARY AREAS IN ACCORDANCE WITH CODE PROVISIONS AND HAVE BEEN APPLIED TO BOTH ALTERNATE AND ADJACENT SPANS WHERE APPROPRIATE TO DERIVE GOVERNING CONDITIONS.

- ALLOWABLE STRESSES HAVE BEEN INCREASED BY 1/3 FOR LOADING COMBINATIONS INCLUDING WIND AND SEISMIC WHERE ALLOWED BY CODE.
 DEAD LOADS HAVE BEEN CALCULATED TO INCLUDE THE ACTUAL WEIGHT OF ALL WORK SHOWN ON THE STRUCTURAL, MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS. NO OTHER EQUIPMENT SHALL BE PLACED ON OR HUNG FROM THE ROOF SYSTEM WITHOUT PRIOR WRITTEN APPROVAL OF THE ENGINEER. ROOF-MOUNTED HVAC UNITS SHALL BE PLACED WITHIN THE DESIGNATED AREAS SHOWN ON THE FRAMING PLANS.
- COMPLETE SHOP DRAWINGS FOR THE STRUCTURAL WORK SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO COMMENCEMENT OF CONSTRUCTION. IN ACCORDANCE WITH THE SPECIFICATIONS, SUCH REVIEW BY THE ENGINEER DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR CORRECT FABRICATION AND CONSTRUCTION OF THE WORK. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR MATERIALS PURCHASED PRIOR TO REVIEW OF SHOP DRAWINGS.

- ANY DEVIATION FROM, ADDITION TO, SUBSTITUTION FOR, OR MODIFICATION TO THE STRUCTURE OR ANY PART OF THE STRUCTURE DETAILED ON THESE DRAWINGS SHALL BE SUBMITTED IN WRITING TO THE ENGINEER FOR REVIEW. SHOP DRAWINGS THAT ARE SUBMITTED FOR REVIEW DO NOT CONSTITUTE "IN-WRITING" UNLESS IT IS CLEARLY NOTED THAT SPECIFIC CHANGES ARE BEING SUGGESTED.
- THE CONTRACTOR SHALL REFER TO ARCHITECTURAL DRAWINGS FOR ELEVATIONS NOT SHOWN AND FOR EXACT LOCATIONS OF ALL ARCHITECTURAL DETAILS. THE CONTRACTOR SHALL COMPARE THE STRUCTURAL SECTIONS WITH THE ARCHITECTURAL SECTIONS AND REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO COMPLETION OF THE SHOP DRAWINGS.
- THE CONTRACTOR SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS AT THE SITE AND SHALL NOTIFY THE ENGINEER OF ANY DISCREPANCIES BETWEEN THE ACTUAL CONDITIONS AND INFORMATION SHOWN ON THE DRAWINGS BEFORE PROCEEDING WITH THE WORK.

- PRINCIPAL OPENINGS ARE SHOWN ON THE STRUCTURAL DRAWINGS. CONTRACTOR SHALL REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, CURBS, INSERTS AND OTHER OPENINGS NOT SHOWN. THE CONTRACTOR SHALL PROVIDE FOR ALL OPENINGS, WHETHER SHOWN ON THE STRUCTURAL DRAWINGS OR NOT. SIZE AND LOCATION OF ALL OPENINGS SHALL BE VERIFIED BY THE CONTRACTOR. ANY DEVIATION FROM OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION FOR APPROVAL PRIOR TO CONSTRUCTION.
- THE STRUCTURAL DRAWINGS ARE NOT TO BE SCALED FOR DETERMINATION OF QUANTITIES, LENGTHS, OR FIT OF MATERIALS.
- THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN AND OTHER PERSONS DURING CONSTRUCTION.

- THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL WORK AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY CONDITION WHICH, IN HIS OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.
- CONSTRUCTION MATERIALS SHALL NOT BE STORED ON FLOORS OR ROOFS IN EXCESS OF THE DESIGN LIVE LOADS WHICH ARE INDICATED ON THE DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENFORCE THIS REQUIREMENT. IMPACT SHALL BE AVOIDED WHEN PLACING MATERIALS ON FLOORS OR ROOFS.
- FIELD INSPECTION REPORTS SHALL BE SUBMITTED TO THE ENGINEER AS OUTLINED IN THE SPECIFICATIONS. INSPECTIONS SHALL BE PROVIDED BY A QUALIFIED AGENCY HIRED BY THE OWNER.

EARTHWORK AND FOUNDATIONS

- THE FOUNDATION DESIGN IS BASED ON A SUBSURFACE EXPLORATION AND REPORT BY FOREE & VANN, INC. (PROJECT NO. 11637) DATED JULY 3, 2001. A GEOTECHNICAL ENGINEER SHALL VERIFY THAT SOILS OF THE DESIGN BEARING CAPACITY HAVE BEEN ENCOUNTERED AND THAT THE BUILDING PAD IS SUITABLE FOR CONSTRUCTION.
- THE FOUNDATION DESIGN IS BASED ON POTENTIAL SLAB DIFFERENTIAL MOVEMENTS OF ONE (1) INCH. IF THIS VALUE IS NOT ACCEPTABLE TO THE OWNER OR TENANTS, THE FOUNDATION DESIGN MUST BE REVISED.
- CONTINUOUS AND ISOLATED FOOTINGS ARE DESIGNED FOR AN ALLOWABLE NET BEARING PRESSURE OF 1250 PSF FOR TOTAL LOAD. THESE VALUES ARE APPLICABLE FOR FOOTINGS BEARING ON NATIVE UNDISTURBED SOIL. EXTERIOR FOOTINGS MUST BEAR A MINIMUM OF 30 INCHES BELOW FINISH GRADE.
- ALL SURFACE SOIL, VEGETATION, AND DEBRIS SHALL BE STRIPPED AND REMOVED FOR THE AREA OF THE BUILDING PAD AND AT LEAST THREE (3) FEET BEYOND THE BUILDING PERIMETER.
- AFTER STRIPPING IS COMPLETE, THE EXPOSED SUBGRADE SHALL BE SCARIFIED TO A DEPTH OF TWELVE (12) INCHES, MOISTURE CONDITIONED AND RECOMPACTED PER THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT.
- ANY UNSTABLE SOILS DETECTED SHALL BE REMOVED AND REPLACED WITH STRUCTURAL FILL.
- PLACE STRUCTURAL FILL TO PROPER ELEVATIONS. STRUCTURAL FILL SHALL CONSIST OF SOILS AS RECOMMENDED IN THE GEOTECHNICAL REPORT. PLACE A MINIMUM 12 INCHES OF NON-EXPANSIVE FILL DIRECTLY BELOW THE 4 INCHES AGGREGATE BASE.

- A QUALIFIED SOIL TECHNICIAN SHALL PERFORM SUFFICIENT IN-PLACE DENSITY TESTS DURING FILL OPERATIONS TO VERIFY THAT PROPER LEVELS OF COMPACTION ARE ATTAINED AND THAT FOOTINGS ARE BEARING ON THE PROPER MATERIAL.
- CONCRETE FOR SLABS ON FILL SHALL BE PLACED OVER A CONTINUOUS 10 MIL POLYETHYLENE MOISTURE BARRIER OVER A 4 INCH LAYER OF AGGREGATE BASE COURSE CONSISTING OF FILL AS DESCRIBED IN THE GEOTECHNICAL REPORT. PLACE A MINIMUM OF 12 INCHES OF SELECT STRUCTURAL FILL DIRECTLY BELOW THE BASE COURSE.
- THE ABOVE REQUIREMENTS ARE A SUMMARY OF THE REQUIREMENTS OF THE GEOTECHNICAL REPORT. CONTRACTOR SHALL NOT BE ABSOLVED FROM THE RESPONSIBILITY OF REVIEWING AND COMPLYING WITH THE ENTIRE GEOTECHNICAL REPORT.

CAST IN PLACE REINFORCED CONCRETE

- ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE MOST RECENT EDITION OF ACI 318 "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE".
- STEEL REINFORCING BARS SHALL CONFORM TO ASTM A-615, GRADE 60.
- WELDED WIRE FABRIC SHALL CONFORM TO ASTM A-185, GRADE 65. ALL WELDED WIRE FABRIC SHALL BE SUPPLIED IN FLAT SHEETS, NOT IN ROLLS.
- ALL CONCRETE SHALL UTILIZE NORMAL WEIGHT AGGREGATE UNLESS NOTED OTHERWISE.
- MIXES SHALL BE DESIGNED TO PROVIDE CONCRETE WITH A 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.
- ALL EXTERIOR CONCRETE SHALL BE AIR-ENTRAINED. AIR CONTENT SHALL BE 3%-6%. INTERIOR SLABS SHALL NOT HAVE AIR-ENTRAINMENT. GRADE BEAMS, RETAINING WALLS, AND FOOTINGS DO NOT REQUIRE AIR ENTRAINMENT.
- CONCRETE SLUMPS SHALL BE AS FOLLOWS:
 RAMPS & SLOPING SURFACES 3" MAX.
 CONCRETE CONTAINING SUPERPLASTICIZER 8" MAX.
 ALL OTHER CONCRETE 4"
- THE TESTING LABORATORY SHALL BE NOTIFIED AFTER THE MILD STEEL REINFORCEMENT AND EMBEDS ARE POSITIONED PRIOR TO EACH CONCRETE PLACEMENT. NO CONCRETE SHALL BE PLACED UNTIL THESE ITEMS ARE CHECKED AND APPROVED BY THE TESTING LABORATORY.

WOOD CONSTRUCTION:

- PREPARATION, FABRICATION, AND INSTALLATION OF WOOD MEMBERS AND THEIR FASTENING SHALL CONFORM TO ACCEPTED ENGINEERING PRACTICES AND TO THE REQUIREMENTS OF THE BUILDING CODE. WORK SHALL BE PERFORMED BY EXPERIENCED FABRICATORS.
- WOOD FRAMING MATERIALS:
 A. TYP. WALL SHEATHING SHALL BE 15/32" EXPOSURE 1 APA RATED SHEATHING, INDEX 32/16. TYP. ROOF DECK SHALL BE 3/4" APA RATED SHEATHING, INDEX 48/24. AT TYPICAL OVERHANGS, PROVIDE 3/8" APA RATED SHEATHING OVER 3/8" T1-11 GROOVE SIDE DOWN WITH GROOVES PERPENDICULAR TO RAFTERS. REF. 1/S-5
 B. SHEAR WALL SHEATHING SHALL BE EXPOSURE 1 APA RATED STRUCTURAL 1 SHEATHING OF THICKNESS SPECIFIED. REFER TO SHEAR WALL DETAIL 1/S-6.
 C. PSL AND LVL BEAMS SPECIFIED ON THIS PROJECT SHALL BE MANUFACTURED BY TRUSS JOIST MACMILLAN. AN EQUIVALENT PRODUCT MAY BE SUBSTITUTED UPON APPROVAL BY ENGINEER. MIN. BASIC ALLOWABLE BENDING Fb=2900 PSI AND MIN. ALLOW. SHEAR Fv=290 PSI.
 D. ALL WOOD FRAMING SHALL BE NO. 1 DOUG-FIR OR NO. 2 SOUTHERN PINE WITH A MINIMUM ALLOWABLE BENDING Fb = 975 PSI AND A MIN. ALLOWABLE SHEAR Fv=90 PSI.
 E. ALL GLU-LAM BEAMS SHALL BE NO. 2 SOUTHERN PINE WITH A MINIMUM ALLOWABLE BENDING Fb=2400 PSI, MIN. SHEAR Fv=165 PSI & MIN. E=1,800 KSI.
- ROOF DECKING:
 A. PANEL SHALL BE APPLIED WITH LONG DIMENSION PERPENDICULAR TO FRAMING. ADJACENT ROW OF PANELS SHALL BE STAGGERED.
 B. PANELS SHALL BE FASTENED TO SUPPORTS USING 8d NAILS SPACED AT 12" O.C. AT INTERMEDIATE SUPPORTS AND AT 6" O.C. AT SUPPORTED PANEL EDGES.
 C. 1/8" SPACE SHALL BE ALLOWED AT ALL PANEL ENDS AND EDGES.
 D. PANELS SHALL BE INSTALLED CONTINUOUSLY OVER TWO OR MORE SPANS.
- CONNECTIONS:
 A. THE FRAMING CONTRACTOR SHALL FURNISH ALL HARDWARE REQUIRED TO MAKE CONNECTIONS BETWEEN VARIOUS ELEMENTS. UNLESS NOTED OTHERWISE, REFER TO NAILING SCHEDULE FOR SIZE OF NAILS.
 B. SOLID BLOCKING SHALL BE PROVIDED BETWEEN ALL JOISTS AND TRUSSES AT EACH SUPPORT.
 C. ALL CONNECTIONS NOT SPECIFICALLY DETAILED SHALL UTILIZE JOIST HANGERS, BEAM HANGERS, CONNECTORS, COLUMN BASES, ETC. THIS HARDWARE SHALL BE MANUFACTURED BY SIMPSON COMPANY AND SHALL BE SELECTED AND INSTALLED ACCORDING TO MANUFACTURER'S RECOMMENDATIONS.
 D. ALL BOLTED CONNECTIONS SHALL HAVE WASHERS AT BOLT HEAD AS WELL AS AT THE NUT.
- TRUSS FABRICATION:
 A. TRUSS FABRICATOR SHALL VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
 B. LOADS FROM ALL EQUIPMENT SHOWN ON PLAN SHALL BE INCLUDED. REFER TO TRUSS PROFILE ON SHEET S-6
 C. TRUSS MANUFACTURER SHALL SUBMIT SHOP DRAWINGS AND DESIGN CALCULATIONS STAMPED BY A REGISTERED ENGINEER FOR REVIEW AND APPROVAL PRIOR TO FABRICATION.
 D. TRUSS MANUFACTURER SHALL PROVIDE ALL NECESSARY BRIDGING. TRUSSES SHALL BE DESIGNED FOR THE NET WIND UPLIFT LOAD NOTED ABOVE.
 E. ALL TRUSSES SHALL BE DESIGNED WITH THE FOLLOWING DEFLECTION CRITERIA:
 - ALLOWABLE LL DEFLECTION - L/360
 - ALLOWABLE TL DEFLECTION - L/240
 F. ALL TRUSSES SHALL BE FASTENED TO SUPPORTS WITH GALVANIZED METAL CONNECTORS AS INDICATED IN FRAMING DETAILS.
 G. ALL TRUSSES SHALL BE DESIGNED AND FABRICATED USING #3 SOUTHERN PINE OR #1 DOUGLAS-FIR PER THE RECOMMENDATIONS OF THE TP1 DESIGN SPECIFICATIONS.

STEEL

- STRUCTURAL STEEL SHALL BE NEW STEEL AND SHALL CONFORM TO THE A.I.S.C. "SPECIFICATION FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS."
- STRUCTURAL STEEL ROLLED SHAPES AND PLATES NOTED "ROLLED 50" SHALL CONFORM TO ASTM A-572, GRADE 50. ALL OTHER ROLLED SHAPES AND PLATES SHALL CONFORM TO ASTM A-36.
- STRUCTURAL STEEL PIPE SHALL CONFORM TO ASTM A-53, TYPES "E" OR "S", GRADE B OR ASTM 501.
- STRUCTURAL STEEL TUBING SHALL CONFORM TO ASTM A-500, GRADE B.
- ALL STRUCTURAL STEEL CONNECTIONS AND DETAILS SHALL CONFORM TO THE A.I.S.C. "CODE OF STANDARD PRACTICE FOR STEEL, BUILDINGS AND BRIDGES."
- UNFINISHED THREADED FASTENERS SHALL CONFORM TO ASTM A-307, GRADE A BOLTS AND NUTS WITH HEXAGONAL HEADS. UNFINISHED THREADED FASTENERS SHALL BE USED ONLY FOR BOLTED CONNECTIONS OF SECONDARY FRAMING MEMBERS TO PRIMARY MEMBERS, FOR TEMPORARY BRACING TO FACILITATE ERECTION, AND FOR ANCHORAGE TO CONCRETE OR MASONRY CONSTRUCTION.
- BOLTED CONNECTIONS OF PRIMARY MEMBERS SHALL CONFORM TO THE REQUIREMENTS OF THE A.I.S.C. "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A-325 OR A-490 BOLTS." UNLESS NOTED OTHERWISE, ALL PRIMARY CONNECTIONS WITH HIGH STRENGTH BOLTS SHALL USE ASTM A-325 BOLTS AND HEAVY HEX NUTS.
- WELDED CONSTRUCTION SHALL CONFORM TO AWS D-1.1. "STRUCTURAL WELDING CODE." WELDING PROCESSES AND OPERATORS SHALL BE QUALIFIED IN ACCORDANCE WITH AWS "STANDARD QUALIFICATION PROCEDURE." ELECTRODES FOR FIELD AND SHOP WELDS SHALL BE E70XX, UNLESS NOTED OTHERWISE.
- SHEAR STUDS SHALL CONFORM TO ASTM A-108, HEADED ANCHORS.
- STEEL MEMBERS SHALL NOT BE SPLICED EXCEPT AS SHOWN ON THE DRAWINGS.
- ALL EXTERIOR STEEL EXPOSED TO WEATHER, INCLUDING LINTELS, SHALL BE GALVANIZED.

REINFORCED MASONRY

- ALL REINFORCED MASONRY SHALL CONFORM TO THE PROVISIONS OF ACI 530.1/ASCE 5 (WITH EXCEPTIONS NOTED IN THE SPECIFICATIONS).
- CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90, GRADE N, TYPE 1, LIGHTWEIGHT UNITS WITH A MINIMUM NET AREA COMPRESSIVE STRENGTH OF 1900 PSI.
- MORTAR SHALL CONFORM TO ASTM C-270, TYPE M OR S. AGGREGATES FOR MORTAR SHALL CONFORM TO ASTM C-144. MORTAR SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 1800 PSI AT 28 DAYS.
- GROUT SHALL CONFORM TO ASTM C-476. AGGREGATES FOR GROUT SHALL CONFORM TO ASTM C-404. GROUT SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. CONSOLIDATE ALL GROUTED MASONRY WITH MECHANICAL VIBRATION.
- MASONRY CONSTRUCTION SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH (f'm) OF 1500 PSI AT 28 DAYS.
- SPECIAL INSPECTION SHALL BE PROVIDED BY THE TESTING LAB AND REPORTED TO THE ENGINEER. THE TESTING LAB SHALL PERFORM PERIODIC INSPECTIONS DURING PREPARATION AND TAKING OF PRISMS OR TEST SPECIMENS, PLACING OF REINFORCING AND GROUT, AND LAYING UP OF MASONRY.
- BOND BEAMS SHALL BE CONSTRUCTED WITH PORTLAND CEMENT GROUT HAVING A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI AT 28 DAYS, AND A MAXIMUM AGGREGATE SIZE OF 3/4".
- REINFORCING STEEL SHALL CONFORM TO ASTM A-615, GRADE 60.
- HORIZONTAL JOINT REINFORCEMENT SHALL BE FACTORY FABRICATED, TRUSS TYPE, 9 GAGE WIRE CONFORMING TO ASTM A-82. PROVIDE 1-#4 MINIMUM ABOVE AND BELOW ALL WALL OPENINGS AND EXTENDING 24" MINIMUM INTO JAMBS.
- ALL CELLS CONTAINING REINFORCEMENT, BOLTS, OR OTHER METAL ANCHORS SHALL BE GROUTED SOLID. ANY CELLS AT OR BELOW GRADE SHALL BE GROUTED SOLID, WHETHER REINFORCED OR NOT.

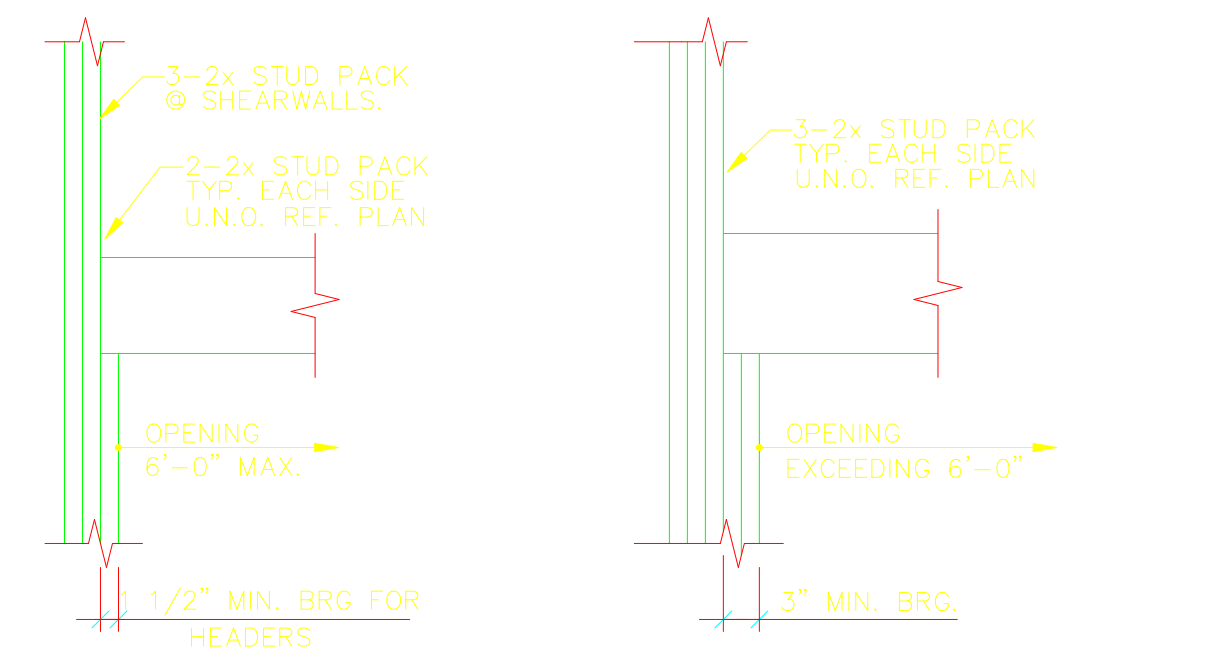
NAILING SCHEDULE

CONNECTION	NAILING
(1) JOIST TO SILL OR GIRDER, TOENAIL	3-16d
(2) BRIDGING TO JOIST, TOENAIL EACH END	2-8d
(3) 1" x 6" SUBFLOOR OR LESS TO EACH JOIST, FACE NAIL	2-8d
(4) WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST, FACE NAIL	3-8d
(5) 2" SUBFLOOR TO JOIST OR GIRDER, BLIND AND FACE NAIL	2-16d
(6) SOLE PLATE TO JOIST OR BLOCKING, FACE NAIL	16d @ 16" O.C.
(7) TOP PLATE TO STUD, END NAIL	2-16d
(8) STUD TO SOLE PLATE	4-8d
(9) DOUBLE STUDS, FACE NAIL	TOENAIL OR 2-16d O.C. ALONG EACH EDGE
(10) DOUBLED TOP PLATES, FACE NAIL	16d @ 24" O.C.
(11) TOP PLATES, LAPS AND INTERSECTIONS, FACE NAIL	2-16d
(12) CONTINUOUS HEADER, TWO PIECES	16d @ 16" O.C.
(13) CEILING JOISTS TO PLATE, TOENAIL	3-8d
(14) CONTINUOUS HEADER TO STUD, TOENAIL	4-8d
(15) CEILING JOISTS, LAPS OVER PARTITIONS, FACE NAIL	3-16d
(16) CEILING JOISTS TO PARALLEL RAFTERS, FACE NAIL	3-16d
(17) RAFTER TO HIP	3-10d TOENAIL OR 2-16d FACE NAIL
(18) 1" BRACE TO EACH STUD AND PLATE, FACE NAIL	2-8d
(19) 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL	2-8d
(20) WIDER THAN 1" x 8" SHEATHING TO EACH BEARING, FACE NAIL	3-8d
(21) BUILT-UP CORNER STUDS	16d @ 24" O.C.
(22) BUILT-UP GIRDER AND BEAMS	O.C. 20d @ 32" O.C. @ TOP & BOT. & STAGGERED 2-20d @ ENDS & @ EACH SPLICE

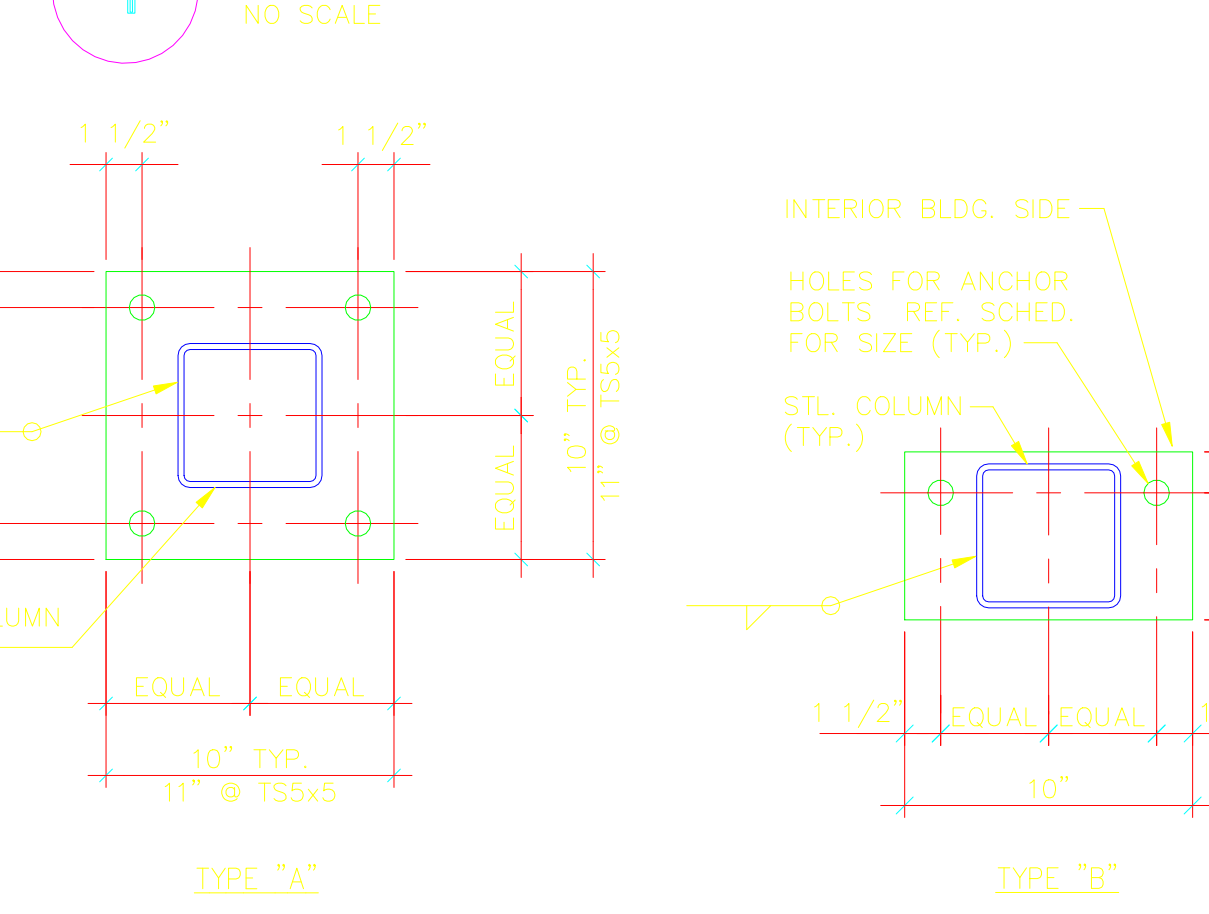
MAXIMUM CLEAR SPAN FOR HEADERS

HEADER SIZE MATERIAL AT EDGE	SUPPORTING ONLY CEILING AND ROOF
3-2x6	4'-0"
3-2x8	6'-0"
3-2x10	8'-0"
3-2x12	10'-0"

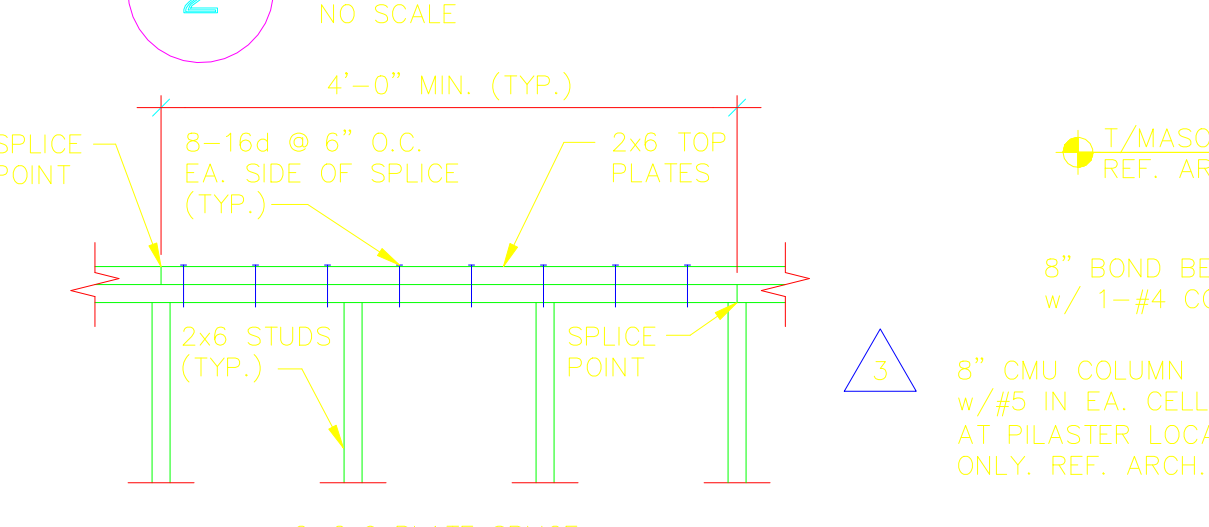
- HEADER SIZES MAY BE USED IF NO SIZES ARE CALLED OUT ON THE PLANS OR DETAILS.
- FOR WALL OPENING SIZES REFERENCE ARCHITECTURAL DRAWINGS.
- USE 1/2" FLITCH BETWEEN 3-2x HEADERS.



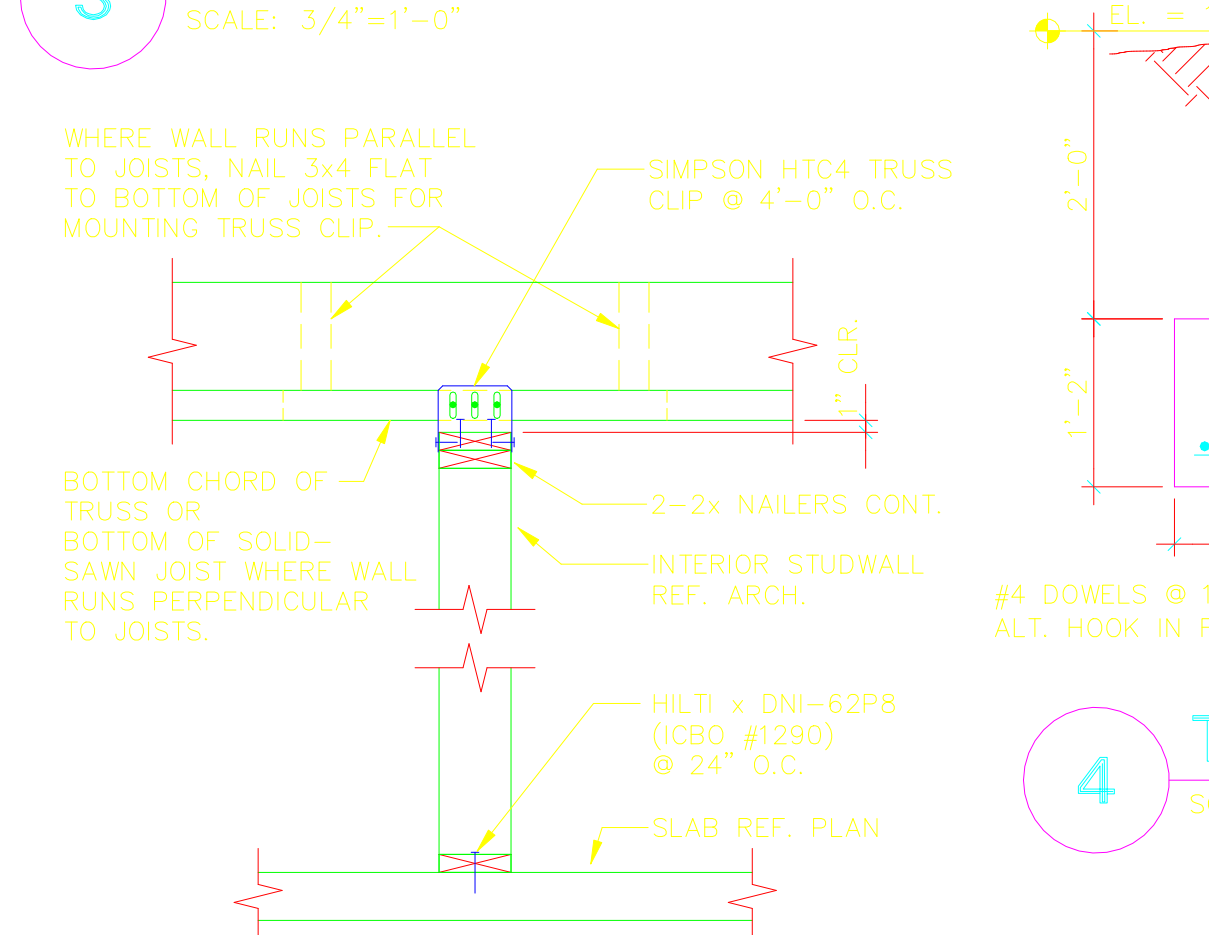
1 HEADER BEARINGS



2 TYPICAL BASE PLATE



3 TYPICAL SECTION



5 TYP. DETAIL

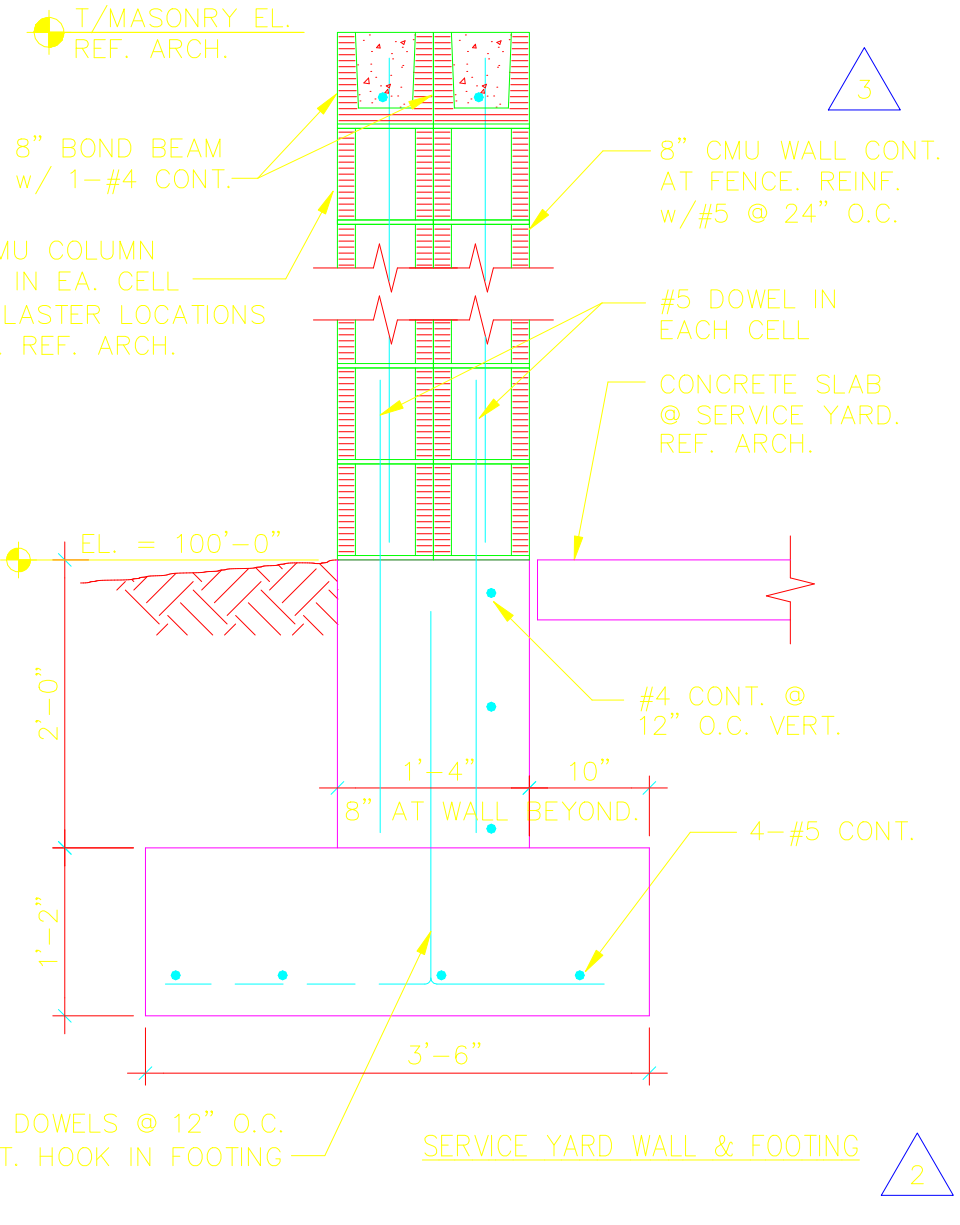
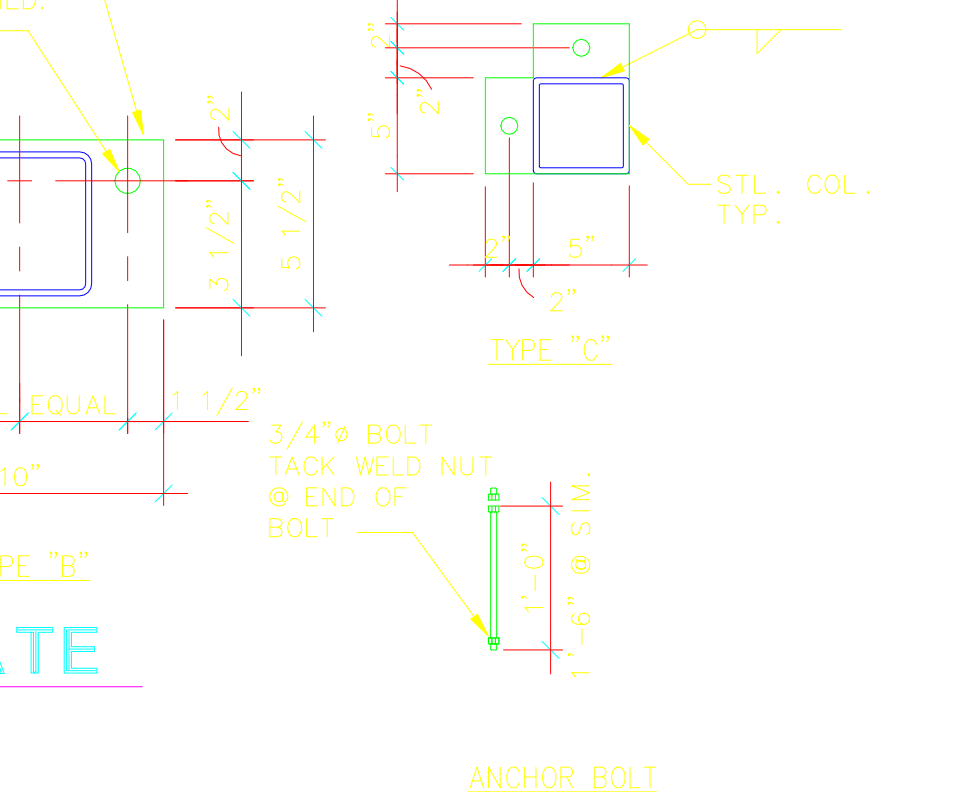
LAP SPLICE SCHED.

BAR SIZE	LAP	BAR SIZE	LAP
3	1'-10"	8	4'-9"
4	2'-3"	9	6'-1"
5	2'-8"	10	7'-3"
6	3'-2"		
7	3'-9"		

HANGER SCHEDULE

JOISTS	HANGER
2x8	U26
2x10	U28
2x12	U210
2-2x12	U210-2
3-2x12	U210-3
5 1/4"x11 1/4" PSL	HCLTV5.511
5 1/4"x18" PSL	GLTV5.518

- PROVIDE THESE HANGERS U.N.O. ON THE PLANS OR SECTIONS.
- ALL HANGERS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE.
- HANGERS FOR WOOD I-JOISTS SHALL BE SPECIFIED BY I-JOIST MANUFACTURER (U.N.O.).
- SLOPE AND SKEW HANGERS AS REQ'D. REF. PLANS AND ARCH. ELEVATIONS.



4 TYP. SECTION

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PERMIT ISSUE:
 22 SEPT. 2003

REVISIONS:
 28 OCT. 2003 ADDENDUM #1
 12 NOV. 2003 CITY COMMENTS
 18 DEC. 2003 CITY COMMENTS
 28 JAN. 2004 OWNER COMMENTS

PROTO 90 HWY 287 AT PEARL RD. CASA GRANDE, AZ

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