City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

WANDERIST OFFICE & RETAIL

3743 E. INDIAN SCHOOL ROAD, PHOENIX, AZ 85018

ABBREVIATIONS

A	AIR
A/C	AIR CONDITIONING
ACT	ACOUSTICAL TREATMENT (CEILING TILE OR PANEL)
AD	AREA DRAIN
ADD	ADDENDUM
ADJ	ADJUSTABLE
AFF	ABOVE FINISH FLOOR
AL, ALUM	ALUMINUM
ALT	ALTERNATE
ANOD	ANODIZED
APPROX	APPROXIMATE
ARCH	ARCHITECT, -URAL
BETW	BETWEEN
BLDG	BUILDING
BOC	BOTTOM OF CURB
BOF	BOTTOM OF FOOTING
CAB	CABINET
CARD	CARD READER
CB	CATCH BASIN
CEM	CEMENT
CJ	CONTROL JOINT
CL	CENTERLINE
CLG	CEILING
CLO	CLOSET
CLR	CLEAR, -ANCE
CM	CENTIMETER
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONC	CONCRETE
CONST, CONSTR	CONSTRUCTION
CONT	CONTINUE, -OUS
CORR	CORRIDOR
CTR	CENTER
DEMO	DEMOLISH, DEMOLITION
DEP, DEPR	DEPRESSED
DET, DTL	DETAIL
DIA	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DN	DOWN
DP	DAMPPROOFING
DWG	DRAWING
E	EAST
EA	EACH
EIFS	EXTERIOR INSULATION AND FINISH SYSTEM
EJ	EXPANSION JOINT
EL	ELEVATION
ELEC	ELECTRICAL
ELEV	ELEVATOR
EMER	EMERGENCY
EP	ELECTRICAL PANEL
EPS	EXPANDED POLYSTYRENE
EQ	EQUAL
EQUIP	EQUIPMENT
EX, (E)	EXISTING
EXP	EXPOSED
EXT	EXTERIOR
FA	FIRE ALARM
FD	FLOOR DRAIN
FDN	FOUNDATION
FE	FIRE EXTINGUISHER
FEC	FIRE EXTINGUISHER CABINET
FF	FINISHED FLOOR
FHC	FIRE HOSE CABINET
FIN	FINISH
FLR, FL	FLOOR, -ING
FOC	FACE OF CONCRETE
FOF	FACE OF FINISH
FOM	FACE OF MASONRY
FOS	FACE OF STUDS
FUT	FUTURE
GA	GAUGE
GAL, GALV	GALVANIZED
GFI	GROUND FAULT INTERRUPTER
GL	GLASS, GLAZING, GLAZED
GWB	GYPSUM BOARD
GYP	GYPSUM
HB	HOSE BIB
HGT, HT	HEIGHT
HM	HOLLOW METAL
HOR, HORIZ	HORIZONTAL
HSS	HOLLOW STEEL SHAPE
HVAC	HEATING, VENTILATING, AIR CONDITIONING
ID	INSIDE DIAMETER
INCL	INCLUDE, -D, -ING
INSUL	INSULATE, -ION, -D, -ING
INT	INTERIOR
JT	JOINT
KIT	KITCHEN
LAM	LAMINATE
LAV	LAVATORY
LT	LIGHT
LVL	LEVEL
MANUF	MANUFACTURER
MAS	MASONRY
MAT, MATL	MATERIAL, -S
MAX	MAXIMUM
MDF	MEDIUM DENSITY FIBERBOARD
MECH	MECHANIC, -AL
MED	MEDIUM
MEMB	MEMBRANE
MET	METAL, -LIC
MFD	MANUFACTURED

MICRO	MICROWAVE
MIN	MINIMUM
MIR	MIRROR
MISC	MISCELLANEOUS
MM	MILLIMETER, -S
MTL	METAL
MULL	MULLION
N	NORTH
NA	NOT APPLICABLE
NIC	NOT IN CONTRACT
NO, #	NUMBER
NOM	NOMINAL
NTS	NOT TO SCALE
OC	ON CENTER
OD	OVERFLOW DRAIN
OFCI	OWNER FURNISHED/CONTRACTOR INSTALLED
OFI	OWNER FURNISHED & INSTALLED
OH	OPPOSITE HAND
OPP	OPPOSITE
OSB	ORIENTED STRANDBOARD
OZ	OUNCE
PCF	POUNDS PER CUBIC FEET
PERF	PERFORATE, -D
PL	PLATE
PLAM	PLASTIC LAMINATE
PLAS	PLASTER
PLYWD	PLYWOOD
PNL	PANEL
PNT, P	PAINT, -ED
PORC	PORCELAIN
POS	POSTITION
PREFAB	PREFABRICATE, -D
PTN	PARTITION
R	RECEPTACLE
R	RISER
RAD	RADIUS
RCP	REFLECTED CEILING PLAN
RD	ROOF DRAIN
REF	REFERENCE
REFL	REFLECT, -ED, -IVE, -OR
REFR	REFRIGERATOR
REINF	REINFORCE
REM	REMOVE
REQ'D	REQUIRED
REV	REVISE, REVISION
RO	ROUGH OPENING
S	SOUTH
SCHED	SCHEDULE
SEAL	SEALANT
SECT	SECTION
SHT	SHEET
SHTHG	SHEATHING
SHWR	SHOWER
SIL	SILICONE
SIM	SIMILAR
SPEC	SPECIFICATION (S)
SPF	SPRAY POLYURETHANE FOAM
SPK	SPEAKER
SPR	SINGLE-PLY ROOFING
SQ	SQUARE
SST, SS	STAINLESS STEEL
STC	SOUND TRANSMISSION CLASS
STD	STANDARD
STL	STEEL
STOR	STORAGE
STR, STRL	STRUCTURE, STRUCTURAL
SYM	SYMMETRY, -IC(AL)
T	TEL/DATA OUTLET
T STAT	THERMOSTAT
T&G	TONGUE AND GROOVE
TEL	TELEPHONE
THK	THICK, -NESS
THRU	THROUGH
TOC	TOP OF CONCRETE, CURB
TOF	TOP OF FOOTING
TOP	TOP OF PAVEMENT
TOS	TOP OF STEEL
TOW	TOP OF WALL
TRANS, TPT	TRANSPARENT
TV	TELEVISION
TYP	TYPICAL
UC	UNDER CABINET
UL	UNDERWRITERS' LABORATORIES
UNO	UNLESS NOTED OTHERWISE
UON	UNLESS OTHERWISE NOTED
VCT	VINYL COMPOSITION TILE
VERT	VERTICAL
VIF	VERIFY IN FIELD
W	WEST
W	WIDTH
W/	WITH
W/O	WITHOUT
WC	WATER CLOSET
WD	WOOD
WDW	WINDOW
WF	WIDE FLANGE
WP	WATERPROOF, -ING
WP/C	WATERPROOFING, CRYSTALLINE
WT	WEIGHT
WWF	WELDED WIRE FABRIC
XPS	EXTRUDED POLYSTYRENE INSULATION

SHEET INDEX

GENERAL NOTES

IF THERE IS A CONFLICT BETWEEN ANY NOTES, DRAWINGS, OR SPECIFICATIONS, THE MOST RESTRICTIVE SHALL APPLY.

ALL WORK SHALL BE IN STRICT ACCORDANCE WITH THE GOVERNING EDITION OF THE INTERNATIONAL BUILDING CODE, OR SUCH OTHER LEGAL CODES, AND SHALL CONFORM TO ANY SPECIAL REQUIREMENTS OF ANY LENDING OR GOVERNMENTAL INSTITUTIONS.

CONTRACTOR AND SUBCONTRACTORS SHALL BE LICENSED IN THE STATE OF THE PROJECT SITE AND SHALL BE KNOWLEDGEABLE, SKILLED, AND COMPETENT TO PERFORM THE INTENDED WORK.

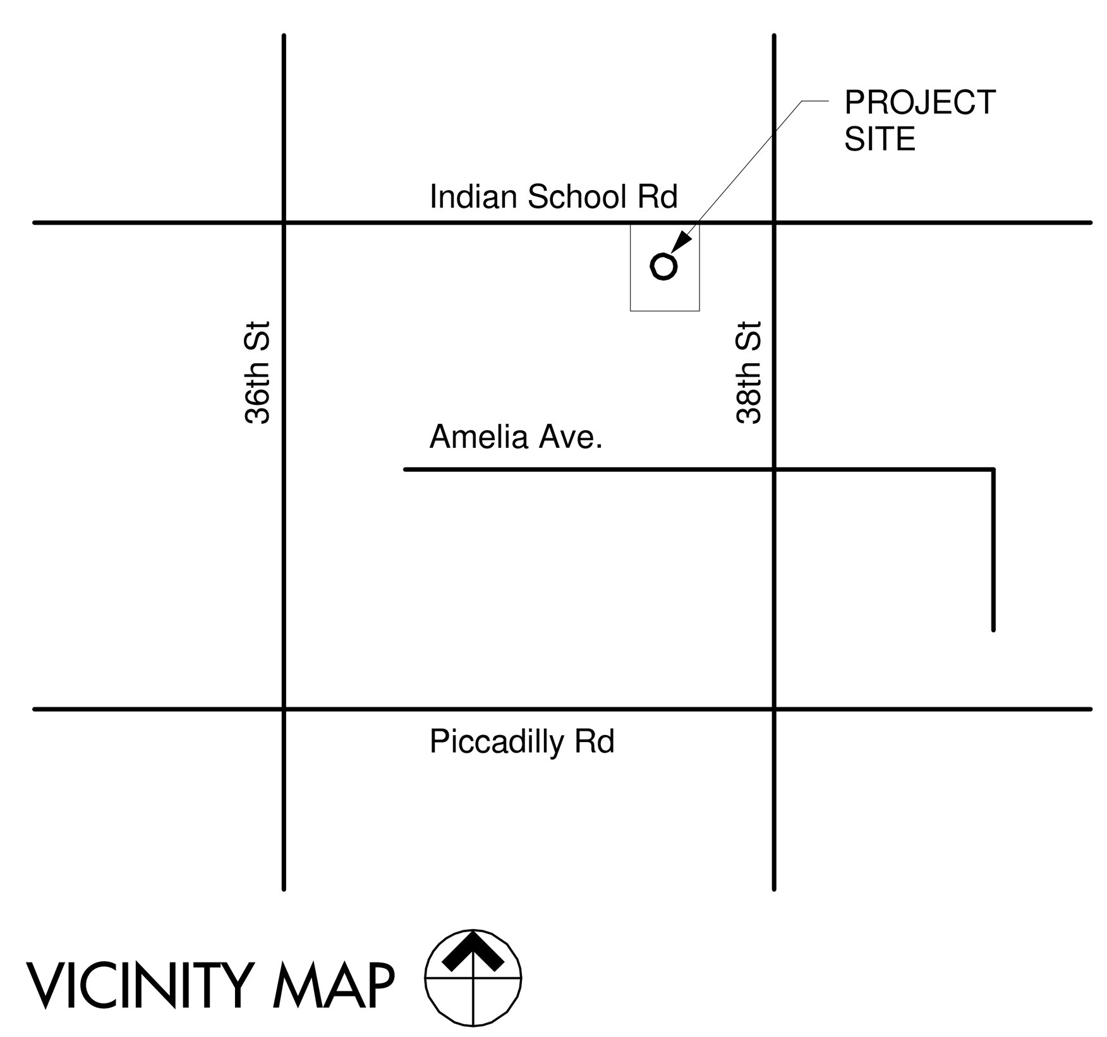
CONTRACTOR AND SUBCONTRACTOR SHALL ENSURE THAT ALL WORK IS PERFORMED IN A PROFESSIONAL MANER BY SKILLED CRAFTSMAN OR TRADESMAN AND SHALL REPLACE ANY ITEMS DAMAGED BY THE CONTRACTOR OR SUBCONTRACTORS AT NO COST TO THE OWNER. SUBCONTRACTORS SHOULD COOPERATE FULLY WITH EACH OTHER DURING THE COURSE OF CONSTRUCTION TO DETERMINE THE EXACT EXTENT AND OVERLAP OF EACH OTHERS WORK AND TO SUCCESSFULLY COMPLETE THE EXECUTION OF THE WORK IN A TIMELY MANNER.

CONTRACTOR AND SUBCONTRACTORS SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, AND PROCEDURES, AND FOR THE SAFETY PRECAUTIONS IN CONNECTION WITH THE WORK.

CONTRACTOR AND SUBCONTRACTORS SHALL AT ALL TIMES INDEMNIFY AND HOLD THE ARCHITECT HARMLESS AGAINST ALL LIABILITY FOR CLAIMS AND LIENS FOR LABOR PERFORMED OR MATERIALS USED OR FURNISHED TO BE USED ON THE JOB, INCLUDING ANY COSTS AND EXPENSES FOR ATTORNEY FEES AND ALL INCIDENTIAL OR CONSEQUENTIAL DAMAGES RESULTING TO THE ARCHITECT ARISING FROM SUCH CLAIMS.

ALL BIDS SUBMITTED AND ACCEPTED UNDER THIS CONTRACT SHALL INCLUDE ALL MATERIALS AND LABOR NECESSARY TO COMPLETE THE PROJECT IN ACCORDANCE WITH THE DOCUMENTS.

THE ARCHITECT NEITHER WARRANTS NOR GUARANTEES ANY CONSTRUCTION MATERIAL, EQUIPMENT, APPLIANCE, FIXTURE, HARDWARE, FINISH, OR MEAN/METHOD OF CONSTRUCTION. THE ARCHITECT SHALL NOT BE RESPONSIBLE FOR ANY PROJECT SITE GRADING OR DRAINAGE, NOR ANY TOXIC AND HAZARDOUS MATERIAL, GROUND EROSION, CORROSION, SUBSOIL, OR AIR AND WATER CONDITIONS, OR SIMILAR SUCH CONDITIONS OF THE PROJECT.



VICINITY MAP

PROJECT DESCRIPTION

NEW 3,760 SF OFFICE/RETAIL BUILDING CONSTRUCTED ON EXISTING SLAB ON GRADE.

DEFERRED SUBMITTALS

FIRELINE
FIRE SPRINKLER
FIRE ALARM
FIRE ACCESS
GATE ACCESS
KNOX BOX

SEPARATE SUBMITTALS

SIGNAGE
LANDSCAPE
INVENTORY/SALVAGE
GATES

CODE COMPLIANCE

2018 INTERNATIONAL BUILDING CODE
2018 UNIFORM PLUMBING CODE
2018 INTERNATIONAL MECHANICAL CODE
2017 NATIONAL ELECTRIC CODE
2018 INTERNATIONAL FUEL AND GAS CODE
2018 INTERNATIONAL ENERGY CONSERVATION CODE
2010 ADA STANDARDS FOR ACCESSIBLE DESIGN
2012 INTERNATIONAL FIRE CODE

SPECIAL INSPECTIONS

SEE STRUCTURAL S.02

CONTRACTOR & OWNER NOTICE

THIS PROJECT HAS BEEN PERMITTED UNDER THE CITY OF PHOENIX SELF-CERTIFICATION PROGRAM. THE PROJECT IS SUBJECT TO AUDIT AND FIELD INSPECTION BY THE PLANNING & DEVELOPMENT DEPARTMENT. IF THE CONSTRUCTION OF THE PROJECT IS CONTRARY TO, OR DOES NOT MEET THE STANDARD OF THE CITY OF PHOENIX BUILDING CONSTRUCTION CODES, THE OWNER, AT HIS/HER OWN EXPENSE, SHALL REMOVE OR MODIFY ANY AND ALL COMPONENTS THAT DO NOT CONFORM. ANY DEVIATIONS FROM THE APPROVED PLAN MUST BE COORDINATED IN ADVANCE WITH THE CITY INSPECTOR AND REVISED PLANS OR SKETCHES MUST BE PROVIDED BY THE SELF-CERTIFIED PROFESSIONAL.

CERTIFICATION STATEMENT

I HEREBY CERTIFY THAT THESE DRAWINGS ARE PREPARED BY ME, UNDER MY SUPERVISION, OR REVIEWED BY ME AND TO THE BEST OF MY PROFESSIONAL KNOWLEDGE CONFORM TO THE PHOENIX BUILDING CONSTRUCTION CODE.

SELF CERTIFIED BY: [illegible] DATE: 03/11/19
DONALD ANDREWS CERTIFICATE #45

- PLANS WERE PREPARED BY OR UNDER THE DIRECT SUPERVISION OF, OR REVIEWED BY THE SELF-CERTIFIED PROFESSIONAL,
- PLANS ARE COMPLETE,
- THE PLANS ARE, AS OF THE DATE OF SUBMISSION, IN ACCORDANCE WITH THE REQUIREMENTS OF THE PHOENIX BUILDING CONSTRUCTION CODE AND ALL OTHER APPLICABLE LAWS.

City of Phoenix
PLANNING & DEVELOPMENT DEPARTMENT
Self-Certified Plans - Official Construction Set

This set of Self Certified plans shall be kept at the construction site. Acceptance of these plans shall not prevent the City from requiring correction of errors in the plans where such errors are subsequently found to be in violation of any code, law, ordinance, health, safety, or other design issues.

IBC - Stevan Varnell 602-534-8705

IMC-UPC - John Lanoue 602-534-2881

KIVA #18-1372
SDEV #1800276
PAPP #1806619
PRLC
QS Q16-36

ERWIN | ARCHITECTURE DEVELOPMENT

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DON ANDREWS JR.
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MESA, AZ 85202
(E) DGRAPSAS@UNITEDSTR.COM
(P) 480.382.9768

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PETERSON ENGINEERING
DAVID MCKERCHER
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(E) DAMEM@MPECONSULT.COM
(P) 602.388.1716

LANDSCAPE
NORRIS DESIGN
JOEL THOMAS
(E) JTHOMAS@NORRIS-DESIGN.COM
(P) 512.900.7888

SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE
-	PRE-APP MTG	10.10.18
-	MINOR SITE PLAN	01.09.19
-	CITY SUBMITTAL	03.06.19

REGISTERED ARCHITECT
CERTIFICATE NO. 54596
WILLIAM FRANKLIN ERWIN
Date Signed 3/6/19
ARIZONA, U.S.A.

Expires 6.30.19

Owner	JONATHAN PITT
Proj. Name	WANDERIST OFFICE & RETAIL

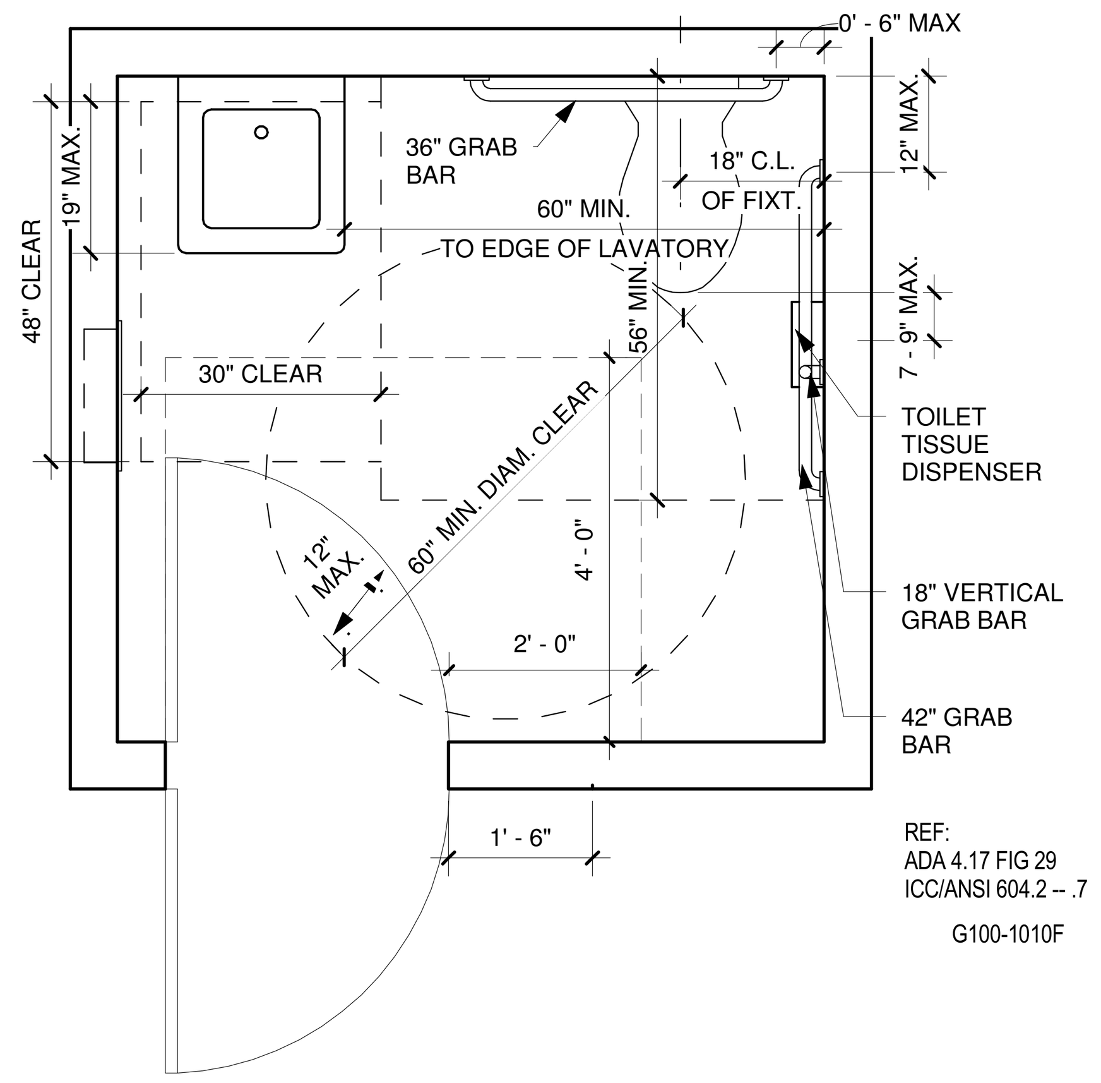
COVER SHEET

Date	03/06/19

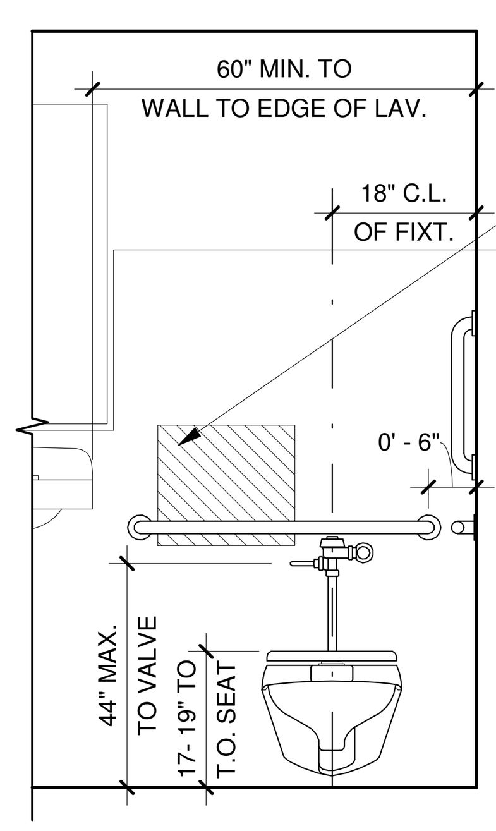
A000

Scale	1/4" = 1'-0"

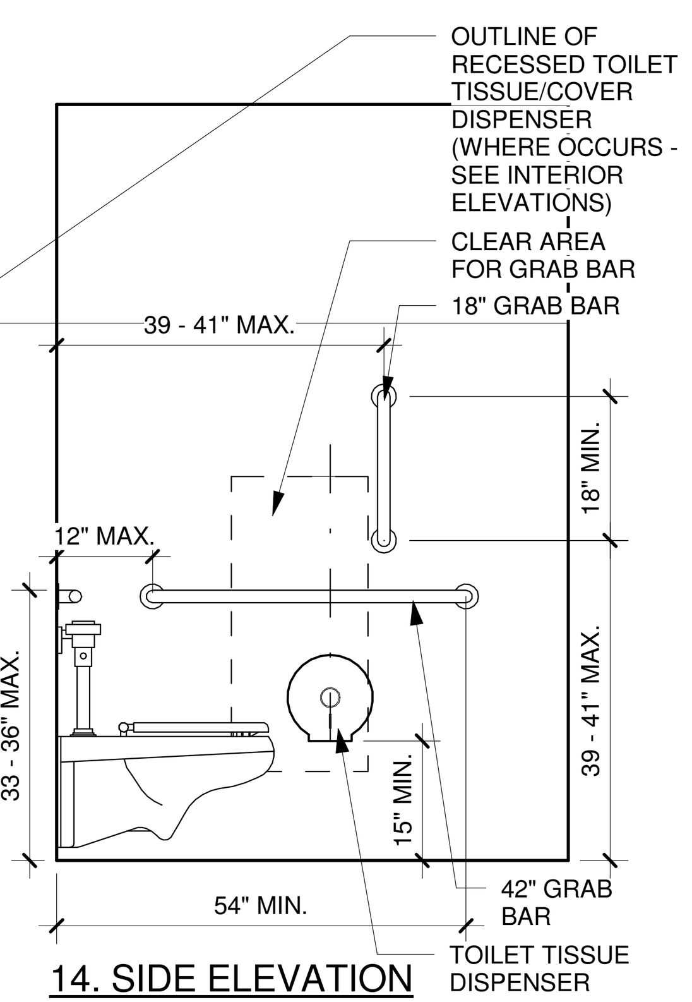
City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19



16. BATHROOM PLAN

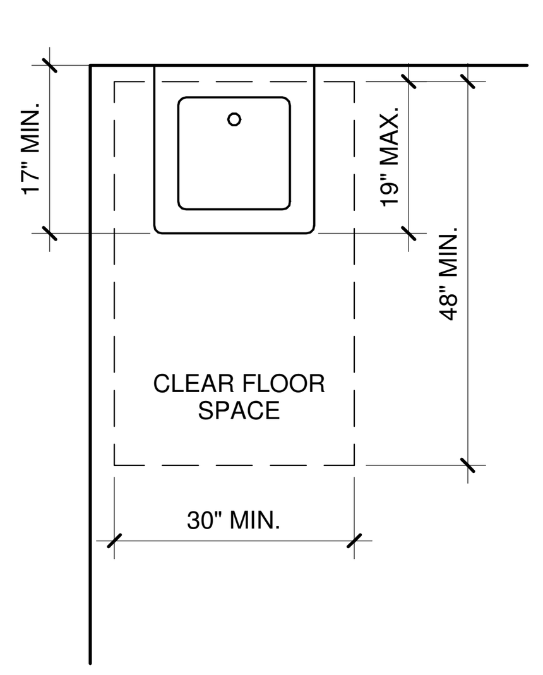


15. FRONT ELEVATION

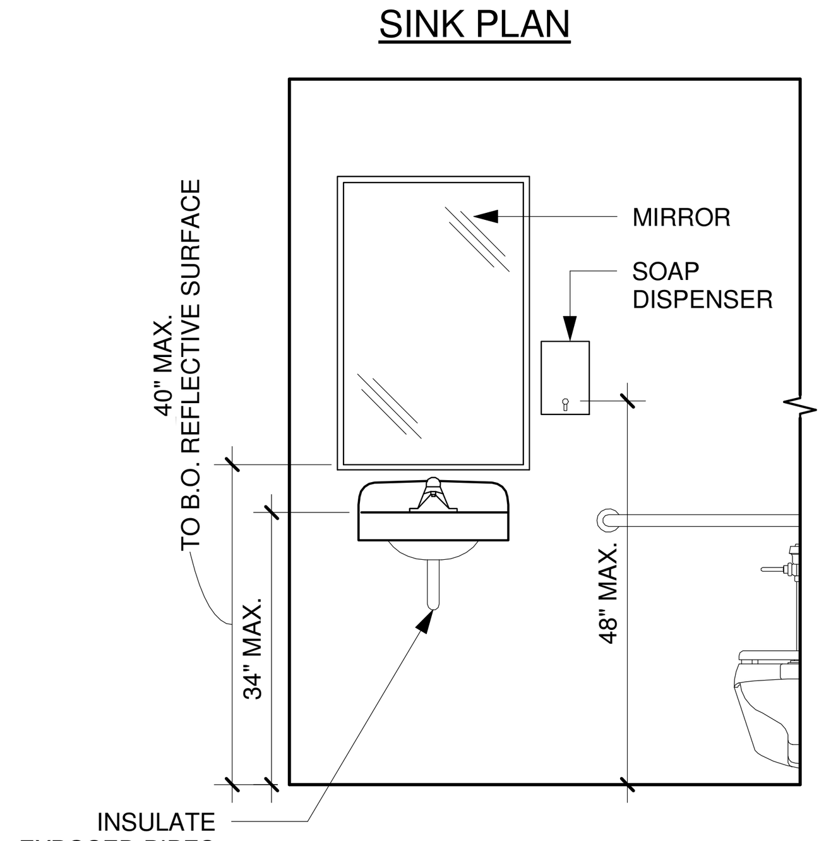


14. SIDE ELEVATION

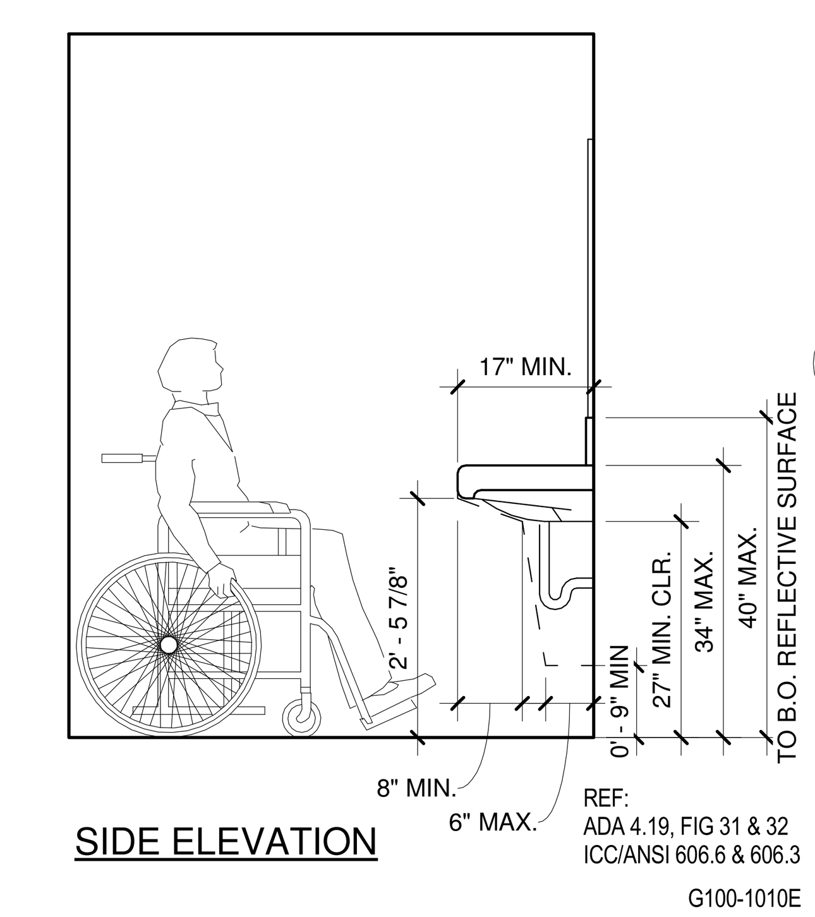
3 SINGLE TOILET ROOM
1/2" = 1'-0"



SINK PLAN



FRONT ELEVATION



SIDE ELEVATION

2 ACCESSIBLE LAVATORY
1/2" = 1'-0"

PLUMBING FIXTURE COUNTS

IBC TABLE 2902.1

CLASSIFICATION	OCCUPANCY	WATER CLOSETS	LAVS	TUB / SHOWERS	DRINKING FOUNTAINS	OTHER
BUSINESS	B	1 per 25 for the first 50 and 1 per 50 for the remainder exceeding 50 14/50 = .28	1 per 40 for the first 80 and 1 per 80 for the remainder exceeding 80 14/40 = .35	-	1 per 100 14/100 = .14	1 Service Sink
MERCANTILE	M	1 per 500 78/500 = .15	1 per 750 78/750 = .10	-	1 per 1000 78/1000 = .078	1 Service Sink

WATER CLOSETS
1 REQUIRED
2 PROVIDED

DRINKING FOUNTAINS
1 REQUIRED
WATER COOLER PROVIDED IN LIEU OF DRINKING FOUNTAIN

SERVICE SINK
1 REQUIRED
1 PROVIDED

NOTE: PER IBC 2902.2 SEPARATE FACILITIES ARE NOT REQ'D FOR EA. SEX IN MERCANTILE OCCUPANCIES W/ MAXIMUM OCCUPANT LOAD OF 100 OR FEWER OR BUSINESS OCCUPANCIES W/ 25 OR FEWER. PROVIDE UNISEX SIGNAGE PER IBC 2902.4

IECC DATA

ALL NEW FENESTRATION MUST MEET REQUIREMENTS OF 2012 IECC TABLE C402.3 CLIMATE ZONE 2

TABLE C402.3
BUILDING ENVELOPE REQUIREMENTS: FENESTRATION

CLIMATE ZONE	1	2	3	4 EXCEPT MARINE	5 AND MARINE 4	6	7	8
Vertical fenestration								
U-factor								
Fixed fenestration	0.50	0.50	0.46	0.38	0.38	0.36	0.29	0.29
Operable fenestration	0.65	0.65	0.60	0.45	0.45	0.43	0.37	0.37
Entrance doors	1.10	0.83	0.77	0.77	0.77	0.77	0.77	0.77
SHGC								
SHGC	0.25	0.25	0.25	0.40	0.40	0.40	0.45	0.45
Skylights								
U-factor	0.75	0.65	0.55	0.50	0.50	0.50	0.50	0.50
SHGC	0.35	0.35	0.35	0.40	0.40	0.40	NR	NR

NR = No requirement.

OCCUPANT LOAD

OCCUPANT LOAD TABLE

AREA NAME	USE GROUP	AREA	NET OR GROSS	LOAD FACTOR	OCCUPANT LOAD
OFFICE & STOCK ROOM	B	1408 SF	GROSS	100 SF	14
RETAIL AREA	M	2336 SF	GROSS	30 SF	78

NO SEPARATION BETWEEN USES REQUIRED PER TABLE 508.4

EXIT ARRANGEMENT

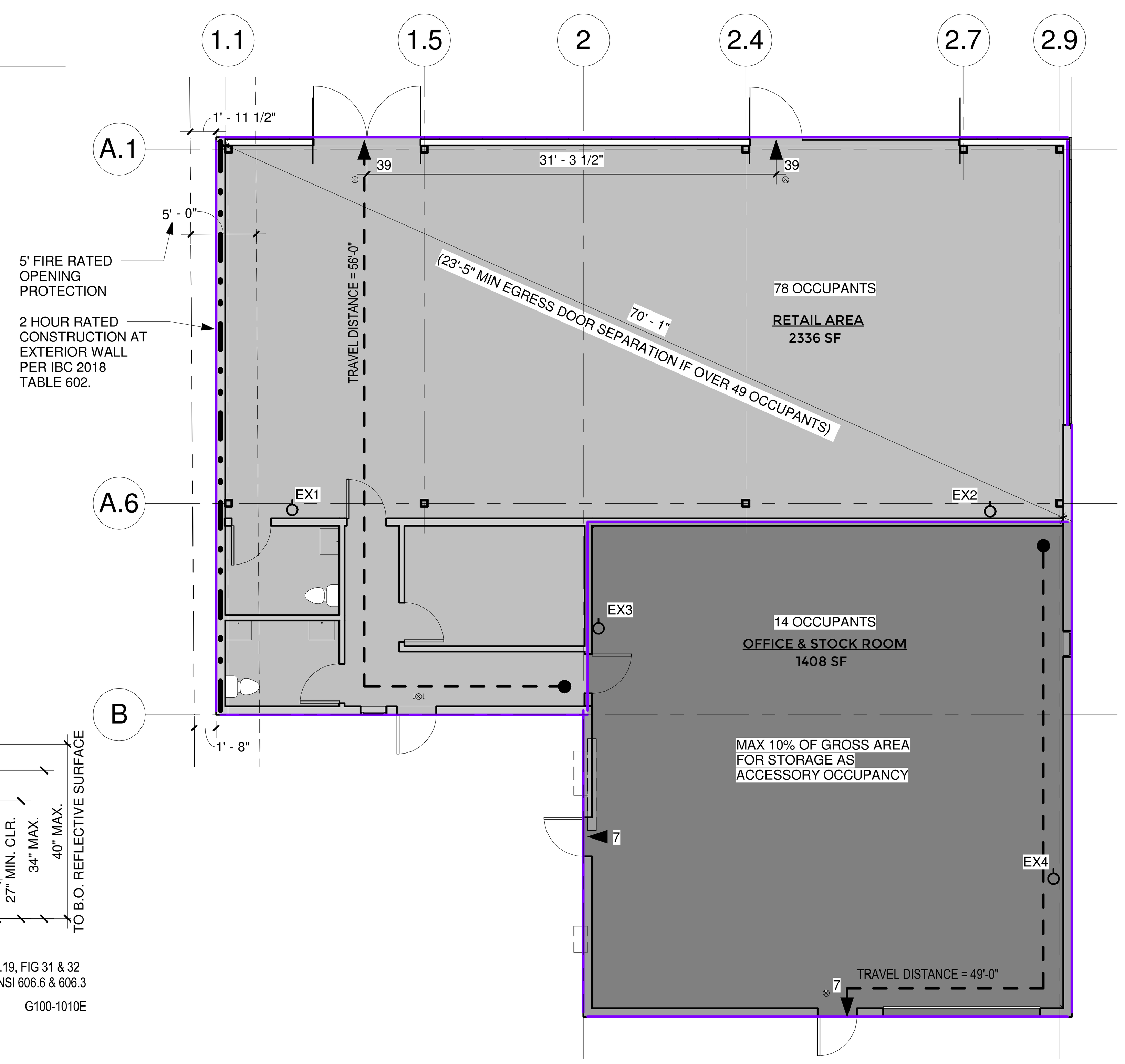
REFERENCE IBC SECTION 1015 & 1021

A MINIMUM OF TWO EXITS WILL BE PROVIDED WHERE EVER THE OCCUPANT LOAD IS GREATER THAN 49 PERSONS IN B AND M USES.

RETAIL AREA
2 EXITS REQUIRED
2 EXITS PROVIDED

PRINT AREA
1 EXIT REQUIRED
2 EXITS PROVIDED

WHERE EVER TWO EXITS ARE REQUIRED FROM ANY PORTION OF THE BUILDING, THE EXITS WILL BE LOCATED A DISTANCE OF NOT LESS THAN ONE-THIRD OF THE LENGTH OF THE MAXIMUM OVERALL DIAGONAL DIMENSION OF THE SPACE.



1 CODE PLAN AND EXITING DIAGRAM
1/8" = 1'-0"

EGRESS COMPONENTS

EXIT SIGNS:
1. EXITS AND EXIT ACCESS DOORS WILL BE MARKED BY AN APPROVED EXIT SIGN READILY VISIBLE FROM ANY DIRECTION OF EGRESS TRAVEL. EXIT SIGN PLACEMENT WILL BE SUCH THAT NO POINT IN A CORRIDOR IS MORE THAN 100 FEET, OR THE LISTED VIEWING DISTANCE FRO THE SIGN, WHICH EVER IS LESS, FROM THE NEAREST VISIBLE EXIT SIGN.

2. EXIT SIGN LETTERS TO BE NOT LESS THAN 2" WIDE X 6" HIGH (EXCEPT LETTER I). AND THE MINIMUM SPACING BETWEEN THE LETTERS WILL NOT BE LESS THAN (3/4) INCHES. IBC FIGURE 1011.6.1

4. EXIT SIGN LETTERS TO BE IN HIGH CONTRAST WITH THE BACKGROUND AND CLEARLY DISCERNABLE WHEN THE MEANS OF EGRESS ILLUMINATION IS OR IS NOT ENERGIZED.

5. TO ENSURE CONTINUED ILLUMINATION FOR A DURATION OF NOT LESS THAN 90 MINUTES IN CASE OF PRIMARY POWER LOSS, THE SIGN WILL BE CONNECTED TO AN EMERGENCY POWER SYSTEM PROVIDED FROM AN ONSITE GENERATOR.

DOORS:
1. MINIMUM CLEAR WIDTH SHALL BE .2 INCHES PER OCCUPANT SERVED. MINIMUM CLEAR WIDTH SHALL BE REDUCED TO .15 INCHES PER OCCUPANT SERVED IN BUILDING EQUIPPED THROUGHOUT AUTOMATIC SPRINKLER SYSTEM & EMERGENCY VOICE/ALARM COMMUNICATION SYSTEM, BUT NOT LESS THAN 32 INCHES. IBC, SECTION 1005.3.2 AND TABLE 1008.1.1

2. MINIMUM HEIGHT SHALL BE 80 INCHES. IBC, SECT 1008.1.1

3. MAXIMUM WIDTH OF SWINGING DOOR LEAF IS 48 INCHES. IBC, SECT 1008.1.1

4. DOORS WILL BE SIDE HINGED SWINGING TYPE, AND WILL SWING IN THE DIRECTION OF TRAVEL WHERE THE AREA SERVED HAS AN OCCUPANT OF 50 OR MORE. IBC SECT 1008.1.2

5. DOORS WILL BE SET IN MOTION WHEN SUBJECTED TO A 30 POUND FORCE, AND SWING TO THE FULLY OPEN POSITION WHEN SUBJECTED TO A 15 POUND FORCE. IBC, TABLE 1008.1.3

6. DOORS WILL BE OPERABLE FROM THE INSIDE WITHOUT THE USE OF A KEY, SPECIAL KNOWLEDGE, OR SPECIAL EFFORT.

CORRIDORS:
1. MINIMUM CLEAR WIDTH SHALL BE .15 INCHES PER OCCUPANT SERVED IN BUILDING EQUIPPED THROUGHOUT AUTOMATIC SPRINKLER SYSTEM & EMERGENCY VOICE/ALARM COMMUNICATION SYSTEM, BUT NOT LESS THAN 44 INCHES. IBC, SECT 1005.3.2 & 1018.2

2. MIN CLEAR WIDTH WITH AN OCCUPANT CAP OF 50 OR LESS IS 36 INCHES. IBC SECT 1018.2

3. THE MAXIMUM LENGTH OF DEAD-END CORRIDORS IS 50 FEET FOR GROUP B, M, S, & R-2 AND 20 FEET FOR ALL OTHER OCCUPANCIES. IBC, SECTION 1018.4

INTERVENING ROOMS:
1. EGRESS FROM A ROOM OR SPACE MAY NOT PASS THROUGH ADJOINING OR INTERVENING ROOMS OR AREAS, EXCEPT WHERE SUCH ADJOINING ROOMS OR AREAS ARE ACCESSORY TO THE AREA SERVED, NOT A HIGH-HAZARD OCCUPANCY, AND PROVIDE A DISCERNABLE PATH OF EGRESS TRAVEL TO AN EXIT. IBC SECT 1014.2

2. EGRESS MAY NOT PASS THROUGH STORAGE ROOMS, CLOSETS, OR SPACES USED FOR SIMILAR PURPOSES.

3. EXIT ACCESS MAY NOT PASS THROUGH A ROOM THAT CAN BE LOCKED TO PREVENT EGRESS. IBC, SECTION 1014.2

CODE DATA

2018 CITY OF PHOENIX BUILDING CONSTRUCTION CODE INCLUDING THE FOLLOWING CODES AND AMENDMENTS:
2018 IBC (INTERNATIONAL BUILDING CODE)
2018 IECC (INTERNATIONAL ENERGY CONSERVATION CODE)
2018 IFC (INTERNATIONAL FIRE CODE)
2017 NEC (NATIONAL ELECTRIC CODE)
2018 IMC (INTERNATIONAL MECHANICAL CODE)
2018 IPC (INTERNATIONAL PLUMBING CODE
2018 UPC (UNIFORM PLUMBING CODE)
ACCESSIBILITY:
CHAPTER 11 OF THE IBC
2009 ANSI A117.1, ACCESSIBLE AND USABLE BUILDINGS AND FACILITIES
2010 ADA STANDARDS FOR ACCESSIBLE DESIGN
OTHER:
VARIOUS NFPA CODES AND STANDARDS AS REFERENCED BY CODES LISTED ABOVE

FIRE EXTINGUISHERS

EX — PER IBC TABLE SECTION 906 PROVIDE 2-A RATED EXTINGUISHERS. MAX TRAVEL DISTANCE TO EXTINGUISHER 75'-0". MAXIMUM FLOOR AREA PER UNIT OF "A" IS 3,000 SF.

⊗ EXIT SIGN

SELF CERTIFIED BY: [illegible] DATE: 03/11/19
DONALD ANDREWS CERTIFICATE #45

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ZONING DATA

PARCELS: 127-25-120-J & 127-25-122
ZONING: C-1
ADDRESS: 3743 E. INDIAN SCHOOL ROAD, PHOENIX, AZ 85018

CONSTRUCTION TYPE

TYPE VB - SPRINKLERED (UNDER SEPARATE PERMIT)
OCCUPANCY CLASSIFICATION B, M
2018 IECC CLIMATE ZONE - 2B

BUILDING LIMITATIONS

REFERENCE IBC TABLE 504.3, SECTION 504.4, AND SECTION 506.2

GROUP	TYPE 5B	
M	2 / 27,000	UL/UL
B	3 / 27,000	

HEIGHT
AREA

MAX HEIGHT 60'

THE PROPOSED BUILDING IS A SINGLE STORY

OCCUPANCY CLASSIFICATION

REFERENCE IBC TABLE 1004.1.2

AREA OF USE	OCCUPANCY	LOAD FACTOR
PARKING GARAGE	S-2	200 GROSS
STORAGE	S-1	300 GROSS
MECH/ELEC	S-1	300 GROSS
BUSINESS	***B***	***100 GROSS***
MERCANTILE	***M***	***30 GROSS***
SWIMMING POOL	A-3	50 GROSS
SWIMMING POOL DECK	A-3	15 GROSS
RESIDENTIAL UNIT	R-2	200 GROSS
RES. BALCONY/PATIO	R-2	200 GROSS
CIRCULATION SPACE	N/A	100 GROSS
ASSEMBLY (UNCONCENTRATED)	A-3	15 NET
ASSEMBLY (CONCENTRATED)	A-3	7 NET

FIRE RESISTANCE RATING

BUILDING ELEMENT		TYPE 5B	
STRUCTURAL FRAME		0 HR	TABLE 601
EXTERIOR NON-BEARING WALLS	X<5'	2 HR (M)	TABLE 602
INTERIOR NON-BEARING WALLS		0 HR	
EXTERIOR BEARING WALLS		0 HR	TABLE 601
INTERIOR BEARING WALLS		0 HR	TABLE 601
FLOOR CONSTRUCTION		0 HR	TABLE 601
ROOF CONSTRUCTION		0 HR	TABLE 601

SAFETY GLAZING

GLAZING LOCATION	MINIMUM CATEGORY CLASSIFICATION 9 SF OR LESS	MORE THAN 9 SF
FRAMED SWING DOORS	I	II
UNFRAMED SWING DOORS	I	II
TUB AND SHOWER ENCLOSURE	NR	II
ADJACENT TO DOORS	I	II
INDIVIDUAL PANELS	II	II
ADJACENT WALKING SURFACE	NR	II

SAFETY GLAZING WILL NOT BE PROVIDED WHERE ALLOWED BY IBC 2406.3

EXIT TRAVEL DISTANCE

MAXIMUM EXIT ACCESS TRAVEL DISTANCE — **IBC, TABLE 1016.2**

GROUP M — ***250 FEET***
GROUP B — ***300 FEET***

MAXIMUM COMMON PATH OF EGRESS TRAVEL DISTANCE — **IBC, TABLE 1014.3**

GROUP M — ***75 FEET***
GROUP B — ***100 FEET***

DISTANCES REFLECT THE PRESENCE OF AUTOMATIC SPRINKLER SYSTEM

KIVA #18-1372
SDEV #1800276
PAPP #1806619
PRLC
QS Q16-36

ERWIN | ARCHITECTURE DEVELOPMENT

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SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE
-	PRE-APP MTG	10.10.18
-	MINOR SITE PLAN	01.09.19
-	CITY SUBMITTAL	03.06.19

REGISTERED ARCHITECT
CERTIFICATE NO.
54596
WILLIAM FRANKLIN ERWIN
Date Signed 3/6/19
ARIZONA, U.S.A.

Expires 6.30.19

Owner JONATHAN PITT
Proj. Name WANDERIST OFFICE & RETAIL

CODE DATA & EGRESS PLAN

Date 03/06/19

A001

Scale As indicated

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

COM*check* Software Version 4.1.1.0

Envelope Compliance Certificate

Project Information

Energy Code:	2018 IECC
Project Title:	Wanderist Office & Retail
Location:	Phoenix, Arizona
Climate Zone:	2b
Project Type:	New Construction
Vertical Glazing / Wall Area:	29%
Skylight / Roof Area	0%

Construction Site:
3743 E. Indian School Road
Phoenix, AZ 85018

Owner/Agent:
Jonathan Pitt
Superluxe Screen Printing
3007 N 73Rd St Ste. E
Scottsdale, AZ 85251
480.247.6653

Designer/Contractor:
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Erwin Architecture & Development, LLC
5911 W. Park Ave
Chandler, AZ 85226
602.677.8372
will@erwinarchitecture.com

Additional Efficiency Package(s)

Enhanced Envelope Performance

Building Area	Floor Area
1-Retail with office, print area, and support space (Retail) : Nonresidential	3744

Envelope Assemblies

Assembly	Gross Area or Perimeter	Cavity R-Value	Cont. R-Value	Proposed U-Factor	Budget U-Factor(a)
Roof 1: Attic Roof with Wood Joists, [Bldg. Use 1 - Retail with office, print area, and support space]	3744	28.0	10.0	0.026	0.027
Skylight 1: Metal Frame with Thermal Break:Glass, With Curb, Perf. Specs.: Product ID 3762, SHGC 0.35, [Bldg. Use 1 - Retail with office, print area, and support space] (c)	5	---	---	0.650	0.650
Floor 1: Slab-On-Grade:Unheated, [Bldg. Use 1 - Retail with office, print area, and support space] (d)	265	---	---	0.730	0.730
NORTH					
Exterior Wall 5: Wood-Framed, 24" o .c., [Bldg. Use 1 - Retail with office, print area, and support space]	980	20.0	0.0	0.062	0.064
Window 4: Other Window:Fixed, Perf. Specs.: Product ID NA, SHGC 0.25, [Bldg. Use 1 - Retail with office, print area, and support space] (c)	673	---	---	0.180	0.500
Window 5: Other Window:Fixed, Perf. Specs.: Product ID NA, SHGC 0.33, PF 0.38, [Bldg. Use 1 - Retail with office, print area, and support space] (c)	96	---	---	0.500	0.500
Door 4: Glass (> 50% glazing):Nonmetal Frame, Entrance Door, Perf. Specs.: Product ID NA, SHGC 0.37, PF 0.38, [Bldg. Use 1 - Retail with office, print area, and support space] (c)	99	---	---	0.830	0.830
EAST					
Exterior Wall 1: Wood-Framed, 24" o .c., [Bldg. Use 1 - Retail with	1007	20.0	0.0	0.062	0.064

Project Title: Wanderist Office & Retail — Report date: 03/04/19
Data filename: C:\Users\stocci\Desktop\Wanderist.cck — Page 1 of 11

Assembly	Gross Area or Perimeter	Cavity R-Value	Cont. R-Value	Proposed U-Factor	Budget U-Factor(a)
office, print area, and support space]					
Window 1: Other Window:Fixed, Perf. Specs.: Product ID NA, SHGC 0.25, [Bldg. Use 1 - Retail with office, print area, and support space] (c)	275	---	---	0.180	0.500
Window 3: Metal Frame:Operable, Perf. Specs.: Product ID NA, SHGC 0.25, [Bldg. Use 1 - Retail with office, print area, and support space] (c)	22	---	---	0.650	0.650
SOUTH					
Exterior Wall 1 copy 1: Wood-Framed, 24" o .c., [Bldg. Use 1 - Retail with office, print area, and support space]	980	20.0	0.0	0.062	0.064
Window 2: Metal Frame:Operable, Perf. Specs.: Product ID NA, SHGC 0.25, [Bldg. Use 1 - Retail with office, print area, and support space] (c)	7	---	---	0.650	0.650
Door 1: Insulated Metal, Swinging, [Bldg. Use 1 - Retail with office, print area, and support space]	42	---	---	0.610	0.610
Door 2: Insulated Metal, Garage door 14% glazing, [Bldg. Use 1 - Retail with office, print area, and support space]	126	---	---	0.310	0.310
WEST					
Exterior Wall 3: Wood-Framed, 16" o.c., [Bldg. Use 1 - Retail with office, print area, and support space]	750	20.0	0.0	0.064	0.064
Exterior Wall 4: Wood-Framed, 24" o .c., [Bldg. Use 1 - Retail with office, print area, and support space]	340	20.0	0.0	0.062	0.064
Door 3: Insulated Metal, Swinging, [Bldg. Use 1 - Retail with office, print area, and support space]	21	---	---	0.610	0.610

(a) Budget U-factors are used for software baseline calculations ONLY, and are not code requirements.
(b) 'Other' components require supporting documentation for proposed U-factors.
(c) Fenestration product performance must be certified in accordance with NFRC and requires supporting documentation.
(d) Slab-On-Grade proposed and budget U-factors shown in table are F-factors.

Envelope PASSES: Design 12% better than code

Envelope Compliance Statement

Compliance Statement: The proposed envelope design represented in this document is consistent with the building plans, specifications, and other calculations submitted with this permit application. The proposed envelope systems have been designed to meet the 2018 IECC requirements in COM*check* Version 4.1.1.0 and to comply with any applicable mandatory requirements listed in the Inspection Checklist.

William Erwin, President	[illegible]	3/4/19
Name - Title	Signature	Date

Project Title: Wanderist Office & Retail — Report date: 03/04/19
Data filename: C:\Users\stocci\Desktop\Wanderist.cck — Page 2 of 11

SELF CERTIFIED BY: [illegible] DATE: 03/06/2019
DONALD ANDREWS CERTIFICATE #45

- PLANS WERE PREPARED BY OR UNDER THE DIRECT SUPERVISION OF, OR REVIEWED BY THE SELF-CERTIFIED PROFESSIONAL,
- PLANS ARE COMPLETE,
- THE PLANS ARE, AS OF THE DATE OF SUBMISSION, IN ACCORDANCE WITH THE REQUIREMENTS OF THE PHOENIX BUILDING CONSTRUCTION CODE AND ALL OTHER APPLICABLE LAWS.

KIVA #18-1372
SDEV #1800276
PAPP #1806619
PRLC
QS Q16-36

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Owner JONATHAN PITT
Proj. Name WANDERIST OFFICE & RETAIL

ENVELOPE COMCHECK

Date 03/06/19

A002

Scale

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

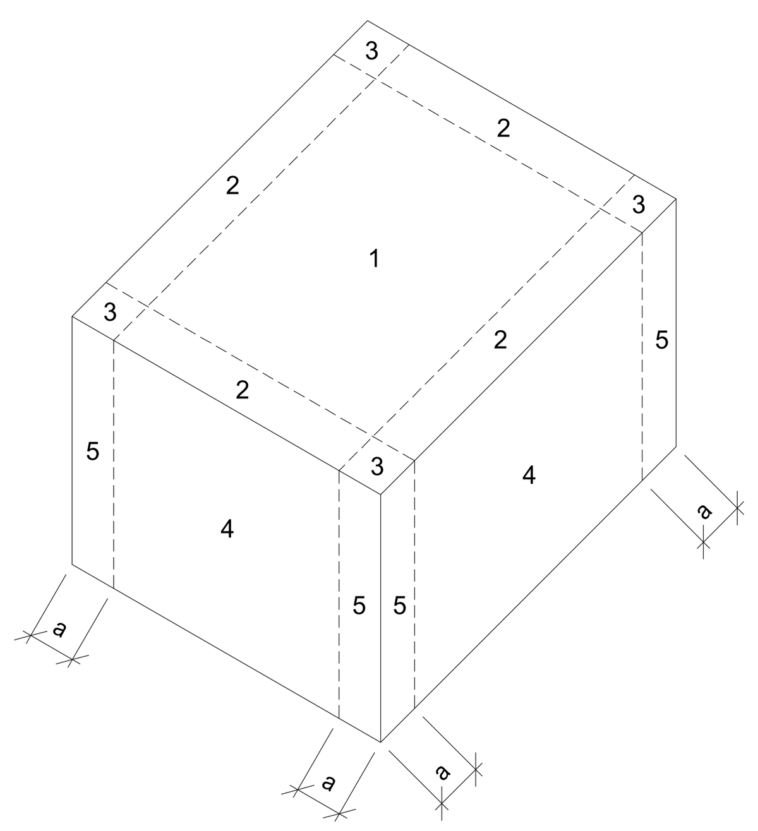
CODE:

2018 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC)

DESIGN LOADS:

1. ROOF:
   LIVE LOAD____20 PSF (REDUCIBLE)
   DEAD LOAD____18 PSF*
   SUPERIMPOSED DEAD LOAD ON TRUSSES____16 PSF*
   (13PSF TOP CHORD / 3PSF BOTTOM CHORD)
   *ROOF DEAD LOAD INCLUDES A 1.5 PSF ALLOWANCE FOR FIRE SPRINKLER PIPE BRANCH LINES LESS THAN 3 INCH DIAMETER. DESIGNERS OF STRUCTURAL MEMBERS ENGINEERED BY OTHERS SHALL COORDINATE THE WEIGHTS AND LOCATIONS OF ALL FIRE SPRINKLER BRANCH LINES GREATER THAN 3 INCH DIAMETER AND DESIGN FOR ADDITIONAL LOADS ACCORDINGLY.
2. WIND LOAD:
   RISK CATEGORY____II
   BASIC WIND SPEED, V :____102 MPH
   EXPOSURE CATEGORY:____C
   IMPORTANCE FACTOR, Iw:____1.0
   MEAN ROOF HEIGHT:____15 FT
   INTERNAL PRESSURE COEFFICIENT____+/- 0.18
   G:____0.85
   Kd:____0.85
   Kzt:____1.0
   Kz:____1.12
   ENCLOSURE CLASSIFICATION: ____ENCLOSED BUILDING
3. SEISMIC LOADS:
   RISK CATEGORY____II
   IMPORTANCE FACTOR, Ie :____1.0
   SEISMIC SITE CLASS:____C
   Ss:____0.186
   S1:____0.066
   SDS:____0.297
   SD1:____0.159
   SEISMIC DESIGN CATEGORY:____B
   BASIC SEISMIC FORCE RESISTING SYSTEM:
   STRUCTURAL STEEL SYSTEMS NOT SPECIFICALLY DETAILED FOR SEISMIC AND LIGHT FRAME WOOD SHEAR WALLS WITH STRUCTURAL SHEAR PANELS
   R:____3.0
   Ω:____2.5
   Cd:____3.0
   Cs:____0.01
   BASE SHEAR, V:____0.01W

COMPONENTS AND CLADDING WIND PRESSURE (ULTIMATE):



a = 12 FEET

SURFACE PRESSURE (PSF)			
COMPONENT ZONE	EFFECTIVE WIND AREA (SQ. FT)		
	10 SQ. FT	50 SQ. FT	100 SQ. FT
NEGATIVE PRESSURE: ZONE 1	-36.1 PSF	-30.6 PSF	-28.2 PSF
POSITIVE PRESSURE: ZONE 1	16.0 PSF	16.0 PSF	16.0 PSF
NEGATIVE PRESSURE: ZONE 2	-47.7 PSF	-40.5 PSF	-37.5 PSF
NEGATIVE PRESSURE: ZONE 3	-47.7 PSF	-40 .5PSF	-37.5 PSF
POSITIVE PRESSURE: ZONE 2&3	20.8 PSF	18.6 PSF	17.7 PSF
PARAPET	63.9 PSF	54.3 PSF	50.2 PSF
NEGATIVE PRESSURE: ZONE 4	-20.8 PSF	-20.4 PSF	-19.4 PSF
NEGATIVE PRESSURE: ZONE 5	-38.1 PSF	-21.6 PSF	-19.7 PSF
POSITIVE PRESSURE: ZONE 4&5	20.8 PSF	18.6 PSF	17.7 PSF

NOTES:

1. POSITIVE PRESSURE AND NEGATIVE PRESSURE SIGNIFY PRESSURES ACTING TOWARD AND AWAY FROM THE SURFACES, RESPECTFULLY.
2. EACH COMPONENT SHALL BE DESIGNED FOR MAXIMUM POSITIVE AND NEGATIVE PRESSURES.

EXISTING DRAWINGS:

1. EXISTING DRAWINGS WERE NOT AVAILABLE AT TIME OF DESIGN. ALL EXISTING CONDITIONS SHALL BE FIELD VERIFIED BY THE CONTRACTOR PRIOR TO START OF CONSTRUCTION.

EXISTING STRUCTURE:

1. EXISTING STRUCTURAL DIMENSIONS AND MEMBER SIZES ARE FOR REFERENCE ONLY. CONTRACTOR SHALL VERIFY ALL DIMENSIONS IN THE FIELD PRIOR TO FABRICATION. THE CONTRACTOR SHALL VERIFY THE ACTUAL CONFIGURATION OF EXISTING CONSTRUCTION AND THE CONDITION OF THE STRUCTURE BEFORE BEGINNING WORK. ANY DISCREPANCIES OR UNSOUND CONDITIONS SHALL BE REPORTED TO THE ARCHITECT FOR RESOLUTION BEFORE BEGINNING WORK. REFER TO ARCHITECTURAL PLANS FOR DIMENSIONS, EMBEDMENTS, AND OPENINGS NOT SHOWN. REFER TO MECHANICAL AND ELECTRICAL PLANS FOR DUCTS, PIPING, EMBEDMENTS, AND OPENINGS NOT SHOWN.
2. TEMPORARY SHORING AND BRACING MAY BE NECESSARY IN ORDER TO PERFORM THE NECESSARY STRUCTURAL MODIFICATIONS TO THE EXISTING STRUCTURE SHOWN ON THE STRUCTURAL AND ARCHITECTURAL PLANS AND DETAILS. THE CONTRACTOR MUST RETAIN A LICENSED STRUCTURAL ENGINEER WHO SHALL INVESTIGATE WHERE THIS TEMPORARY SHORING/BRACING IS REQUIRED, AND SHALL DESIGN THIS TEMPORARY SHORING/BRACING.

FOUNDATIONS:

1. GEOTECHNICAL CONSULTANT: ACS SERVICES LLC
2. REPORT NUMBER: 1901078
3. REPORT DATE:FEBRUARY 11, 2019
4. SPREAD FOOTINGS SHALL BEAR ON COMPACTED FILL. FOR FILL REQUIREMENTS, SEE SOIL REPORT. DESIGN SOIL BEARING VALUE 1,500 PSF. BOTTOM OF FOOTINGS TO BE 2'-0" MINIMUM BELOW FINISHED GRADE. FINISHED GRADE IS DEFINED AS TOP OF SLAB FOR INTERIOR FOOTINGS AND LOWEST ADJACENT FINISHED GRADE WITHIN 5 FEET FOR PERIMETER FOOTINGS. FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF CONCRETE.
5. SPREAD FOOTINGS SHALL BEAR ON COMPACTED NATIVE SOILS. ASSUMED DESIGN SOIL BEARING VALUE 1,500 PSF AND LATERAL BEARING VALUE OF 150 PSF/FT PER IBC TABLE 1806.2 "PRESUMPTIVE LOAD-BEARING VALUES" WITH ASSUMED SAND, SILTY SAND, CLAYEY SAND, ETC. IF ACTUAL SOIL CONDITIONS DIFFER NOTIFY THE STRUCTURAL ENGINEER PRIOR TO PROCEEDING WITH WORK. BOTTOM OF FOOTINGS SHALL BEAR AT A DEPTH NOT LESS THAN 1.5 FT BELOW LOWEST ADJACENT GRADE WITHIN 5 FEET OF STRUCTURE OR FOUNDATION. FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY GEOTECHNICAL ENGINEER PRIOR TO PLACEMENT OF CONCRETE.

GENERAL:

1. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES.
2. THE CONTRACTOR IS RESPONSIBLE FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK THAT CONFORMS TO THE REGULATIONS OF THE OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA) SAFETY AND HEALTH STANDARDS FOR THE CONSTRUCTION INDUSTRY.
3. CONSTRUCTION MATERIAL SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOORS OR ROOF. LOAD SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT.
4. WHERE REFERENCE IS MADE TO VARIOUS TEST STANDARDS FOR MATERIALS, SUCH STANDARDS SHALL BE THE LATEST EDITION AND/OR ADDENDUM.
5. ESTABLISH AND VERIFY ALL OPENINGS AND INSERTS FOR ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING WITH APPROPRIATE TRADES, DRAWINGS AND SUBCONTRACTORS PRIOR TO CONSTRUCTION. DO NOT PENETRATE ANY STRUCTURAL ELEMENTS (BEAMS, COLUMNS, WALLS, SLABS, STEEL DECKS, ETC.) WITHOUT PRIOR WRITTEN APPROVAL OF STRUCTURAL ENGINEER THROUGH ARCHITECT OR OWNER.
6. OPTIONS ARE FOR CONTRACTOR'S CONVENIENCE. HE SHALL BE RESPONSIBLE FOR ALL CHANGES NECESSARY IF HE CHOOSES AN OPTION AND HE SHALL COORDINATE ALL DETAILS.
7. NOTES AND DETAILS ON DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL STRUCTURAL NOTES AND TYPICAL DETAILS. WHERE NO SPECIFIC DETAILS ARE SHOWN, CONSTRUCTION SHALL CONFORM TO SIMILAR WORK ON THE PROJECT.
8. TYPICAL DETAILS ARE NOT CUT ON DRAWINGS, BUT APPLY UNLESS NOTED OTHERWISE.
9. WHERE ANY DISCREPANCIES OCCUR BETWEEN PLANS, DETAILS, GENERAL STRUCTURAL NOTES AND SPECIFICATIONS, THE GREATER REQUIREMENTS SHALL GOVERN.
10. ANY ENGINEERING DESIGN PROVIDED BY OTHERS AND SUBMITTED FOR REVIEW SHALL BEAR THE SEAL OF A CIVIL OR STRUCTURAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED. SUPPLIER OF ENGINEERED STRUCTURAL COMPONENTS SHALL BE RESPONSIBLE FOR COMPLETE DESIGN AND SHALL USE ALL CONTRACT DOCUMENTS FROM ALL DISCIPLINES.
11. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFICATION OF ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND FINISHED GRADE WITH CIVIL DRAWINGS PRIOR TO START OF CONSTRUCTION. ALL DIMENSIONS SHOWN ON STRUCTURAL DRAWINGS ARE TO ASSIST CONTRACTOR IN VERIFICATION. DO NOT SCALE DIMENSIONS FROM DRAWINGS.
12. ITEMS SHOWN BY OTHER DISCIPLINES WITH REFERENCE TO STRUCTURAL DRAWINGS BUT NOT SHOWN ON THESE STRUCTURAL DRAWINGS SHALL BE CONSIDERED DESIGN BUILD ITEMS. CONTRACTOR SHALL SUBMIT DESIGN BY OTHERS FOR REVIEW

SHORING:

1. THE CONTRACTOR IS RESPONSIBLE FOR THE STRENGTH AND STABILITY OF THE STRUCTURE DURING CONSTRUCTION AND SHALL PROVIDE TEMPORARY SHORING, BRACING, AND OTHER ELEMENTS REQUIRED TO MAINTAIN STABILITY UNTIL THE STRUCTURES IS COMPLETE. SHORING DESIGN SHALL BE PROVIDED BY A CIVIL OR STRUCTURAL ENGINEER REGISTERED IN THE STATE IN WHICH THE PROJECT IS LOCATED. IT IS THE CONTRACTOR'S RESPONSIBILITY TO BE FAMILIAR WITH THE WORK REQUIRED IN THE CONSTRUCTION DOCUMENTS AND THE REQUIREMENTS FOR EXECUTING IT PROPERLY.

DEMOLITION:

1. DEMOLITION OF EXISTING STRUCTURE TO BE REMOVED SHALL BE PERFORMED BY THE CONTRACTOR USING MEANS NECESSARY TO PREVENT DAMAGE TO THE EXISTING STRUCTURE TO REMAIN. DAMAGE TO THE EXISTING STRUCTURE TO REMAIN SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE USING METHODS REVIEWED BY THE STRUCTURAL ENGINEER. IF EXISTING CONDITIONS DIFFER FROM THOSE SHOWN IN THE CONTRACT DOCUMENTS, CONTACT STRUCTURAL ENGINEER THROUGH ARCHITECT PRIOR TO PROCEEDING WITH WORK.

DEFERRED SUBMITTALS:

1. DEFERRED SUBMITTALS ARE THOSE PORTIONS OF THE DESIGN WHICH ARE NOT SUBMITTED AT THE TIME OF PERMIT APPLICATION AND WHICH ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITHIN A SPECIFIED PERIOD.
2. SUBMITTAL DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD THROUGH THE ARCHITECT AND GENERAL CONTRACTOR WITHIN 6 WEEKS OF AWARD OF CONTRACT TO THE GENERAL CONTRACTOR. ONCE THE SUBMITTAL DOCUMENTS HAVE BEEN FOUND TO BE IN GENERAL CONFORMANCE TO THE CONTRACT DOCUMENTS, THE ENGINEER OF RECORD WILL FORWARD THEM TO THE ARCHITECT WITH A NOTATION INDICATING THAT THEY ARE IN GENERAL CONFORMANCE WITH THE DESIGN OF THE BUILDING. THE ARCHITECT WILL FORWARD THE DEFERRED SUBMITTAL DOCUMENTS TO THE GENERAL CONTRACTOR WHO WILL MAINTAIN ONE SET ON SITE FOR REFERENCE BY THE CITY INSPECTOR. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THE SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.
3. ITEMS THAT ARE SUBMITTED FOR CONSIDERATION AS DEFERRED SUBMITTALS ARE AS FOLLOWS:

- PREFABRICATED OPEN WEB (TJL TYP) WOOD TRUSSES
- CURTAIN WALL SYSTEM

SHOP DRAWINGS:

1. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS AND ITEMS REQUIRED BY ARCHITECTURAL SPECIFICATIONS. UNITED STRUCTURAL DESIGN, LLC. ASSUMES NO RESPONSIBILITY FOR THE FAILURE OF THE CONTRACTOR TO SUBMIT SHOP DRAWINGS FOR REVIEW.
2. ITEMS NOT IN ACCORDANCE WITH CONTRACT DOCUMENTS SHALL BE FLAGGED UPON CONTRACTORS REVIEW
3. THE CONSTRUCTION DOCUMENTS MAY NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS.
4. ELECTRONIC FILES OF CONSTRUCTION DOCUMENTS WILL NOT BE MADE AVAILABLE FOR USE AS SHOP DRAWINGS.
5. VERIFY ALL DIMENSIONS AND FINISHED GRADE WITH ARCHITECTURAL DRAWINGS, CIVIL DRAWINGS, AND FIELD CONDITIONS.
6. THE ENGINEER OF RECORD HAS THE RIGHT TO APPROVE OR DISAPPROVE ANY CHANGES TO CONTRACT DOCUMENTS AT ANYTIME BEFORE OR AFTER SHOP DRAWING REVIEW.
7. ITEMS OMITTED OR SHOWN INCORRECTLY AND ARE NOT FLAGGED BY THE STRUCTURAL ENGINEER OR ARCHITECT SHALL NOT BE CONSIDERED CHANGES TO THE CONTRACT DOCUMENTS.
8. SHOP DRAWINGS DO NOT REPLACE THE CONTRACT DOCUMENTS. REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP DRAWINGS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO ENSURE ALL ITEMS ARE CONSTRUCTED ACCORDING TO THE CONTRACT DOCUMENTS.

CONCRETE:

1. CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 301, "STANDARD SPECIFICATIONS FOR STRUCTURAL CONCRETE" AND ACI 318, "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE".
2. ADDITION OF WATER TO THE BATCH FOR MATERIAL WITH INSUFFICIENT SLUMP WILL NOT BE PERMITTED, UNLESS THE SUPPLIER HAS SPECIFICALLY WITHHELD WATER FROM THE BATCH AT THE PLANT. IN SUCH CASE THE MIX DESIGN AND TRUCK TICKET MUST CLEARLY STATE THE MAXIMUM AMOUNT OF WATER THAT CAN BE ADDED TO THE BATCH ON SITE. IN NO CASE SHALL THE DESIGN WATER TO CEMENTITIOUS MATERIAL RATIO BE EXCEEDED.
3. CONCRETE CONTAINING SUPERPLASTICIZING ADMIXTURE SHALL HAVE A SLUMP OF 4" +/- 1", TO BE FIELD VERIFIED, PRIOR TO ADDING ADMIXTURE, AND NOT EXCEEDING 8" AT PLACEMENT.
4. MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED, EXCEPT THAT SLABS ON GRADE NEED BE VIBRATED ONLY AROUND UNDER-FLOOR DUCTS, SLAB EDGES, REINFORCING, KEYS, ETC. MECHANICALLY VIBRATE ONLY THE TOP 5 FEET OF DRILLED PIER CONCRETE. REVIBRATE TOP OF DRILLED PIER 15 MINUTES AFTER PLACING CONCRETE.
5. UNLESS APPROVED OTHERWISE IN WRITING BY THE ARCHITECT, ALL CONCRETE SLABS ON GRADE SHALL BE BOUND BY CONSTRUCTION JOINTS, KEYED OR SAW CUT, SUCH THAT THE ENCLOSED AREA DOES NOT EXCEED 150 SQUARE FEET. KEYED CONSTRUCTION JOINTS NEED ONLY OCCUR AT EXPOSED EDGES DURING POURING. ALL OTHER JOINTS MAY BE SAW CUT. CONTRACTOR SHALL SUBMIT PROPOSED SAWCUT AND CONSTRUCTION JOINT LAYOUT FOR REVIEW PRIOR TO CONSTRUCTION. CAST CLOSURE POUR AROUND COLUMNS AFTER DEAD LOAD IS APPLIED.
6. TEST DATA FOR CONCRETE SUBMITTALS SHALL BE SUBMITTED FOR REVIEW PRIOR TO PLACEMENT OF CONCRETE. REFERENCE ACI 318 CHAPTER 5, TABLE R5.3 FOR SPECIFIC REQUIREMENTS.
7. CLOSURE POUR SHALL BE CAST AROUND COLUMNS AFTER FULL COLUMN DEAD LOAD HAS BEEN APPLIED.
8. IF PERMITTED BY ARCHITECTURAL SPECIFICATIONS, FLY ASH SHALL BE LIMITED TO 25% OF THE TOTAL CEMENTITIOUS MATERIALS BY WEIGHT. FLY ASH PER ASTM C618. FLY ASH SHALL NOT BE USED IN ARCHITECTURALLY EXPOSED CONCRETE OR IN SLABS WITH AN ACID OR BURNISHED FINISH.
9. CONCRETE TESTING SAMPLES SHALL BE CAST FOR EACH CLASS OF CONCRETE PLACED EACH DAY. ONE SAMPLE SHALL BE TAKEN EVERY 150 YD3. CONCRETE SAMPLING PER ASTM C31 AND TESTING OF SAMPLES PER ASTM C39.
10. VAPOR BARRIER IF REQUIRED BY ARCHITECTURAL SPECIFICATION OR SOILS REPORT SHALL CONSIST OF A MINIMUM 15 MIL MATERIAL LAPPED A MINIMUM OF 6 INCHES AND TAPED PER MANUFACTURER RECOMMENDATIONS. REFER TO SOILS REPORT FOR ADDITIONAL INFORMATION.
11. AT CONCRETE OVER PRECAST TEES OR STEEL DECK, ACTUAL CONCRETE VOLUMES MAY EXCEED THEORETICAL VOLUMES DUE TO CAMBER AND DEFLECTION. CONTRACTOR SHOULD MAKE ALLOWANCE FOR THIS IN THE BID. NO CLAIMS FOR ADDITIONAL CONCRETE VOLUMES WILL BE ALLOWED.
12. DRILLED PIER CONCRETE SHALL BE CHANNELED TO FREE FALL DOWN THE SHAFT WITHOUT STRIKING THE REINFORCING OR THE SIDES OF THE SHAFT. MAXIMUM HEIGHT OF FREE-FALL IS 10'-0".
13. CONCRETE PROPERTIES:

CONCRETE USE	MINIMUM 28 DAY COMPRESSIVE STRENGTH
UNLESS NOTED OTHERWISE	
ALL CONCRETE SHALL BE	3,000 PSI
SLABS ON GRADE	4,000 PSI
FOOTINGS AND STEM WALLS	3,000 PSI

DRYPACK/FLOWABLE GROUT:

1. THE SPACE BENEATH ALL BASEPLATES AND BEARING PLATES SHALL BE THOROUGHLY CLEANED BEFORE DRYPACKING OR GROUTING. DRYPACK/GROUT SOLID BENEATH ALL BASEPLATES AND BEARING PLATES (MINIMUM 95% BEARING). NO VOIDS ARE PERMISSIBLE. USE OF DRYPACK OR FLOWABLE GROUT IS AT THE CONTRACTORS OPTION UNLESS SPECIFICALLY NOTED ON THE PLANS OR DETAILS. DRYPACK/GROUT PER THE FOLLOWING:

- DRYPACK - PORTLAND CEMENT, ASTM C150, TYPE I; AND CLEAN, NATURAL SAND, ASTM C404, SIZE NO. 2. MIX AT RATIO OF 1 PART CEMENT TO 2 1/2 PARTS SAND, BY VOLUME, WITH MINIMUM WATER REQUIRED FOR PLACEMENT AND HYDRATION. MINIMUM COMPRESSIVE STRENGTH SHALL BE 3000 PSI AT 28 DAYS WHEN TESTED IN ACCORDANCE WITH ASTM C109.
- FLOWABLE GROUT - PREMIXED, NONMETALLIC, NONCORROSIVE, NONSTAINING GROUT CONTAINING SELECTED SILICA SANDS, PORTLAND CEMENT, SHRINKAGE COMPENSATING AGENTS, PLASTICIZING AND WATER-REDUCING AGENTS, COMPLYING WITH ASTM C1107, OF CONSISTENCY SUITABLE FOR APPLICATION, AND A 30-MINUTE WORKING TIME. MINIMUM COMPRESSIVE STRENGTH SHALL BE 5000 PSI AT 28 DAYS WHEN TESTED IN ACCORDANCE WITH ASTM C1107. GROUT MUST BE CURED WITH WATER OR AN ASTM C309 CURE.

MASONRY:

1. MASONRY WORK SHALL CONFORM TO ALL REQUIREMENTS OF ACI 530, "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES".
2. ALL UNITS SHALL BE LAID IN RUNNING BOND UNLESS NOTED OTHERWISE. VERTICAL ALIGNMENT OF CELLS SHALL MAINTAIN A CONTINUOUS CLEAR, UNOBSTRUCTED CELL NOT LESS THAN 3 INCHES SQUARE. MINIMUM DEPTH OF HORIZONTAL BOND BEAM CHANNEL BELOW TOP OF UNIT SHALL BE 1 1/2", AND CHANNEL SHALL BE 3" WIDE MINIMUM. ALL UNITS SHALL BE FREE OF DUST AND DIRT AT THE TIME OF LAYING.
3. MORTAR SHALL CONFORM TO ASTM C270 AND SHALL BE TYPE S WITH COMPRESSIVE STRENGTH = 1,800 PSI. MASONRY CEMENT AND EXTENDED LIFE MORTAR SHALL NOT BE USED.
4. GROUT SHALL CONFORM TO ASTM C-476. GROUT FOR WALLS CONSTRUCTED WITH HOLLOW CONCRETE MASONRY UNITS OR FOR TWO-WYTHE WALLS SHALL HAVE AN F'g = 2000 PSI. GROUT FOR WALLS CONSTRUCTED WITH HOLLOW BRICK MASONRY UNITS SHALL HAVE AN F'g = 3000 PSI.
5. VERTICAL REINFORCING (UNLESS NOTED OTHERWISE): PLACE #4 (6" WALLS), #5 (8" WALLS), #5 (12" WALL) BAR IN CENTER OF GROUT AT CENTER OF WALL, CONTINUOUS FULL HEIGHT OF WALL, WITH ONE BAR AT ALL CORNERS, INTERSECTIONS, WALL ENDS, BEAM BEARING, JAMBS AND EACH SIDE OF CONTROL JOINTS AND AT INTERVALS NOT TO EXCEED 48" O.C. TIE AT 8'-0" VERTICALLY, WITH SINGLE WIRE LOOP TIE BY A.A. PRODUCTS COMPANY. UNLESS NOTED OTHERWISE, LAP SPLICES SHALL BE PER LAP SPLICE SCHEDULE IN TYPICAL DETAILS. DOWEL ALL VERTICAL REINFORCING TO FOUNDATION WITH STANDARD 90 DEGREE HOOKED DOWELS TO MATCH VERTICAL REINFORCING.
6. HORIZONTAL REINFORCING (UNLESS NOTED OTHERWISE): PLACE (2) #4 (6" WALL), (2) #5 (8" WALL), (2) #5 (12" WALL) BARS IN MINIMUM 8" DEEP GROUTED CONTINUOUS BOND BEAM AT ROOF AND ELEVATED FLOOR LINES. PLACE #4 (6" WALL), #5 (8" WALL), #5 (12" WALL) BAR IN MINIMUM 8" DEEP GROUTED CONTINUOUS BOND BEAM AT TOP OF PARAPET OR TOP OF A FREE-STANDING WALL. PLACE THESE BARS CONTINUOUS THROUGH CONTROL JOINT. WRAP MASTIC TAPE FOR 1'-6" EACH SIDE OF CONTROL JOINT. PROVIDE BENT BARS, TO MATCH HORIZONTAL BOND BEAM REINFORCING, AT CORNERS AND WALL INTERSECTIONS TO MAINTAIN BOND BEAM CONTINUITY. UNLESS NOTED OTHERWISE, LAP SPLICES SHALL BE PER TYPICAL REINFORCING BAR SPLICE DETAIL. STAGGER ALTERNATE SPLICES A MINIMUM OF 4'-0". PROVIDE STANDARD WEIGHT (NO.9 GAGE WIRE) DUR-O-WALL OR DUR-O-WIRE LADDER TYPE JOINT REINFORCING AT 16" O.C. IN MASONRY WALLS. LAP JOINT REINFORCING 6" MINIMUM.
7. GROUT SHALL BE CONSOLIDATED BY MECHANICAL VIBRATION DURING PLACING AND RECONSOLIDATED AFTER EXCESS MOISTURE HAS BEEN ABSORBED, BUT BEFORE WORKABILITY IS LOST.
8. PROVIDE CLEANOUTS IF GROUT POUR EXCEEDS 5'-0" IN HEIGHT. IF CLEANOUTS ARE PROVIDED, GROUT POUR MAXIMUM HEIGHT = 12'-0", IN LIFTS NOT TO EXCEED 6'-0".
9. UNLESS NOTED OTHERWISE ON THE PLANS, PLACE CONTROL JOINTS IN MASONRY WALLS SUCH THAT NO STRAIGHT RUN OF WALL EXCEEDS 24'-0". CONTROL JOINTS SHALL NOT OCCUR WITHIN 24" OF WALL CORNERS, INTERSECTIONS, ENDS OVER OPENINGS, OR WITHIN 24" OF JAMBS OR CONCENTRATED LOADS. CONTRACTOR SHALL PROVIDE MCJ LAYOUT TO ARCHITECT AND ENGINEER OF RECORD FOR REVIEW PRIOR TO START OF CONSTRUCTION.
10. GROUT ALL CELLS CONTAINING REINFORCING AND ALL MASONRY BELOW GRADE.
11. MASONRY UNIT PROPERTIES:

a. HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C90 WITH A COMPRESSIVE STRENGTH OF 2,000 PSI AND A DENSITY BETWEEN 105 PCF AND 125 PCF (MEDIUM WEIGHT). F'm FOR DESIGN IS 2000 PSI.

STEEL REINFORCING:

1. ALL BARS PER CRSI SPECIFICATIONS AND HANDBOOK. LATEST ACI CODE AND DETAILING MANUAL APPLY. SECURELY TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE. REINFORCING BAR SPACINGS GIVEN ARE MAXIMUM ON CENTERS.
2. ALL REINFORCING TO BE WELDED SHALL BE WELDED IN ACCORDANCE WITH AWS D1.4. NO TACK WELDING OF REINFORCING BARS IS ALLOWED WITHOUT PRIOR REVIEW OF PROCEDURE BY STRUCTURAL ENGINEER.
3. REINFORCING LAP SPLICES IN CONCRETE SHALL BE PER TYPICAL DETAIL UNLESS NOTED OTHERWISE. ALL SPLICE LOCATIONS ARE SUBJECT TO APPROVAL. PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZONTAL BARS AT CORNERS AND INTERSECTIONS OF FOOTINGS AND WALLS.
4. LAP IN WELDED WIRE FABRIC SHALL BE MADE SO THAT THE OVERLAP MEASURED BETWEEN THE OUTERMOST CROSS WIRES OF EACH FABRIC SHEET IS NOT LESS THAN THE SPACING OF CROSS WIRES PLUS 2 INCHES.
5. TYPICAL REINFORCING BAR STRENGTHS
6. REINFORCING (NON-WELDABLE): ASTM A615, DEFORMED, Fy = 60 KSI
7. REINFORCING (WELDABLE): ASTM A706, DEFORMED, Fy = 60 KSI
8. WELDED WIRE FABRIC: ASTM A185, WIRE PER ASTM A82

6. TYPICAL CLEAR CONCRETE COVERAGES
- CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: 3"
- FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:
  #6 AND LARGER: 2"
  #5 AND SMALLER: 1 1/2"
- FORMED CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
  SLABS, WALLS OR JOISTS:
  #14 AND LARGER: 1 1/2"
  #11 AND SMALLER: 3/4"
- BEAMS, COLUMNS (TO PRIMARY REINF., TIES OR STIRRUPS): 1 1/2"

ALL OTHERS PER LATEST EDITION OF ACI 318.

SHEET LIST	
SHEET NUMBER	SHEET NAME
S0.1	GENERAL STRUCTURAL NOTES
S0.2	GSN, CONT & SPECIAL INSPECTIONS
S0.3	SPECIAL INSPECT./ SCHEDULE SHEET
S1.1	TYPICAL DETAILS
S1.2	TYPICAL DETAILS
S1.3	TYPICAL DETAILS
S1.4	TYPICAL DETAILS
S1.5	TYPICAL DETAILS
S2.1	FOUNDATION PLAN
S3.1	FRAMING PLAN
S4.1	FOUNDATION DETAILS
S4.2	FOUNDATION DETAILS
S5.1	FRAMING DETAILS
S5.2	FRAMING DETAILS

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STRUCTURAL STEEL:

1. LATEST AISC AND AWS CODES APPLY. THE WORD APPROVED INSPECTION 4.4 OF THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES IS REDEFINED AS REVIEWED.
2. STEEL SHALL BE FINISHED AT LOCATIONS EXPOSED TO WEATHER WITH A CORROSION RESISTANT COATING APPLICABLE TO WEATHER AND EXPOSURE CONDITIONS OF PROJECT LOCATION.
3. WHEN STRUCTURAL STEEL IS FURNISHED TO A SPECIFIED MINIMUM YIELD POINT GREATER THAN 36 KSI, THE ASTM OR OTHER SPECIFICATION DESIGNATION SHALL BE INCLUDED NEAR THE ERECTION MARK ON EACH SHIPPING ASSEMBLY OR IMPORTANT CONSTRUCTION COMPONENT OVER ANY SHOP COAT OF PAINT PRIOR TO SHIPMENT FROM THE FABRICATORS PLANT.
4. IF IT IS NECESSARY TO SPLICE ANY MEMBER, SPLICE LOCATIONS ARE SUBJECT TO REVIEW BY STRUCTURAL ENGINEER. SPLICES SHALL BE FULL PENETRATION WELDED AND TESTED PER THIS SECTION. INDICATE ALL SPLICE LOCATIONS, AND WELDING PROCEDURES ON SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION.
5. ALL BEAMS SHALL BE ERECTED WITH THE NATURAL CAMBER UPWARDS.
6. ALL BOLTS SHALL BE INSTALLED WITH STEEL WASHERS.
7. ALL REFERENCE TO HEADED STUDS SHALL INDICATE AUTOMATIC WELDED HIGH STRENGTH HEADED STUDS (NELSON OR EQUIVALENT). SHEAR CONNECTORS SHALL BE NELSON TYPE S3L OR EQUIVALENT AND SHALL BE MANUFACTURED FROM COLD DRAWN STEEL CONFORMING TO ASTM A 108. STUDS SHALL CONFORM TO ALL REQUIREMENTS OF THE LATEST EDITION OF THE AWS C5.4 "RECOMMENDED PRACTICES FOR STUD WELDING" AND THE AWS D1.1 "STRUCTURAL WELDING CODE" PUBLISHED BY THE AMERICAN WELDING SOCIETY. CONFORMANCE SHALL INCLUDE, BUT NOT BE LIMITED TO, ALL QUALITY CONTROL TESTING PROVISIONS OF THE AFOREMENTIONED PUBLICATIONS.
8. HEADED SHEAR CONNECTOR STUDS ON COMPOSITE STEEL BEAMS SHALL BE 3/4" DIAMETER TYPICAL UNLESS NOTED OTHERWISE AND UNIFORMLY SPACED. FOR LENGTH AND SPACING REQUIREMENTS, SEE TYPICAL DETAIL. USE NOT MORE THAN ONE STUD PER RIB WHERE THE NUMBER OF STUDS REQUIRED IS LESS THAN OR EQUAL TO THE NUMBER OF RIBS AVAILABLE. WHERE THE NUMBER OF STUDS REQUIRED EXCEEDS THE NUMBER OF RIBS AVAILABLE, PLACE A MINIMUM OF ONE STUD PER RIB FULL LENGTH OF THE BEAM. PLACE ADDED STUDS (NO MORE THAN TWO PER RIB TOTAL) IN EACH RIB BEGINNING AT THE SUPPORTS AT EACH END AND MOVING TOWARDS MIDSPAN UNTIL REQUIRED NUMBER OF STUDS ARE SUPPLIED. MAXIMUM LONGITUDINAL STUD SPACING IS 32" CENTER TO CENTER. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO INSTALLATION. HEADED STUD LENGTHS AS SPECIFIED SHALL BE INTERPRETED AS THE FINISHED LENGTH AFTER INSTALLATION.
9. ALL WELDING BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES, CERTIFICATES SHALL BE THOSE ISSUED BY AN INDEPENDENT TESTING AGENCY.
10. ALL WELDING DONE BY E70 SERIES LOW HYDROGEN RODS. USE E90 SERIES FOR ASTM A706 REINFORCING BARS. USE E308 SERIES FOR STAINLESS TO STAINLESS WELDS AND E309 SERIES FOR STAINLESS TO CARBON STEELS.
11. ALL WELDING PER AMERICAN WELDING SOCIETY STANDARDS. ALL WELDS ON DRAWINGS ARE SHOWN AS SHOP WELDS. CONTRACTOR MAY SHOP WELD OR FIELD WELD AT THEIR DISCRETION. SHOP WELDS OR FIELD WELDS SHALL BE SHOWN ON SHOP DRAWINGS.
12. SLAG SHALL BE REMOVED FROM ALL COMPLETED WELDS, AND THE WELD AND ADJACENT BASE METAL SHALL BE CLEANED BY BRUSHING OR OTHER SUITABLE MEANS. WELDED JOINTS SHALL NOT BE PAINTED UNTIL AFTER WELDING HAS BEEN COMPLETED AND THE WELD ACCEPTED.
13. ALL COMPLETE PENETRATION WELDS SHALL BE TESTED.
14. STEEL FABRICATOR TO COORDINATE ALL BRACING, PLATES, ERECTION BOLTS, ETC. WITH STEEL JOIST MANUFACTURER AND STEEL ERECTOR.
15. ALL STRUCTURAL STEEL SHALL BE FABRICATED BY A FABRICATOR WITH ANY ONE OF THE FOLLOWING MINIMUM QUALIFICATIONS. QUALIFICATIONS SHALL BE IN EFFECT AT TIME OF BID.

- INTERNATIONAL ACCREDITATION SERVICE, INC. (IAS) APPROVED FABRICATOR.
- AISC CERTIFIED FABRICATOR (STD).

16. STEEL PROPERTIES

- WIDE FLANGE COLUMNS, BEAMS AND TEES: ASTM A992 (Fy = 50 KSI)
- CHANNELS, PLATES AND ANGLES: ASTM A36 (Fy = 36 KSI)
- PIPE STEEL: ASTM A53 Gr. B (Fy = 35 KSI)
- HSS RECTANGULAR STEEL: ASTM A500 Gr. B (Fy = 46 KSI)
- BOLTS: ASTM A325 OR ASTM A F1852 TWIST-OFF TYPE
- ANCHOR RODS: ASTM F1554 Gr. 36 (Fy = 36 KSI)

17. STEEL BOLTS SHALL BE PRETENSIONED UNLESS OTHERWISE NOTED AS A SNUG-TIGHT CONNECTION ON THE DRAWINGS OR DETAILS.
ONE
OF HE FOLLOWING METHODS SHALL BE USED TO ASSURE
ADEQUATE
PRETENSIONING IS ACHIEVED:
- TURN-OF-NUT METHOD
- DIRECT TENSION INDICATOR WASHERS
- CALIBRATED WRENCH
- TWIST-OFF TYPE BOLT

SELF CERTIFIED BY: DATE: 03/06/2019
DONALD ANDREWS CERTIFICATE #45

- PLANS WERE PREPARED BY OR UNDER THE DIRECT SUPERVISION OF, OR REVIEWED BY THE SELF-CERTIFIED PROFESSIONAL,
- PLANS ARE COMPLETE,
- THE PLANS ARE, AS OF THE DATE OF SUBMISSION, IN ACCORDANCE WITH THE REQUIREMENTS OF THE PHOENIX BUILDING CONSTRUCTION CODE AND ALL OTHER APPLICABLE LAWS.

ERWIN | ARCHITECTURE DEVELOPMENT

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SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE

Owner JONATHAN PITT
Proj. Name WANDERIST OFFICE & RETAIL

GENERAL STRUCTURAL NOTES

Date 03/06/2019

S0.1

Scale 1/4" = 1'-0"

3/7/2019 8:55:12 AM

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

WOOD:

1. DO NOT NOTCH OR DRILL JOISTS, BEAMS OR LOAD BEARING STUDS WITHOUT PRIOR APPROVAL OF STRUCTURAL ENGINEER THRU THE ARCHITECT.
2. WOOD CONNECTORS SHALL BE AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY OR OTHER MANUFACTURER WITH CURRENT AND EQUIVALENT I.C.C. APPROVAL. WHERE "TYPE" OF CONNECTOR IS INDICATED ON THE DRAWINGS/DETAILS, THE CONNECTOR AND ATTACHMENT SHALL BE PER THE MAXIMUM MODEL NUMBER BASED ON THE SIZE OF THE MEMBERS CONNECTED. ALL NAIL HOLES IN JOIST HANGERS AND MISCELLANEOUS FRAMING ANCHORS SHALL BE FILLED WITH NAILS PER MANUFACTURERS PUBLISHED NAIL SIZES. ALL BOLTED OR NAILED STRAP/SPLICE CONNECTIONS SHALL HAVE AN EQUAL NUMBER OF BOLTS OR NAILS EACH SIDE OF THE SPLICE JOINT. THE FIRST BOLT OR NAIL FROM EACH SIDE OF THE SPLICED OR STRAPPED MEMBER SHALL BE EQUIDISTANT FROM THE SPLICE.
3. ALL BEAMS AND JOISTS SHALL HAVE FULL UNIFORM BEARING AT SUPPORTS, BEAM SEATS AND COLUMN CAPS.
4. ALL NAILING NOT NOTED SHALL BE ACCORDING TO IBC TABLE 2304.9.1.
5. IN WOOD STUD WALLS, UNLESS NOTED OTHERWISE, DOUBLE UP STUDS AT ALL JAMBS, CORNERS, INTERSECTIONS, AND AT ISOLATED BEARING POINTS OF FRAMING MEMBERS ABOVE. WOOD FRAME BEARING WALLS SHALL HAVE A SIMPSON CONNECTOR/ANCHOR TOP AND BOTTOM OF STUDS AT 32" O.C. MAXIMUM, EXCEPT WHERE PLYWOOD SHEATHING IS NAILED DIRECTLY TO THE TOP AND BOTTOM PLATES. PROVIDE 2X SOLID BLOCKING AT MID-HEIGHT OF BEARING STUD WALLS.
6. AT WOOD STUD WALLS, WOOD PLATE ANCHOR RODS SHALL BE 1/2" DIAMETER PLACED NOT TO EXCEED 4'-0" O.C. UNLESS NOTED OTHERWISE. ANCHOR RODS SHALL BE PLACED AT ALL JAMBS, CORNERS, INTERSECTIONS, AND WALL ENDS. ALL BOTTOM PLATES SHALL HAVE A MINIMUM OF 2 ANCHOR RODS. PROVIDE A MINIMUM OF .229"x 3"x 3" GALVANIZED STEEL PLATE WASHER UNDER EACH NUT AT FOUNDATION ANCHOR BOLTS OF SHEAR WALLS. THE PLATE WASHER SHALL EXTEND TO WITHIN 1/2" OF THE EDGE OF THE BOTTOM PLATE ON THE SIDES WITH SHEATHING. THE HOLE IN THE PLATE WASHER IS PERMITTED TO BE DIAGONALLY SLOTTED WITH A WIDTH OF UP TO 3/16" LARGER THAN THE BOLT DIAMETER AND A SLOT LENGTH NOT TO EXCEED 1-3/4", PROVIDE A STANDARD CUT WASHER BETWEEN THE PLATE WASHER AND NUT.
7. DOUBLE UP FLOOR JOISTS UNDER PARTITIONS. PROVIDE 1 X 3 OR METAL CROSS BRIDGING AT MIDSPAN OF ALL FLOOR JOISTS. PROVIDE 2" SOLID BLOCKING AT SUPPORT OF ALL JOISTS. PROVIDE BLOCKING UNDER ALL PARTITION WALLS PERPENDICULAR TO FLOOR JOISTS.
8. ALL MECHANICAL SUPPLY AND RETURN OPENINGS TO BE BETWEEN FRAMING U.N.O.
9. ALL WOOD PRODUCTS EXPOSED TO WEATHER SHALL BE TREATED PER THE PROJECT SPECIFICATIONS.

WOOD SHRINKAGE:

1. WOOD STUDS AND TRUSSES SHALL HAVE A MAXIMUM MOISTURE CONTENT OF 19% AT TIME OF DELIVERY TO JOB SITE, AFTER ROOF TRUSSES ARE ERECTED, AND PRIOR TO DRYWALL INSTALLATION.
2. ALL STRAPS AND HOLD-DOWN ANCHORS SHALL BE RETIGHTENED AND CHECKED FOR LOOSE CONNECTIONS.
3. MECHANICAL, PLUMBING, ELECTRICAL, AND DRYWALL SUBCONTRACTORS SHALL ACCOUNT FOR A MAXIMUM DIFFERENTIAL SHRINKAGE OF 1/6 INCH PER FLOOR IN ALL CONDUITS, DUCTS, AND CONNECTIONS.

PRESERVATIVE-TREATED WOOD:

1. ALL FOUNDATION PLATES OR SILLS AND SLEEPERS IN CONTACT WITH CONCRETE, AND WOOD FRAMING MEMBERS ATTACHED TO CONCRETE/MASONRY WALLS BELOW GRADE SHALL BE PRESERVATIVE-TREATED WOOD.
2. ALL WOOD FRAMING MEMBERS, INCLUDING WOOD SHEATHING THAT ARE LOCATED AT EXTERIOR WALLS THAT ARE LESS THAN 8 INCHES FROM FINISHED GRADE SHALL BE PRESERVATIVE-TREATED.
3. ALL FASTENERS INCLUDING NUTS AND WASHERS IN CONTACT WITH PRESERVATIVE-TREATED WOOD SHALL BE HOT-DIPPED GALVANIZED, ZINC-COATED GALVANIZED, OR STAINLESS STEEL. THE COATING WEIGHTS FOR ZINC-COATED FASTENERS SHALL BE PER ASTM 153. FASTENERS OTHER THAN NAILS, WOOD SCREWS, AND LAG SCREWS ARE PERMITTED TO BE OF MECHANICALLY DEPOSITED ZINC-COATED STEEL WITH COATING WEIGHTS PER ASTM B 695, CLASS 55 MINIMUM. CONTRACTOR SHALL COORDINATE WITH SUPPLIER TO PROVIDE ADEQUATE CORROSION RESISTANT METALS (NAILS, WASHERS, BOLTS, ETC.) BASED UPON THE CHEMICALS USED IN TREATED WOOD.

PREFABRICATED WOOD TRUSS MEMBERS:

1. PREFABRICATED OPEN WEB WOOD TRUSSES SHALL BE DESIGNED, FABRICATED, AND ERECTED IN ACCORDANCE WITH A CURRENT I.C.C. REPORT. FRAMING MEMBERS SHALL BE AGENCY STAMPED AND CONFORM TO THE GOVERNING CODE. FABRICATOR SHALL HAVE ISS APPROVAL OR BE APPROVED ACCORDING TO THE BUILDING JURISDICTION. MINIMUM WOOD TRUSS SIZES ARE AS INDICATED ON PLANS.
2. CONNECTIONS AND BEARING MATERIAL TO BE SHOP CONNECTED TO TRUSSES AND DESIGNED AND FURNISHED BY TRUSS FABRICATOR.
3. PREFABRICATED WOOD TRUSSES SHALL BE DESIGNED TO SUPPORT THEIR OWN WEIGHT PLUS SUPERIMPOSED DEAD AND LIVE LOADS STATED IN THE GENERAL NOTES. BRIDGING AND PERMANENT BRACING REQUIRED FOR TRUSSES ARE NOT SHOWN ON STRUCTURAL DRAWINGS. SUPPLY AND INSTALL ALL BRACING PER TRUSS MANUFACTURER'S REQUIREMENTS.
4. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS, CONNECTION DESIGN AND DETAILS, ERECTION DRAWINGS, AND SEALED CALCULATIONS FOR REVIEW PRIOR TO MANUFACTURE. CALCULATIONS AND SHOP DRAWINGS SHALL SHOW ANY SPECIAL DETAILS REQUIRED AT BEARING POINTS.
5. PRIOR TO ENCLOSING TRUSSES, CONTRACTOR SHALL GIVE NOTIFICATION TO MANUFACTURER REPRESENTATIVE TO PROVIDE AN OPPORTUNITY FOR REVIEW OF THE INSTALLATION. A MANUFACTURER'S INSTALLATION REVIEW LETTER SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD UPON COMPLETION.
6. ALL WOOD TRUSSES SHALL BE DESIGNED FOR AN ADDITIONAL 350 LB. POINT LOAD ANYWHERE ALONG THE SPAN.
7. ADDITIONAL WOOD TRUSSES TO BE SUPPLIED AS REQUIRED TO SUPPORT MECHANICAL EQUIPMENT. VERIFY SIZE, WEIGHT, AND LOCATION OF SUPPORTED EQUIPMENT WITH ARCHITECTURAL, MECHANICAL, PLUMBING, ELECTRICAL, AND SPRINKLER DRAWINGS.
8. MULTIPLE FRAMING MEMBERS SHALL BE FASTENED TOGETHER TO ALLOW TRANSFER OF SHEAR AND TENSION FORCES (MINIMUM 200 PLF) AT PLYWOOD SHEATHING JOINTS AND TO PREVENT CROSS GRAIN BENDING OF TOP CHORDS. ATTACHMENT SHALL BE A CONTINUOUS 20 GAGE METAL PLATE OR OTHER APPROVED MEANS. METHOD OF ATTACHMENT SHALL BE INDICATED ON SHOP DRAWINGS FOR REVIEW.
9. TOTAL LOAD DEFLECTIONS OF WOOD TRUSSES SHALL BE LIMITED TO SPAN/360 AT SIMPLE SPAN FLOOR MEMBERS. LIVE LOAD DEFLECTIONS OF WOOD TRUSSES SHALL BE LIMITED TO SPAN/480 AT SIMPLE SPAN FLOOR MEMBERS AND 2X SPAN/480 AT CANTILEVER FLOOR MEMBERS. TOTAL LOAD DEFLECTIONS OF WOOD TRUSSES SHALL BE LIMITED TO SPAN/240 AT SIMPLE SPAN ROOF MEMBERS AND 2X SPAN/240 AT CANTILEVER ROOF MEMBERS. LIVE LOAD DEFLECTIONS OF WOOD TRUSSES SHALL BE LIMITED TO SPAN/360 AT SIMPLE SPAN ROOF MEMBERS. FABRICATOR SHALL DESIGN MEMBERS FOR PONDING WHERE ROOF SLOPES ARE LESS THAN 1/4" PER FOOT. FRAMING MEMBERS SHALL BE CAMBERED FOR 1.0 TIMES THE DEAD LOAD DEFLECTION. MAXIMUM TOTAL LOAD DEFLECTION OF MEMBERS SHALL BE 1". FABRICATOR SHALL DESIGN ADJACENT MEMBERS FOR A MAXIMUM OF 1/4" DIFFERENTIAL DEFLECTION.
10. ALL CONNECTORS SHALL HAVE CURRENT I.C.C. APPROVAL AND SHALL BE DESIGNED AND SIZED FOR TWICE THE CALCULATED LOAD. NO OFF-SETS FOR CONNECTIONS WILL BE PERMITTED. ALL TOP AND BOTTOM CHORD MATERIAL SHALL BE FINGER JOINTED AT SPLICES AND TENSION TESTED TO A MINIMUM OF 1.2 TIMES THE ALLOWABLE TENSION PARALLEL TO THE GRAIN (PER NATIONAL DESIGN SPECIFICATIONS).
11. ALL PREFABRICATED WOOD TRUSSES SHALL BE CAMBERED FOR THE DESIGN DEAD LOAD.
12. PREFABRICATED WOOD TRUSSES ARE A DEFERRED SUBMITTAL ITEM.

PLYWOOD:

1. PLYWOOD SHALL BE APA "CDX" RATED SHEATHING OR BETTER, WITH AN EXTERIOR OR EXPOSURE 1 DURABILITY CLASSIFICATION AND SHALL BEAR THE STAMP OF AN APPROVED TESTING AGENCY. LAY UP FLOOR AND ROOF PLYWOOD WITH THE FACE GRAIN PERPENDICULAR TO SUPPORTS. STAGGER JOINTS. ON ROOFS WHERE PLYWOOD IS LAYED UP WITH FACE GRAIN PARALLEL TO SUPPORTS, USE A MINIMUM OF 5-PLY PLYWOOD.
2. MAXIMUM MOISTURE CONTENT AT TIME OF INSTALLATION TO BE LESS THAN 16%. PROVIDE PLY CLIPS AT MIDSPAN OF ALL UNSUPPORTED PLYWOOD EDGES. ALL NAILING SHALL BE COMMON NAILS. IF GUN NAILS ARE USED IN LIEU OF COMMON NAILS, REDUCE NAIL SPACING TO 4" AT EDGE NAILING AND 8" AT INTERMEDIATE NAILING. REFER TO TABLE BELOW FOR PLYWOOD PROPERTIES AND ATTACHMENT.
3. SCREWS AT FLOOR SHEATHING SHALL BE #8x2 1/2" LONG FOR SHEATHING LESS THAN 1" THICK. ALL FLOOR SHEATHING SHALL BE GLUED TO SUPPORTING MEMBERS WITH APA AFG-01 QUALIFIED GLUE.
4. NAILS AT FLOOR SHEATHING SHALL BE 0.148 DIA x 2 1/4" LONG SCREW SHANK NAILS FOR SHEATHING LESS THAN 1" THICK. ALL FLOOR SHEATHING SHALL BE GLUED TO SUPPORTING MEMBERS WITH APA AFG-01 QUALIFIED GLUE.
5. ATTACHMENT AT STEEL MEMBERS SHALL BE ITW RAMSET 1500k SERIES, 0.14" DIA. x1 1/2" LONG (3/4" PLYWOOD MAX), POWER-DRIVEN FASTENERS INSTALLED PER ICC ESR-1799, TABLE 4, OR APPROVED ICC EQUIVALENT. SPACING SHALL BE THE SAME AS NAIL SPACING IN SCHEDULE.
6. THE FIRST SHEET OF PLYWOOD SHEATHING ADJACENT AND PARALLEL TO WALLS, PERIMETER MEMBERS OR MEMBERS IDENTIFIED AS CHORD, COLLECTOR OR DRAG MEMBERS (ON ONE OR BOTH SIDES AS APPLICABLE) SHALL BE FULL WIDTH SHEETS. ELSEWHERE MINIMUM SHEET WIDTH 2'-0".
7. ALL SHEATHING SHALL BE GAPPED 1/8" ON THE EDGES AND ENDS. ROOF SHEATHING SHALL HAVE PANEL SHEATHING CLIPS APPROPRIATELY INSTALLED BETWEEN TRUSSES.
8. AT FLOOR PLYWOOD, BLOCK EDGES WITH 2x4 LAID FLAT WHERE NOTED ON THE PLANS AND DETAILS.
9. AT ROOF PLYWOOD, ALL UNSUPPORTED PLYWOOD EDGES TO BE BLOCKED WITH 2x4 LAID FLAT UNO ON THE PLANS.

PLYWOOD PROPERTIES AND ATTACHMENT			
	ROOF	FLOOR	SHEAR WALL (UNO)
THICKNESS	19/32"	23/32" T&G	15/32"
SPAN/INDEX RATIO	32/16	48/24	24/0
EDGE NAILING (COMMON NAILS)	10d (.148 DIA) AT 6" O.C.	10d RING SHANK AT 6" O.C.	8d (.134 DIA) AT 6" O.C.
INTERMEDIATE NAILING (COMMON NAILS)	10d (.148 DIA) AT 12" O.C.	10d RING SHANK AT 10" O.C.	8d (.134 DIA) AT 12" O.C.
MINIMUM NAIL PENETRATION (IN FRAMING)	1 5/8"	1 5/8"	1 1/2"

POST-INSTALLED ANCHORS:

1. POST-INSTALLED ANCHOR SYSTEMS SHALL COMPLY WITH THE LATEST REVISION OF ICC-ES ACCEPTANCE CRITERIA AND HAVE A VALID ICC-ES REPORT IN ACCORDANCE WITH THE APPLICABLE BUILDING CODE.
2. ANCHORS INSTALLED IN THE BOTTOM OF CONCRETE OVER STEEL DECK SHALL BE INSTALLED IN THE BOTTOM FLUTE ONLY.
3. ANCHORS ARE NOT TO BE INSTALLED UNTIL CONCRETE OR GROUT HAS REACHED ITS DESIGN STRENGTH.
4. FOR ANCHOR EMBEDMENT, SEE DRAWINGS OR TYPICAL DETAIL. USE EMBEDMENT RECOMMENDED BY MANUFACTURER WHERE NO EMBEDMENT IS SHOWN.
5. MANUFACTURER'S INSTALLATION TRAINING AND CERTIFICATION IS REQUIRED ON ALL POST-INSTALLED ANCHORS FOR ANCHOR INSTALLER.
6. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION TO SUPPORT SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI PER ACI 318. PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION.
7. ADHESIVE ANCHORS MUST BE INSTALLED IN CONCRETE AGED A MINIMUM OF 21 DAYS PER ACI 318.
8. EXPANSION BOLTS IN CONCRETE SHALL BE ONE OF THE FOLLOWING:
9. HILTI KWIK BOLT TZ CARBON AND STAINLESS STEEL ANCHORS (ICC-ES REPORT ESR-1917).
10. SIMPSON STRONG-TIE STRONG-BOLT 2 ANCHOR (ICC-ES REPORT ESR-3037).
11. DEWALT/POWERS POWER-STUD+SD2 CARBON AND STAINLESS STEEL ANCHOR (ICC-ES REPORT ESR-2502).
7. SCREW ANCHORS IN CONCRETE SHALL BE ONE OF THE FOLLOWING:
a. HILTI KWIK HUS-EZ CONCRETE SCREW ANCHOR (ICC-ES REPORT ESR- 3027.)
8. ADHESIVE ANCHORS IN CONCRETE SHALL BE ONE OF THE FOLLOWING:
a. HILTI HIT-HY 200 ADHESIVE ANCHOR (ICC-ES REPORT ESR-3187).
b. HILTI HIT-RE 500 V3 ADHESIVE ANCHOR (ICC-ES REPORT ESR-3814).
c. HILTI HIT-RE 100 ADHESIVE ANCHOR (ICC-ES REPORT ESR-3829).
d. SIMPSON STRONG-TIE SET-XP EPOXY ADHESIVE ANCHOR (ICC-ES REPORT ESR-2508).
9. ANCHORS IN CONCRETE OVER STEEL DECK SHALL BE ONE OF THE FOLLOWING:
a. HILTI KWIK BOLT TZ CARBON AND STAINLESS STEEL ANCHORS (ICC-ES REPORT ESR-1917).
b. HILTI KWIK HUS-EZ CONCRETE SCREW ANCHOR (ICC-ES REPORT ESR-3027).
c. SIMPSON STRONG-TIE STRONG-BOLT 2 WEDGE ANCHOR (ICC-ES REPORT ESR-3037).
d. SIMPSON STRONG-TIE TITEN HD SCREW ANCHOR (ICC-ES REPORT ESR-2713).
e. DEWALT/POWERS POWER-STUD+SD2 CARBON AND STAINLESS STEEL ANCHOR)ICC-ES REPORT ESR-2502).
10. EXPANSION BOLTS IN MASONRY SHALL BE ONE OF THE FOLLOWING:
a. HILTI KWIK BOLT 3 (ICC-ES REPORT ESR-1385).
b. HILTI KWIK BOLT TZ CARBON AND STAINLESS STEEL ANCHORS (ICC-ES REPORT ESR-3785).
c. SIMPSON STRONG-TIE WEDGE-ALL ANCHOR (ICC-ES REPORT ESR-1396).
11. ADHESIVE ANCHORS IN MASONRY SHALL BE ONE OF THE FOLLOWING:
a. HILTI HIT HY 70 ADHESIVE ANCHOR (ICC-ES REPORT ESR-2682).
b. SIMPSON STRONG-TIE SET XP ADHESIVE ANCHOR (IAPMO UES ER-265).
12. SCREW ANCHORS IN GROUT FILLED MASONRY SHALL BE ONE OF THE FOLLOWING:
a. HILTI KWIK HUS-EZ CONCRETE MASONRY SCREW ANCHOR (ICC-ES REPORT ESR-3056).
b. SIMPSON STRONG-TIE TITEN HD SCREW ANCHOR (ICC-ES REPORT ESR-1056).

SPECIAL STRUCTURAL INSPECTIONS:

PER IBC SECTION 1704 AND 1705 SPECIAL INSPECTIONS ARE IN ADDITION TO THE REQUIRED INSPECTION CONDUCTED BY THE BUILDING JURISDICTION PER IBC SECTION 110.
THE TYPES OF WORK LISTED BELOW SHALL BE INSPECTED BY A SPECIAL INSPECTOR.

1. ALL SPECIAL INSPECTORS SHALL BE UNDER THE SUPERVISION OF A REGISTERED CIVIL OR STRUCTURAL ENGINEER.
2. THE QUALIFICATIONS OF ALL SPECIAL INSPECTORS SHALL BE REVIEWED AND APPROVED BY THE STRUCTURAL ENGINEER OF RECORD.
3. THE MINIMUM QUALIFICATIONS FOR THE SPECIAL INSPECTORS ARE AS FOLLOWS:

- CONCRETE INSPECTION - I.C.C. CERTIFICATION IN REINFORCED CONCRETE OR E.I.T. CERTIFICATION.
- STRUCTURAL WELDING INSPECTION
- VISUAL TESTING - I.C.C. CERTIFICATION IN STRUCTURAL STEEL AND WELDING OR A.W.S. CERTIFIED WELD INSPECTOR (C.W.I.).
- NON-DESTRUCTIVE TESTING - A.W.S. C.W.I.

- HIGH STRENGTH BOLTING INSPECTION - I.C.C. CERTIFICATION IN STRUCTURAL STEEL AND WELDING.
- SPECIAL CASES - EXPERIENCE ACCEPTABLE TO THE STRUCTURAL ENGINEER OF RECORD.

4. DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:

- THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK REQUIRING SPECIAL INSPECTION FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATIONS.
- THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO BE KEPT AT THE SITE FOR USE BY THE BUILDING OFFICIAL, THE CONTRACTOR, THE STRUCTURAL ENGINEER OF RECORD, AND THE ARCHITECT OF RECORD. IF SPECIAL INSPECTION IS PROVIDED BY ANYONE OTHER THAN THE STRUCTURAL ENGINEER OF RECORD, INSPECTION REPORTS SHALL BE SUBMITTED TO THE OFFICE OF THE STRUCTURAL ENGINEER ON A WEEKLY BASIS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN IF UNCORRECTED, TO THE DESIGN AUTHORITY AND THE BUILDING OFFICIAL.
- UPON COMPLETION OF THE ASSIGNED WORK, THE SPECIAL INSPECTOR SHALL COMPLETE AND SIGN A FINAL REPORT CERTIFYING THAT TO THE BEST OF HIS KNOWLEDGE, THE WORK IS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.

5. DUTIES AND RESPONSIBILITIES OF THE CONTRACTOR:

- NOTIFY THE RESPONSIBLE INSPECTOR THAT WORK IS READY FOR INSPECTION AT LEAST ONE WORKING DAY (24 HOURS MINIMUM) BEFORE SUCH INSPECTION IS REQUIRED.
- ALL WORK REQUIRING SPECIAL STRUCTURAL INSPECTION SHALL REMAIN ACCESSIBLE AND EXPOSED UNTIL IT IS OBSERVED BY THE SPECIAL STRUCTURAL INSPECTOR.

6. SPECIAL INSPECTION

- INSPECTION OF FABRICATORS
- INSPECTION OF CONCRETE CONSTRUCTION
- INSPECTION OF MASONRY CONSTRUCTION
- INSPECTION OF STRUCTURAL STEEL
- INSPECTION OF STEEL OTHER THAN STRUCTURAL STEEL
- INSPECTION OF POST-INSTALLED ANCHORS
- INSPECTION OF SOILS

SEE TABLES ON GSN FOR ADDITIONAL INFORMATION.

MATERIAL	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	RESPONSIBLE FIRM
EARTHWORK	GRADING, EXCAVATION, AND FILL	X	-	TESTING LAB
	FILL MATERIAL	-	X	TESTING LAB
	SOIL COMPACTION	-	X	TESTING LAB
CAST-IN-PLACE CONCRETE	REINFORCING STEEL	-	X	UNITED
	USE OF REQUIRED CONCRETE DESIGN MIX	-	X	UNITED
	BOLTS INSTALLED IN CONCRETE (INCLUDING ADHESIVE AND EXPANSION ANCHORS)	X	-	UNITED
	CONCRETE PLACEMENT AND CURING TECHNIQUES	X	-	UNITED
	CONCRETE MATERIALS	-	X	TESTING LAB
UNIT MASONRY ASSEMBLIES	MORTAR, GROUT, UNIT MASONRY MATERIALS, AND MASONRY PRISMS	-	X	TESTING LAB
	SITE-MIXED MORTAR	-	X	UNITED
	PLACEMENT OF MASONRY UNITS AND CONSTRUCTION OF MORTAR JOINTS	-	X	UNITED
	REINFORCEMENT AND CONNECTORS	-	X	UNITED
	GROUT PLACEMENT	X	-	UNITED
	ADHESIVE AND EXPANSION ANCHORS	X	-	UNITED
STRUCTURAL STEEL AND STEEL DECK	STEEL FRAME FOR CONFORMANCE WITH CONSTRUCTION DOCUMENTS	-	X	UNITED
	FIELD WELDED CONNECTIONS	-	X	UNITED
	BOLTED CONNECTIONS	-	X	UNITED
	ULTRASONIC TESTING AND MOMENT CONNECTION FIT UP	X	-	TESTING LAB

2018 IBC, 1705.3 SPECIAL INSPECTION OF CONCRETE CONSTRUCTION

SPECIAL INSPECTION AND VERIFICATIONS FOR CONCRETE CONSTRUCTION SHALL BE AS REQUIRED BY TABLE 1705.3.

EXCEPTIONS: SPECIAL INSPECTIONS SHALL NOT BE REQUIRED FOR:
1. ISOLATED SPREAD CONCRETE FOOTINGS OF BUILDING THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK.
2. CONTINUOUS CONCRETE FOOTINGS SUPPORTING WALLS OF BUILDINGS THREE STORIES OR LESS ABOVE GRADE PLANE THAT ARE FULLY SUPPORTED ON EARTH OR ROCK WHERE:
3. THE FOOTINGS SUPPORT WALLS OF LIGHT-FRAME CONSTRUCTION;
4. THE STRUCTURAL DESIGN OF THE FOOTING IS BASED ON A SPECIFIED COMPRESSIVE STRENGTH, f'c, NO GREATER THAN 2,500 PSI REGARDLESS OF THE COMPRESSIVE STRENGTH SPECIFIED.
3. CONCRETE SLABS ON GRADE. STEEL REINFORCING STILL REQUIRES SPECIAL INSPECTION..

2018 IBC, TABLE 1705.3: REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION				
VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCE STANDARD	IBC REFERENCE
1. INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	-	X	ACI 318: Ch. 20, 25.2, 25.3, 26.5.1-26.5.3	1910.4
2. REINFORCING BAR WELDING. a. VERIFY WELDABILITY OF REINFORCING BARS. OTHER THAN ASTM A706 b. INSPECT SINGLE PASS FILLET WELDS, MAXIMUM 5/16". c. INSPECT ALL OTHER WELDS.	 - - X	 - X X	AWS D1.4 ACI 318: 26.5.4	—
3. INSPECT ANCHORS CAST IN CONCRETE.	-	X	ACI 318: 17.8.2	—
4. INSPECT ANCHORS POST-INSTALLED IN HARDENED CONCRETE MEMBERS. a. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS. b. MECHANICAL ANCHORS AND ADHESIVE ANCHORS NOT DEFINED IN 4.a.	 X -	 - X	 ACI 318: 17.8.2.4 ACI 318: 17.8.2	
5. VERIFYING USE OF REQUIRED DESIGN MIX.	-	X	ACI 318: Ch 19, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	-	ASTM C 172 ASTM C 31 ACI 318: 26.4, 26.12	1908.10
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	-	ACI 318: 26.4.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	X	ACI 318: 26.5.3-26.5.5	1908.9
9. INSPECT PRESTRESSED CONCRETE FOR: a. APPLICATION OF PRESTRESSING FORCES. b. GROUTING OF BONDED PRESTRESSING TENDONS.	 X X	 - -	ACI 318: 26.10	—
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	-	X	ACI 318: Ch 26.8	—
11. VERIFY OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	-	X	ACI 318: 26.11	—
12. INSPECT FORMWORK FOR SHAPE, LOCATION, AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	X	ACI 318: 26.11.1.2 (b)	—

1704.2.5 SPECIAL INSPECTION OF FABRICATORS:

SPECIAL INSPECTION OF FABRICATION OF STRUCTURAL STEEL BEING PERFORMED ON THE PREMISES OF A FABRICATOR'S SHOP IS REQUIRED.

EXCEPTION: SPECIAL INSPECTIONS OF FABRICATORS WITH ONE OF THE FOLLOWING QUALIFICATIONS IS NOT REQUIRED:
- INTERNATIONAL ACCREDITATION SERVICE, INC. (IAS)APPROVED FABRICATOR.
- AISC CERTIFIED FABRICATOR (STD).

THE SPECIAL INSPECTOR SHALL VERIFY THAT THE FABRICATOR MAINTAINS DETAILED FABRICATION AND QUALITY CONTROL PROCEDURES THAT PROVIDE A BASIS FOR INSPECTION CONTROL OF THE WORKMANSHIP AND THE FABRICATOR'S ABILITY TO CONFORM TO APPROVED CONSTRUCTION DOCUMENTS AND REFERENCED STANDARDS. THE SPECIAL INSPECTOR SHALL REVIEW THE PROCEDURES FOR COMPLETENESS AND ADEQUACY RELATIVE TO THE CODE REQUIREMENTS FOR THE FABRICATOR'S SCOPE OF WORK.

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2018 IBC, 1705.4 SPECIAL INSPECTION FOR MASONRY CONSTRUCTION

SPECIAL INSPECTION OF MASONRY CONSTRUCTION SHALL BE INSPECTED AND VERIFIED IN ACCORDANCE WITH TMS 402/ACI 530/ASCE 5 AND TMS 602/ACI 530.1/ ASCE 6 QUALITY ASSURANCE PROGRAM REQUIREMENTS.

LEVEL B QUALITY ASSURANCE PROGRAM PER TABLE 3.1.2. APPLIES

TMS 402/ACI 530 TABLE 3.1.2 - LEVEL B QUALITY ASSURANCE PROGRAM				
MINIMUM TESTS				
VERIFICATION OF SLUMP FLOW AND VISUAL STABILITY INDEX (VSI) AS DELIVERED TO THE PROJECT SITE IN ACCORDANCE WITH SPECIFICATION ARTICLE 1.5 B.1.b.3 FOR SELF CONSOLIDATING GROUT				
VERIFICATION OF f'm AND f'AAC IN ACCORDANCE WITH SPECIFICATION 1.4 B PRIOR TO CONSTRUCTION, EXCEPT WHERE SPECIFICALLY EXEMPTED BY CODE.				
MINIMUM INSPECTION				
VERIFICATION AND INSPECTION NEED TMS 402-16	CONTINUOUS	PERIODIC	REFERENCE TMS 402/ ACI 530/ ASCE 5	REFERENCE TMS 602/ ACI 530.1/ ASCE 6
1. VERIFY COMPLIANCE WITH APPROVED SUBMITTALS.	-	X		Art. 1.5
2. AS MASONRY CONSTRUCTION BEGINS, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
a. PROPORTIONS OF SITE-PREPARED MORTAR	-	X		Art. 2.1, 2.6 A
b. CONSTRUCTION OF MORTAR JOINTS	-	X		Art. 3.3 B
c. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES	-	X		Art. 2.4 B, 2.4 H
d. LOCATION OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES	-	X		Art. 3.4, 3.6 A
e. PRESTRESSING TECHNIQUE	-	X		Art. 3.6 B
f. PROPERTIES OF THIN-BED MORTAR FOR AAC MASONRY	X	X		Art. 2.1C
3. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:				
a. GROUT SPACE	-	X		Art. 3.2 D, 3.2F
b. GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOLTS, AND PRESTRESSING TENDONS AND ANCHORAGES.	-	X	Sec. 6.1	Art. 2.4, 3.4
c. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES.	-	X	Sec. 6.1, 6.2.1, 6.2.6, 6.2.7	Art. 3.2 E, 3.4, 3.6 A
d. PROPORTIONS OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS	-	X		Art. 2.6 B, 2.4 G.1.b
e. CONSTRUCTION OF MORTAR JOINTS	-	X		Art. 3.3 B
4. VERIFY DURING CONSTRUCTION:	-	X		
a. SIZE AND LOCATION OF STRUCTURAL ELEMENTS	-	X		Art. 3.3 F
b. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, OR OTHER CONSTRUCTION.	-	X	Sec. 1.2.1(e), 6.1.4.3, 6.2.1	
c. WELDING OF REINFORCEMENT.	X	-	Sec. 8.1.6.7.2, 9.3.3.4 (c), 11.3.3.4 (b)	
d. PREPARATION, CONSTRUCTION AND PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40 DEG F) OR HOT WEATHER (TEMPERATURE ABOVE 90 DEG F)	-	X		Art. 1.8 C, 1.8 D
e. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE.	X	-		Art. 3.6 B
f. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDED TENDONS IS IN COMPLIANCE	X	-		Art. 3.5, 3.6 C
g. PLACEMENT OF AAC MASONRY UNITS AND CONSTRUCTION OF THIN-BED MORTAR JOINTS	X	X		Art. 3.3 B.9, 3.3 F.1.b
5. OBSERVE PREPARATION OF GROUT SPECIMENS, MORTAR SPECIMENS, AND/OR PRISMS	-	X		Art. 1.4 B.2.a.3 1.4 B.2.b.3, 1.4 B.2.c.3, 1.4 B.3, 1.4 B.4

SELF CERTIFIED BY: DATE: 03/06/2019
DONALD ANDREWS CERTIFICATE #45

- PLANS WERE PREPARED BY OR UNDER THE DIRECT SUPERVISION OF, OR REVIEWED BY THE SELF-CERTIFIED PROFESSIONAL,
- PLANS ARE COMPLETE,
- THE PLANS ARE, AS OF THE DATE OF SUBMISSION, IN ACCORDANCE WITH THE REQUIREMENTS OF THE PHOENIX BUILDING CONSTRUCTION CODE AND ALL OTHER APPLICABLE LAWS.

ERWIN | ARCHITECTURE DEVELOPMENT

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SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE

Registered Professional Engineer (Structural)
CERTIFICATE NO. 61523
DAVID GRAPSAS
Date Signed 3/6/2019
EXPIRES: 03/31/2022

Owner JONATHAN PITT
Proj. Name WANDERIST OFFICE & RETAIL

GSN, CONT & SPECIAL INSPECTIONS

Date 03/06/2019

S0.2

Scale 1/4" = 1'-0"

3/7/2019 8:55:13 AM

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

SPECIAL CASES: POST-INSTALLED ANCHORS

SPECIAL INSPECTION FOR EXPANSION, SCREW, AND EPOXY/ADHESIVE ANCHORS ARE REQUIRED DURING THE PLACEMENT OF ALL POST-INSTALLED ANCHORS SHOWN ON STRUCTURAL DRAWINGS AND INCLUDE:
- VISUAL VERIFICATION OF HOLE DIAMETER, HOLE DEPTH, AND DRILL BIT CONFORMANCE.
- VISUAL VERIFICATION OF HOLE CLEANING PER SPECIFIED PRODUCT MANUFACTURER'S RECOMMENDATIONS.
- VISUAL VERIFICATION OF ANCHOR INSTALLATION ACCORDING TO SPECIFIED PRODUCT MANUFACTURER'S RECOMMENDATIONS. INSPECTION OF EXPANSION AND SCREW ANCHORS SHALL INCLUDE VERIFICATION OF THE TIGHTENING TORQUE REQUIRED PER SPECIFIED ANCHOR MANUFACTURER.

IBC, 1705.6 SPECIAL INSPECTION OF SOILS

SPECIAL INSPECTION FOR EXISTING SITE SOIL CONDITIONS, FILL PLACEMENT AND LOAD-BEARING REQUIREMENTS SHALL BE AS REQUIRED BY TABLE 1705.6.

IBC, TABLE 1705.6: REQUIRED VERIFICATION AND INSPECTION OF SOILS

VERIFICATION AND INSPECTION TASK	CONTINUOUS	PERIODIC
1. VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	X
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	-	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	X	-
5. PRIOR TO PLACEMENT OF COMPACTED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	-	X

1705.2.2 SPECIAL INSPECTION OF STRUCTURAL STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL

SPECIAL INSPECTION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH TABLE 1705.2.2

IBC, TABLE 1705.2.2 REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION OTHER THAN STRUCTURAL STEEL

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCE STANDARD
1. MATERIAL VERIFICATION OF COLD-FORMED STEEL			
a. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS	-	X	APPLICABLE ASTM MATERIAL STANDARD
b. MANUFACTURER'S CERTIFIED TEST REPORTS.	-	X	
2. INSPECTION OF WELDING:			
a. COLD-FORMED STEEL DECK:			
1) FLOOR AND ROOF DECK WELDS.	-	X	AWS D1.3
b. REINFORCING STEEL:			AWS D1.4 ACI 318: SECTION 3.5.2
1) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A706	-	X	
2) REINFORCING STEEL RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL STRUCTURAL WALLS OF CONCRETE AND SHEAR REINFORCING.	X	-	
3) SHEAR REINFORCEMENT.	X	-	
4) OTHER REINFORCING STEEL.	-	X	

1705.2 SPECIAL INSPECTION OF STRUCTURAL STEEL CONSTRUCTION

SPECIAL INSPECTION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360.

NONDESTRUCTIVE TESTING OF WELDED JOINTS (SEE DESIGN LOADS FOR RISK CATEGORY):
- FOR RISK CATEGORY III OR IV - UT SHALL BE PERFORMED ON ALL CJP GROOVE WELDS SUBJECT TO TRANSVERSELY APPLIED TENSION LOADING.
- FOR RISK CATEGORY II - UT SHALL BE PERFORMED ON 10% OF WELDS IN MATERIALS 5/16" OR THICKER. WHERE MATERIAL IS LESS THAN 5/16", NO UT IS REQUIRED.
- FOR RISK CATEGORY I - UT NOT REQUIRED.

O - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS

P - PERFORM THESE TASKS FOR EACH WELDED JOINT OR MEMBER

AISC 360 TABLE N5.4-1: INSPECTION TASKS PRIOR TO WELDING

WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	P
MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	P
MATERIAL IDENTIFICATION (TYPE/GRADE)	O
WELDER IDENTIFICATION SYSTEM*	O
FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY) • JOINT PREPARATION • DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL) • CLEANLINESS (CONDITION OF STEEL SURFACES) • TACKING (TACK WELD QUALITY AND LOCATION) • BACKING TYPE AND FIT (IF APPLICABLE)	O
CONFIGURATION AND FINISH OF ACCESS HOLES	O
FIT-UP OF FILLET WELDS • DIMENSIONS (ALIGNMENT, GAPS AT ROOT) • CLEANLINESS (CONDITION OF STEEL SURFACES) • TACKING (TACK WELD QUALITY AND LOCATION)	O

*THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM BY WHICH A WELDER WHO HAS WELDED A JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOW-STRESS TYPE.

AISC 360 TABLE N5.4-2: INSPECTION TASKS DURING WELDING

USE OF QUALIFIED WELDERS	O
CONTROL AND HANDLING OF WELDING CONSUMABLES • PACKAGING • EXPOSURE CONTROL	O
NO WELDING OVER CRACKED TACK WELDS	O
ENVIRONMENTAL CONDITIONS • WIND SPEED WITHIN LIMITS • PRECIPITATION AND TEMPERATURE	O
WPS FOLLOWED • SETTINGS ON WELDING EQUIPMENT • TRAVEL SPEED • SELECTED WELDING MATERIALS • SHIELDING GAS TYPE/FLOW RATE • PREHEAT APPLIED • INTERPASS TEMPERATURE MAINTAINED (MIN/MAX) • PROPER POSITION (F,V,H,OH)	O
WELDING TECHNIQUES • INTERPASS AND FINAL CLEANING • EACH PASS WITHIN PROFILE LIMITATIONS • EACH PASS MEETS QUALITY REQUIREMENTS	O

AISC 360 TABLE N5.4-3: INSPECTION TASKS AFTER WELDING

WELDS CLEANED	O
SIZE, LENGTH AND LOCATION OF WELDS	P
WELDS MEET VISUAL ACCEPTANCE CRITERIA • CRACK PROHIBITION • WELD/BASE-METAL FUSION • CRATER CROSS SECTION • WELD PROFILES • WELD SIZE • UNDERCUT • POROSITY	P
ARC STRIKES	P
K-AREA*	P
BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	P
REPAIR ACTIVITIES	P
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	P

*WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA, VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 INCHES OF THE WELD.

AISC 360 TABLE N5.6-1: INSPECTION TASKS PRIOR TO BOLTING

MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS.	P
FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS	O
PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE).	O
PROPER BOLTING PROCEDURE SELECTED FOR JOINT DETAIL	O
CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS.	O
PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED.	O
PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS	O

AISC 360 TABLE N5.6-2: INSPECTION TASKS DURING BOLTING

FASTENER ASSEMBLIES, OF SUITABLE CONDITION, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED	O
JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION.	O
FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING.	O
FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGE.	O

AISC 360 TABLE N5.6-3: INSPECTION TASKS AFTER BOLTING

DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	P

AISC 360 TABLE N6.1: INSPECTION OF STEEL ELEMENTS OF COMPOSITE CONSTRUCTION PRIOR TO CONCRETE PLACEMENT

PLACEMENT AND INSTALLATION OF STEEL DECK	P
PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	P
DOCUMENT ACCEPTANCE OR REJECTION OF STEEL ELEMENTS	P

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CONTINUOUS FOOTING (WF) SCHEDULE

NOTE:
IF FIELD DIMENSION OF FOOTING IS LARGER THAN SHOWN IN SCHEDULE, CONTRACTOR TO PLACE ADDITIONAL REINFORCING TO MAINTAIN ACI 318 MINIMUM AREA OF STEEL REQUIREMENTS.

MARK	WIDTH	DEPTH	REINFORCING		NOTES
			TRANSVERSE	CONTINUOUS	
WF1	1' - 8"	1' - 0"		(2) #5 TOP AND BOTTOM	

ISOLATED FOOTING (F) SCHEDULE

NOTE:
IF FIELD DIMENSION OF FOOTING IS LARGER THAN SHOWN IN SCHEDULE, CONTRACTOR TO PLACE ADDITIONAL REINFORCING TO MAINTAIN ACI 318 MINIMUM AREA OF STEEL REQUIREMENTS.

MARK	WIDTH	LENGTH	DEPTH	REINFORCING
F1	4' - 0"	4' - 0"	1' - 0"	(5) #5 EACH WAY, BOTTOM
F2	5' - 0"	5' - 0"	1' - 0"	(6) #5 EACH WAY, BOTTOM

STEEL COLUMN (C) SCHEDULE

MARK	SIZE	BASE PLATE AND ANCHORAGE
C1	HSS4X4X1/4	1/2"x11"x11" STEEL BASEPLATE WITH (4) 3/4" DIA. ANCHOR RODS WITH 7" EMBEDMENT

WOOD/STEEL STUD WALL (W) SCHEDULE

MARK	STUD SIZE	STUD SPACING	REMARKS
WS1	6"	16" O.C.	

SHEAR WALL SCHEDULE (*SW*)

NOTE:
- WHERE NAIL SPACING IS LESS THAN 6" ON CENTER ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS OR FRAMING SHALL BE 3" NOMINAL OR THICKER AND NAILS ON EACH SIDE SHALL BE STAGGERED TYP.

MARK	SHEATHING MATERIAL	NAILING	SHEATHING SIDES	BOTTOM PLATE TO CONCRETE	ALLOWABLE SHEAR
SW1	3/8" PLYWOOD (OR OSB APA RATED) (ALL EDGES BLOCKED)	8d COMMON NAILS AT 6" O.C. EDGE 12" O.C. FIELD	ONE SIDE	1/2" DIA x 7" LONG ANCHOR BOLTS WITH 1/4"x3"x3" GALVANIZED PLATE WASHERS AT 48" O.C.	372.5 PLF
SW2	3/8" PLYWOOD (OR OSB APA RATED) (ALL EDGES BLOCKED)	8d COMMON NAILS AT 6" O.C. EDGE 12" O.C. FIELD	ONE SIDE	1/2" DIA. X 5" LONG ADHESIVE ANCHOR BOLTS AT 48" O.C. WITH 1/4"x3"x3" GALVANIZED PLATE WASHERS.	372.5 PLF

HOLDOWN SCHEDULE (▲1)

SEE DETAILS XX&XX/SXXX FOR ADDITIONAL INFORMATION

SYMBOL	HOLDOWN	ANCHOR	EMBEDMENT	CAPACITY	POST AT HOLDOWN
▲1	SIMPSON HDU2-SDS2.5	SSTB16	12 5/8"	3.61K	(2) 2x6 STUDS MIN.
▲2	SIMPSON HDU2-SDS2.5	5/8" DIA. ADHESIVE ANCHOR BOLT	7"	2.66K	(2) 2x6 STUDS MIN.

SELF CERTIFIED BY: _____ DATE: 03/06/2019
DONALD ANDREWS CERTIFICATE #45

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- PLANS ARE COMPLETE,
- THE PLANS ARE, AS OF THE DATE OF SUBMISSION, IN ACCORDANCE WITH THE REQUIREMENTS OF THE PHOENIX BUILDING CONSTRUCTION CODE AND ALL OTHER APPLICABLE LAWS.

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SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE

Registered Professional Engineer (Structural) CERTIFICATE NO. 61523 DAVID GRAPSAS Date Signed: 3/6/2019 ARIZONA U.S.A.
EXPIRES: 03/31/2022

Owner JONATHAN PITT
Proj. Name WANDERIST OFFICE & RETAIL

SPECIAL INSPECT./ SCHEDULE SHEET

Date 03/06/2019

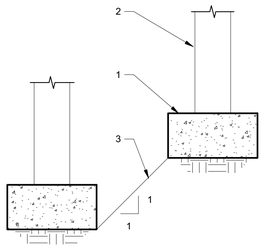
S0.3

Scale 1/4" = 1'-0"

3/7/2019 8:55:14 AM

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

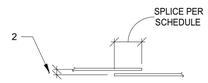
1. CONCRETE WALL FOOTING.
2. CONCRETE OR MASONRY WALL ABOVE.
3. MAXIMUM SLOPE BETWEEN BOTTOMS OF FOOTINGS SHALL BE 45 DEGREES. STEP FOOTINGS AS REQUIRED. SEE TYPICAL STEPPED FOOTING DETAIL.



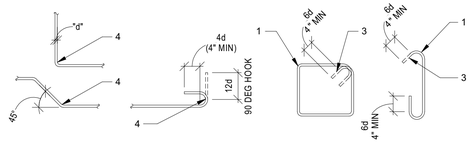
C1 TYPICAL MAXIMUM SLOPE BETWEEN ADJACENT FOOTINGS
NO SCALE 105-12

1. BEND AROUND 1 1/2" PIN FOR #3 BARS. BEND AROUND 2" PIN FOR #4 BARS. BEND AROUND 2 1/2" PIN FOR #5 BARS.
2. MAXIMUM 1/5 LAP LENGTH BUT NOT MORE THAN 6". 1" MIN.
3. 135 DEGREE BENDS
4. RADIUS
   - 3d FOR BARS NOT OVER #8
   - 4d FOR #9, #10, AND #11 BARS
   - 5d FOR #14 AND #18
   - RADIUS = 5d FOR ALL GRADE 40 BARS WITH 180 DEGREE HOOK

BAR OFFSET



SPLICE DETAIL



BEND AND HOOK DETAILS BEND AND HOOK DETAILS

C2 TYPICAL CONCRETE AND MASONRY REINFORCING BAR DETAILS
NO SCALE 103-02

1. CONCRETE WALL FOOTING.
2. CONCRETE OR MASONRY WALL ABOVE.
3. TOP OF WALL FOOTING.
4. WALL FOOTING LONGITUDINAL REINFORCING

B1 TYPICAL STEPPED FOOTING
NO SCALE 105-10

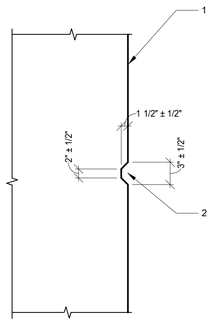
1. TOP BARS ARE ANY HORIZONTAL BARS PLACED SO THAT MORE THAN 12" OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE REINFORCING.
2. THIS TABLE IS BASED ON NORMAL WEIGHT CONCRETE.

TYPICAL CONCRETE LAP SPLICE TABLE

		CLASS B TENSION SPLICE LENGTH, INCHES					
CONCRETE STRENGTH		f'c >= 2,500 PSI / 3,000 PSI		f'c >= 4,000 PSI		f'c >= 5,000 PSI	
BAR SIZE	BAR GRADE	BAR LOCATION		BAR LOCATION		BAR LOCATION	
		OTHER	TOP	OTHER	TOP	OTHER	TOP
#3	Gr 60	24"	31"	19"	24"	17"	22"
#4	Gr 60	32"	41"	25"	32"	22"	29"
#5	Gr 60	39"	51"	31"	40"	28"	36"
#6	Gr 60	47"	61"	37"	48"	33"	43"
#7	Gr 60	69"	89"	54"	70"	49"	63"
#8	Gr 60	78"	102"	62"	80"	55"	72"
#9	Gr 60	88"	115"	70"	91"	63"	81"
#10	Gr 60	99"	129"	79"	102"	70"	91"

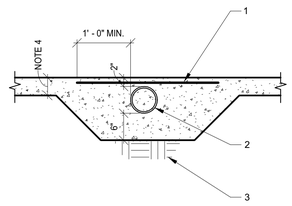
B2 TYPICAL LAP SPLICE IN CONCRETE
NO SCALE 103-01C

1. CONCRETE.
2. REMOVE FORM MATERIAL PRIOR TO PLACING ADJACENT CONCRETE.

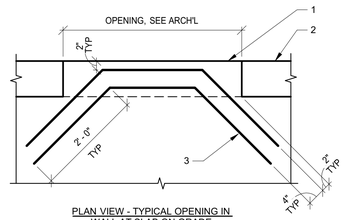


A1 TYPICAL KEY IN CONCRETE
NO SCALE 105-01

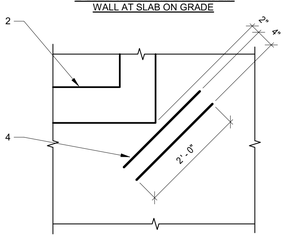
1. 4x4 - W1.4xW1.4 WWF OR #4 AT 12" O.C.
2. PIPE OR CONDUIT.
3. FIRM UNDISTURBED SOIL OR COMPACTED BASE.
4. TYPICAL SLAB THICKNESS.



A2 SLEEVE FOR PIPE AT SLAB ON GRADE
NO SCALE 101-12



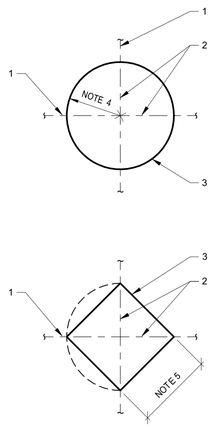
PLAN VIEW - TYPICAL OPENING IN WALL AT SLAB ON GRADE



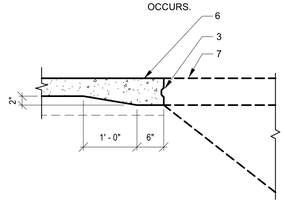
PLAN VIEW - TYPICAL AT REENTRANT CORNERS IN SLAB ON GRADE

1. CONCRETE CONSTRUCTION JOINT OR EDGE OF SLAB.
2. CONCRETE OR MASONRY WALL.
3. (2) #3 CENTERED IN SLAB AT OPENINGS IN WALLS.
4. (2) #3 CENTERED IN SLAB AT ALL REENTRANT CORNERS.

B3 TYPICAL REENTRANT CORNER REINFORCING IN SLAB ON GRADE
NO SCALE 101-10



1. 'CONC CJ' WHERE SHOWN ON PLAN.
2. CENTERLINE OF COLUMN.
3. KEYED JOINT - SEE TYPICAL KEY IN CONCRETE DETAIL.
4. 1'-6" MIN./3'-0" MAX.
5. 2'-0" MIN./4'-0" MAX.
6. CONCRETE SLAB ON GRADE
7. LINE OF CONCRETE CLOSURE POUR OR SLAB AS OCCURS.



NOTE:
- COLUMNS OMITTED FOR CLARITY.
- FOR CONFIGURATION OF SPECIFIC CLOSURE POURS, SEE PLAN.

A3 TYPICAL COLUMN CLOSURE POUR AT CONCRETE SLAB ON GRADE
NO SCALE 101-05

SELF CERTIFIED BY: _____ DATE: 03/06/2019
DONALD ANDREWS CERTIFICATE #45

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SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE

Registered Professional Engineer (Structural) CERTIFICATE NO. 61523 DAVID GRAPSAS 03/6/2019 EXPIRES: 03/31/2022

Owner	JONATHAN PITT
Proj. Name	WANDERIST OFFICE & RETAIL

TYPICAL DETAILS

Date	03/06/2019

S1.1

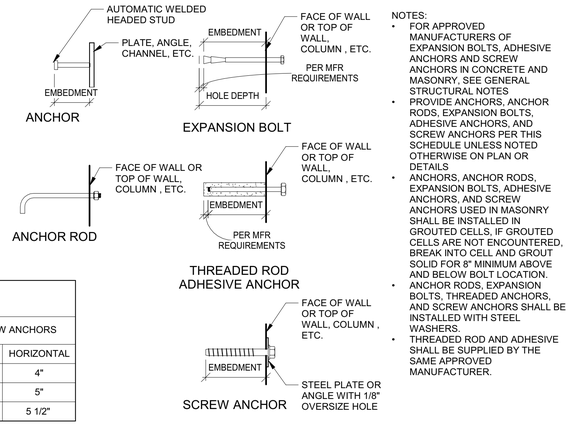
Scale	As indicated

3/7/2019 8:55:15 AM

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

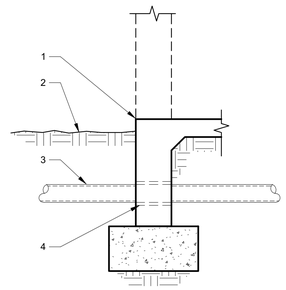
VERTICAL AND HORIZONTAL ANCHOR EMBEDMENT LENGTH FOR ANCHORS IN CONCRETE						
ANCHOR DIAMETER	ANCHOR AND ANCHOR RODS		EXPANSION ANCHORS	THREADED ROD ADHESIVE ANCHORS	REBAR ADHESIVE ANCHORS	SCREW ANCHORS
	VERTICAL	HORIZONTAL				
1/2" (#4)	5"	4"	5"	4"	4"	4"
5/8" (#5)	6 1/2"	4 1/2"	6 1/8"	5"	5"	4 3/8"
3/4" (#6)	7"	5"	7 1/2"	6"	6"	6 1/4"
7/8" (#7)	8"	6"	-	7"	7"	-
1" (#8)	9"	7"	9 3/4"	8"	8"	-
1 1/4"	11"	9"	-	-	-	-
1 1/2"	12"	10"	-	-	-	-

VERTICAL AND HORIZONTAL ANCHOR EMBEDMENT LENGTH FOR ANCHORS IN MASONRY								
ANCHOR DIAMETER	ANCHOR AND ANCHOR RODS		EXPANSION ANCHORS		THREADED ROD ADHESIVE ANCHORS		SCREW ANCHORS	
	VERTICAL	HORIZONTAL	VERTICAL	HORIZONTAL	VERTICAL	HORIZONTAL	VERTICAL	HORIZONTAL
1/2" (#4)	5"	4"	3"	3 1/2"	4 1/2"	4 1/2"	4 1/2"	4"
5/8" (#5)	6 1/2"	4 1/2"	3 1/2"	4 3/8"	5 5/8"	5 5/8"	4 1/2"	5"
3/4" (#6)	7"	5"	-	5 1/4"	-	6 3/4"	-	5 1/2"



NOTES:
- FOR APPROVED MANUFACTURERS OF EXPANSION BOLTS, ADHESIVE ANCHORS AND SCREW ANCHORS IN CONCRETE AND MASONRY, SEE GENERAL STRUCTURAL NOTES
- PROVIDE ANCHORS, ANCHOR RODS, EXPANSION BOLTS, ADHESIVE ANCHORS, AND SCREW ANCHORS PER THIS SCHEDULE UNLESS NOTED OTHERWISE ON PLAN OR DETAILS
- ANCHORS, ANCHOR RODS, EXPANSION BOLTS, ADHESIVE ANCHORS, AND SCREW ANCHORS USED IN MASONRY SHALL BE INSTALLED IN GROUTED CELLS, IF GROUTED CELLS ARE NOT ENCOUNTERED, BREAK INTO CELL AND GROUT SOLID FOR 8" MINIMUM ABOVE AND BELOW BOLT LOCATION.
- ANCHOR RODS, EXPANSION BOLTS, THREADED ANCHORS, AND SCREW ANCHORS SHALL BE INSTALLED WITH STEEL WASHERS.
- THREADED ROD AND ADHESIVE SHALL BE SUPPLIED BY THE SAME APPROVED MANUFACTURER.

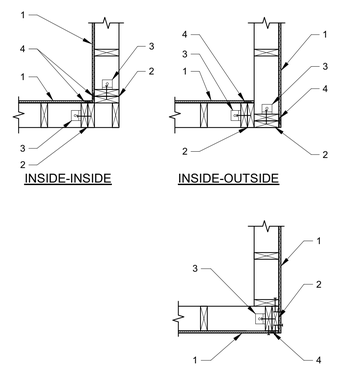
C1 TYPICAL ANCHOR, ANCHOR ROD, EXPANSION BOLT, ADHESIVE ANCHORS, AND SCREW ANCHOR SCHEDULE
NO SCALE 401-01



1. FOUNDATION WALL BELOW GRADE AND WALL ABOVE WHERE OCCURS.
2. FINISHED GRADE AS OCCURS
3. PIPE OR CONDUIT SLEEVE. PIPE SLEEVE TO HAVE AN INSIDE DIAMETER 1" LARGER THAN PIPE PASSING THROUGH WALL TO ALLOW FOR 1/2" ALL AROUND.
4. PIPE OR CONDUIT PASSING THROUGH WALL ABOVE FOOTING.

NOTE:
- NO PIPES SHALL PASS THROUGH FOOTING

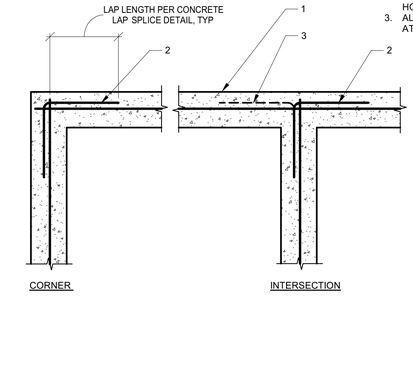
B1 TYPICAL PIPE PASSING THROUGH FOUNDATION STEM WALL
NO SCALE 105-19



1. SHEARWALL SHEATHING AS OCCURS..
2. POST AT END OF SHEARWALL PER HOLDOWN SCHEDULE.
3. HOLDOWN ATTACHED TO POST PER HOLDOWN SCHEDULE.
4. STRAP TYPE HOLDOWN AS OCCURS.

NOTE:
- VERIFY DISTANCE FROM FACE OF POST TO CENTER OF BOLT WITH HOLDOWN MFR.

B2 PLAN - SHEARWALL HOLDOWNS AT CORNER
NO SCALE 614-02



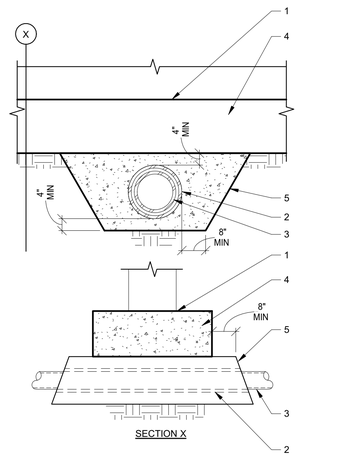
1. CONCRETE FOOTING OR CONCRETE WALL.
2. CORNER BARS SAME SIZE AND SPACING AS HORIZONTAL REINFORCING.
3. ALTERNATE LEG DIRECTION AT INTERSECTING WALLS.

B3 PLAN - TYPICAL CORNER REINFORCING IN CONCRETE FOOTING AND STEM WALL
NO SCALE 105-15

NOMINAL BEAM DEPTH "D"	NUMBER OF 3/4" DIAMETER BOLTS (ASTM F1852)
UP TO 7"	2
8" - 11"	2
12" - 14"	3
15" - 17"	4
18" - 20"	5
21" - 23"	6
24" - 29"	7
30" - 32"	8
33" - 35"	9
36"	10

1. THE TYPICAL STEEL BEAM TO STEEL COLUMN OR STEEL BEAM TO STEEL BEAM CONNECTION CONSISTS OF 3/8" SINGLE SHEAR PLATES WITH BOLTS PER SCHEDULE. USE 5/8" SHEAR PLATES WHERE "D" = 27" OR GREATER.
2. FABRICATOR SHALL TAKE NECESSARY PRECAUTIONS AGAINST BOLT SHANK OUT.
3. FABRICATOR'S OPTION:
4. PROVIDE THICKER SHEAR PLATE.
5. PROVIDE MULTIPLE WASHERS AT EITHER END OF BOLT (2 MAXIMUM PER END).

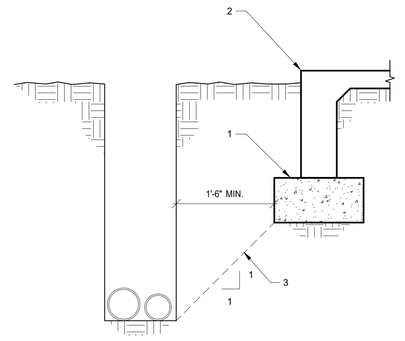
A1 TYPICAL BOLT SCHEDULE FOR TYPICAL STEEL CONNECTIONS
NO SCALE 401-03



1. CONCRETE FOOTING.
2. PIPE SLEEVE TO ALLOW FOR 1/2" CLEARANCE ALL AROUND PIPE PASSING UNDER FOOTING.
3. PIPE PASSING UNDER FOOTING.
4. NO PIPES ARE TO PASS THROUGH FOOTING.
5. CONCRETE FILL. PLACE ALL FILL PRIOR TO POURING FOOTINGS. FILL MUST REACH DESIGN STRENGTH PRIOR TO POURING FOUNDATIONS. (MINIMUM f'c = 500 PSI)

NOTE:
- NO PIPES SHALL PASS THROUGH FOOTING OR UNDER COLUMN FOOTINGS.

A2 TYPICAL PIPE PASSING BELOW WALL FOOTING
NO SCALE 105-18



1. CONCRETE WALL FOOTING.
2. BUILDING SLAB ABOVE AS OCCURS.
3. DO NOT EXCAVATE A TRENCH CLOSER THAN A 45 DEGREE ANGLE BELOW BOTTOM OF FOOTING OR FOUNDATION.

A3 TYPICAL TRENCH PARALLEL TO FOUNDATION
NO SCALE 105-13

SELF CERTIFIED BY: _____ DATE: 03/06/2019
DONALD ANDREWS CERTIFICATE #45

- PLANS WERE PREPARED BY OR UNDER THE DIRECT SUPERVISION OF, OR REVIEWED BY THE SELF-CERTIFIED PROFESSIONAL,
- PLANS ARE COMPLETE,
- THE PLANS ARE, AS OF THE DATE OF SUBMISSION, IN ACCORDANCE WITH THE REQUIREMENTS OF THE PHOENIX BUILDING CONSTRUCTION CODE AND ALL OTHER APPLICABLE LAWS.

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SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE

Registered Professional Engineer (Structural) CERTIFICATE NO. 61523 DAVID GRAPSAS 3/6/2019 EXPIRES: 03/31/2022

Owner	JONATHAN PITT
Proj. Name	WANDERIST OFFICE & RETAIL

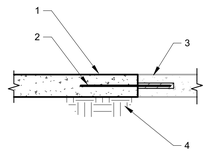
TYPICAL DETAILS

Date	03/06/2019
	S1.2
Scale	As indicated

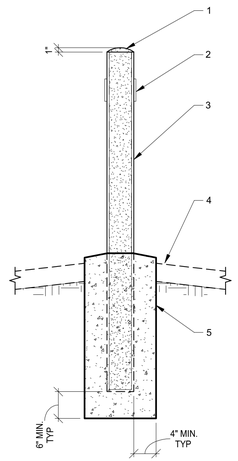
3/7/2019 8:55:16 AM

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

1. CONCRETE SLAB ON GRADE
2. #4 x 1'-8" LONG EMBEDDED 6" INTO EXISTING CONCRETE SLAB AND SET IN EPOXY. SPACE AT 24" O.C.
3. EXISTING CONCRETE SLAB ON GRADE.
4. FOR SUBASE REQUIREMENTS SEE GSN AND SOILS REPORT.



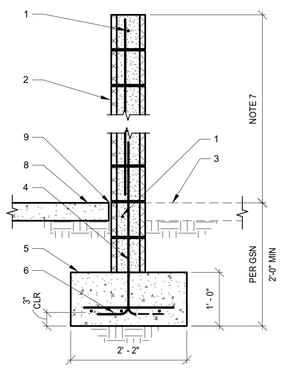
C3 TYPICAL CONCRETE SLAB AT EXISTING CONCRETE SLAB
NO SCALE 912-01



1. FILL WITH GROUT AND CROWN TOP.
2. REFLECTIVE ENGINEERS TAPE PER ARCH'L DRAWINGS.
3. 4" OR 6" STD x 8'-6" STEEL POST. SCHEDULE 40; GALVANIZED.
4. FINISHED GRADE, CONCRETE SLAB, OR ASPHALT AS OCCURS.
5. CONCRETE FOOTING (CLASS B) F'c = 2,500

NOTES:
- SAFETY POST SHALL COMPLY WITH THE MINIMUM REQUIREMENTS OF CITY OR AHJ

B3 STEEL SAFETY POST (BOLLARD)
NO SCALE 192-01

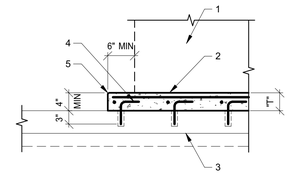


1. (1) #5 IN 8" DEEP CONTINUOUS GROUTED BOND BEAM.
2. 8" MASONRY WALL WITH #5 VERTS AT 8" O.C. GROUT SOLID.
3. FINISHED GRADE OR CONCRETE SLAB AS OCCURS.
4. DOWELS TO MATCH AND LAP VERTICAL WALL REINFORCING. LAP PER TYPICAL LAP SCHEDULE.
5. CONCRETE FOOTING WITH (3) #5 CONTINUOUS AND #5 AT 48" O.C. TRANSVERSE.
6. STANDARD 90 DEGREE HOOK. ALTERNATE BENDS.
7. FOR TOP OF WALL ELEVATION SEE ARCHITECTURAL, 6'-0" MAXIMUM.
8. 6" THICK CONCRETE SLAB ON GRADE WITH W2.9xW2.9 6x6 WWF CENTERED IN SLAB OVER A.B.C. FILL.
9. EXPANSION FILLER.

NOTES:
- TRASH ENCLOSURE SHALL COMPLY WITH THE MINIMUM REQUIREMENTS OF CITY OR AHJ

A3 6'-0" MAXIMUM FREE STANDING TRASH ENCLOSURE MASONRY WALL AND FOOTING
NO SCALE 198-05

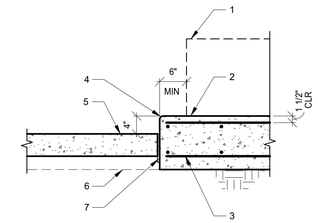
"T"	RIENFORCING
4"	#3 AT 18" O.C. EACH WAY
6"	#4 AT 16" O.C. EACH WAY
8"	#4 AT 12" O.C. EACH WAY
1'-2"	#5 AT 12" O.C. EACH WAY
1'-6"	#4 AT 16" O.C. EACH WAY TOP AND BOTTOM



1. EQUIPMENT.
2. CONCRETE HOUSEKEEPING PAD.
3. EXISTING CONCRETE SLAB ON GRADE.
4. #3 WITH 4" HOOK AT 18"O.C. EACH WAY. SET IN EPOXY.
5. TOOLED EDGE.

NOTE:
- FOR EQUIPMENT PAD THICKNESS, SIZE AND LOCATION, SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS.
- EXACT DIMENSIONS AND LOCATIONS ARE SUBJECT TO VERIFICATION PRIOR TO CONSTRUCTION DUE TO VENDER SPECIFIC INFORMATION.

B4 TYPICAL HOUSEKEEPING PAD AT SLAB ON GRADE
NO SCALE 101-21A



1. EQUIPMENT.
2. MINIMUM 12" THICK CONCRETE SLAB ON SUBGRADE.
3. #4 AT 12" O.C. EACH WAY TOP AND BOTTOM.
4. TOOLED EDGE.
5. CONCRETE SLAB.
6. FOR SUBBASE REQUIREMENTS, SEE PLANS.
7. 1/2"x6" EXPANSION MATERIAL - TYPICAL ALL SIDES.

NOTE:
- COORDINATE EQUIPMENT PAD EDGE DIMENSION AND DIMENSION ABOVE FINISHED FLOOR REQUIREMENTS WITH MECHANICAL DRAWINGS.

A4 THICKENED SLAB AT EQUIPMENT PAD
NO SCALE 101-18

SELF CERTIFIED BY: _____ DATE: 03/06/2019
DONALD ANDREWS CERTIFICATE #45

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SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE

Registered Professional Engineer (Structural)
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61523
DAVID GRAPSAS
Date Signed 3/6/2019
EXPIRES: 03/31/2022

Owner JONATHAN PITT
Proj. Name WANDERIST OFFICE & RETAIL

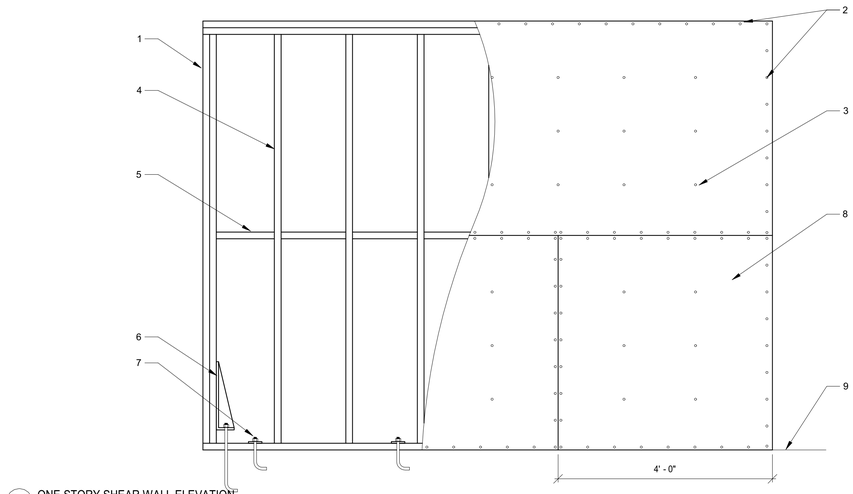
TYPICAL DETAILS

Date 03/06/2019

S1.3

Scale 3/4" = 1'-0"

3/7/2019 8:55:16 AM



C1 ONE STORY SHEAR WALL ELEVATION
NO SCALE 612-05

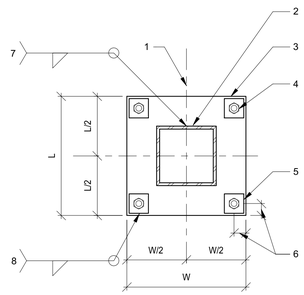
1. MULTIPLE STUDS AT END OF PANEL NAILED AT BUILT-UP POST. (MIN. 2 U.N.O.) - TYP.
2. EDGE NAILING - SEE SHEAR WALL SCHEDULE AND GSN.
3. INTERMEDIATE NAILING - SEE SHEAR WALL SCHEDULE AND GSN.
4. WOOD STUDS.
5. BLOCKING REQUIRED AT SHEATHING PANEL JOINTS.
6. HOLDOWN - FOR SIZE AND LOCATION, SEE FOUNDATION PLAN AND SHEAR WALL SCHEDULE.
7. ANCHOR RODS - FOR SIZE AND SPACING, SEE GENERAL STRUCTURAL NOTES.
8. SHEATHING MATERIAL
9. FINISHED FLOOR.

NOTE:
- WHEN SHEATHING TYPE MATERIAL IS GYPSUM BOARD, INTERMEDIATE NAILING IS THE SAME SIZE AND SPACING AS EDGE NAILING.

TYPICAL NAILING SCHEDULE	
DESCRIPTION OF BUILDING ELEMENTS	CONNECTION*
1. BLOCKING BETWEEN CEILING JOISTS, RAFTERS OR TRUSSES TO TOP PLATE OR OTHER FRAMING BELOW	(3)8d COMMON; EACH END, TOENAIL; (3)10d BOX; EACH END, TOENAIL
BLOCKING BETWEEN RAFTERS OR TRUSS NOT AT THE WALL TOP PLATE, TO RAFTER OR TRUSS	(2)8d COMMON; EACH END, TOENAIL (2)16d COMMON; END NAIL
FLAT BLOCKING TO TRUSS AND WEB FILLER	16d COMMON @ 6" O.C.; FACE NAIL
2. CEILING JOISTS TO TOP PLATE	(3)8d COMMON; EACH END, TOENAIL (3)10d BOX; EACH END, TOENAIL
3. CEILING JOIST NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS (NO THRUST)	(3)16d COMMON; FACE NAIL (4)10d BOX; FACE NAIL
4. CEILING JOIST ATTACHED TO PARALLEL RAFTER (HEEL JOINT)	PER IBC TABLE 2308.7.3.1
5. COLLAR TIE TO RAFTER	(3)10d COMMON; FACE NAIL (4)10d BOX; FACE NAIL
6. RAFTER OR ROOF TRUSS TO TOP PLATE	(3)10d COMMON; FACE NAIL (3)16d BOX; FACE NAIL (4)10d BOX; FACE NAIL
7. ROOF RAFTERS TO RIDGE VALLEY OR HIP RAFTERS; OR ROOF RAFTER TO 2-INCH RIDGE BEAM	(2)16d COMMON; END NAIL (3)10d BOX; END NAIL (3)10d COMMON; TOENAIL (3)16d BOX; TOENAIL (4)10d BOX (3" × 0.128")
8. STUD TO STUD (NOT AT BRACED WALL PANELS)	16d COMMON; 24" O.C. FACE NAIL 10d BOX; 16" O.C. FACE NAIL
9. STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS (AT BRACED WALL PANELS)	16d COMMON; 16" O.C. FACE NAIL 16d BOX; 12" O.C. FACE NAIL
10. BUILT-UP HEADER (2" TO 2" HEADER)	16d COMMON; 16" O.C. EACH EDGE, FACE NAIL 16d BOX; 12" O.C. EACH EDGE, FACE NAIL
11. CONTINUOUS HEADER TO STUD	(4)8d COMMON; TOENAIL (4)10d BOX; TOENAIL
12. TOP PLATE TO TOP PLATE	16d COMMON; 16" O.C. FACE NAIL 10d BOX; 12" O.C. FACE NAIL
13. TOP PLATE TO TOP PLATE, AT END JOINTS	(8)16d COMMON OR (12)10d BOX;EACH SIDE OF END JOINT, FACE NAIL (MINIMUM 24" LAP SPLICE LENGTH EACH SIDE OF END JOINT)
14. BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING (NOT AT BRACED WALL PANELS)	16d COMMON; 16" O.C. FACE NAIL 16d BOX; 12" O.C. FACE NAIL
15. BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING AT BRACED WALL PANELS	(2)16d COMMON; 16" O.C. FACE NAIL (3)16d BOX; 16" O.C. FACE NAIL
16. STUD TO TOP OR BOTTOM PLATE	(4)8d COMMON; TOENAIL (4)10d BOX; TOENAIL (2)16d COMMON; END NAIL (3)10d BOX; END NAIL
17. TOP OR BOTTOM PLATE TO STUD	(2)16d COMMON; END NAIL (3)10d BOX; END NAIL
18. TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	(2)16d COMMON; FACE NAIL (3)10d BOX; FACE NAIL
19. 1" BRACE TO EACH STUD AND PLATE	(2)8d COMMON; FACE NAIL (2)10d BOX; FACE NAIL
20. 1" × 6" SHEATHING TO EACH BEARING	(2)8d COMMON; FACE NAIL (2)10d BOX; FACE NAIL
21. 1" × 8" AND WIDER SHEATHING TO EACH BEARING	(3)8d COMMON; FACE NAIL (3)10d BOX; FACE NAIL
22. JOIST TO SILL, TOP PLATE, OR GIRDER	(3)8d COMMON; TOENAIL (3)10d BOX; TOENAIL
23. RIM JOIST, BAND JOIST, OR BLOCKING TO TOP PLATE, SILL OR OTHER FRAMING BELOW	8d COMMON; 6" O.C. TOENAIL 10d BOX; 6" O.C. TOENAIL
24. 1" × 6" SUBFLOOR OR LESS TO EACH JOIST	(2)8d COMMON; FACE NAIL (2)10d BOX; FACE NAIL
25. 2" SUBFLOOR TO JOIST OR GIRDER	(2)16d COMMON; FACE NAIL
26. 2" PLANKS (PLANK & BEAM FLOOR & ROOF)	(2)16d COMMON; EACH BEARING, FACE NAIL
27. BUILT-UP GIRDERS AND BEAMS, 2" LUMBER LAYERS	20d COMMON; 32" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES. 10d BOX; 24" O.C. FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES. AND; (2)20D COMMON; ENDS AND AT EACH SPLICE, FACE NAIL. (3)10d BOX; ENDS AND AT EACH SPLICE, FACE NAIL.
28. LEDGER STRIP SUPPORTING JOISTS OR RAFTERS	(3)16d COMMON; EA JOIST OR RAFTER, FACE NAIL (4)10d BOX; EA JOIST OR RAFTER, FACE NAIL
29. JOIST TO BAND JOIST OR RIM JOIST	(3)16d COMMON; END NAIL (4)10d BOX; END NAIL
30. BRIDGING OR BLOCKING TO JOIST, RAFTER OR TRUSS	(2)8d COMMON; EACH END TOENAIL (2)10d BOX; EACH END TOENAIL

*CONTRACTOR TO PICK ONE OF THE ATTACHMENTS LISTED.

A1 NAILING SCHEDULE - U.N.O. INTERNATIONAL BUILDING CODE
NO SCALE 601-06

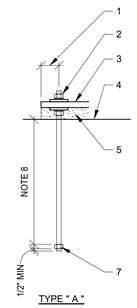


1. CENTERLINE OF STEEL COLUMN AND BASE PLATE.
2. STEEL COLUMN.
3. STEEL BASE PLATE.
4. ANCHOR RODS WITH DOUBLE NUTS.
5. STEEL PLATE WASHERS PER TYPICAL DETAIL.
6. EDGE DISTANCE PER TYPICAL DETAIL.
7. 3/16" FILLET WELD AT WALL THICKNESS 1/4" OR LESS, 1/4" FILLET WELD AT WALL THICKNESS 5/16" AND 3/8", AND 5/16" FILLET WELD AT WALL THICKNESS GREATER THAN 3/8".
8. WELD TOP STEEL PLATE WASHERS ALL AROUND AT MOMENT FRAME AND BRACED FRAME COLUMNS WHERE NOTED ON PLAN.

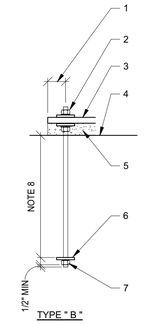
B2 PLAN - TYPICAL TUBE STEEL COLUMN BASE PLATE
NO SCALE 405-06

STEEL COLUMN BASE PLATES (GRADE 36 ANCHOR RODS WHERE SPECIFICALLY INDICATED)					
F1554, GRADE 36 ANCHOR ROD DIA	STEEL PLATE WASHER (A36)	HOLE SIZE AT STEEL PLATE WASHER	OVERSIZED HOLE DIA AT STEEL BASE PLATE	EDGE DISTANCE OF BASE PLATE TO CENTERLINE OF HOLE	THICKNESS OF GROUT
3/4"	1/2"x2 1/2"x2 1/2"	13/16"	1 5/16"	1 1/2"	2"
7/8"	5/8"x2 3/4"x2 3/4"	15/16"	1 9/16"	1 3/4"	2"
1"	3/4"x3 1/4"x3 1/4"	1 1/16"	1 13/16"	2"	3"
1 1/4"	1"x3 1/2"x3 1/2"	1 5/16"	2 1/16"	2 1/4"	3"
1 1/2"	1"x3 3/4"x3 3/4"	1 9/16"	2 5/16"	2 1/2"	3"
1 3/4"	1 1/4"x4 1/4"x4 1/4"	1 13/16"	2 3/4"	3"	4"
2"	1 1/4"x5"x5"	2 1/16"	3 1/4"	3 1/2"	4"
2 1/2"	1 1/2"x5 1/2"x5 1/2"	2 9/16"	3 3/4"	4"	5"

A2 TYPICAL PLATE WASHERS, HOLE SIZES AND EDGE DISTANCES AT STEEL COLUMN BASE PLATES
NO SCALE 405-01



TYPE " A "

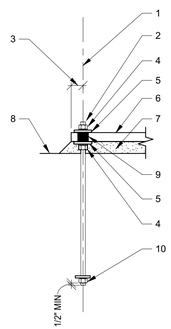


TYPE " B "

1. EDGE DISTANCE PER TYPICAL DETAILS.
2. ANCHOR ROD.
3. STEEL BASE PLATE.
4. TOP OF CONCRET
5. DRY-PACK/FLOWABLE GROUT
6. SQUARE STEEL PLATE WASHER - SIZE TO MATCH BASE PLATE WASHERS - SEE TYPICAL PLATE WASHER DETAIL.
7. HEAVY HEX NUT-TACK WELD.
8. EMBEDMENT DEPTH PER COLUMN OR BASE PLATE SCHEDULE.

NOTE:
- CONTRACTOR SHALL COORDINATE ANCHOR ROD EXTENSIONS AND THREADED LENGTHS

B3 TYPICAL ANCHOR BOLT EMBEDMENT
NO SCALE 405-02



1. CENTERLINE OF ANCHOR RODS AND HOLES
2. ANCHOR ROD
3. EDGE DISTANCE
4. HEAVY HEX NUT
5. SQUARE STEEL PLATE WASHER
6. STEEL BASE PLATE
7. DRY-PACK/FLOWABLE GROUT
8. TOP OF CONCRETE
9. OVERSIZED HOLES AT BASE PLATE
10. HEAVY HEX NUT-TACK WELD

NOTE:
- CONTRACTOR SHALL COORDINATE ANCHOR ROD EXTENSIONS AND THREAD LENGTHS.

SELF CERTIFIED BY: _____ DATE:03/06/2019
DONALD ANDREWS CERTIFICATE #45

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SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE

Registered Professional Engineer (Specialty)
CERTIFICATE NO.
61523
DAVID
GRAPSAS
Date Signed 3/6/2019
EXPIRES: 03/31/2022

Owner JONATHAN PITT
Proj. Name WANDERIST OFFICE & RETAIL

TYPICAL DETAILS

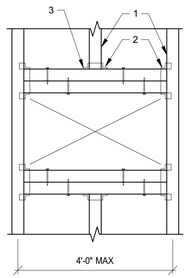
Date 03/06/2019

S1.4

Scale As indicated

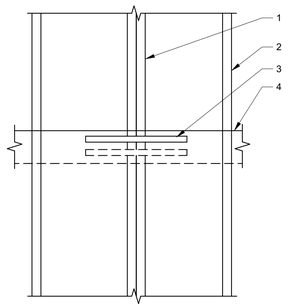
3/7/2019 8:55:17 AM

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19



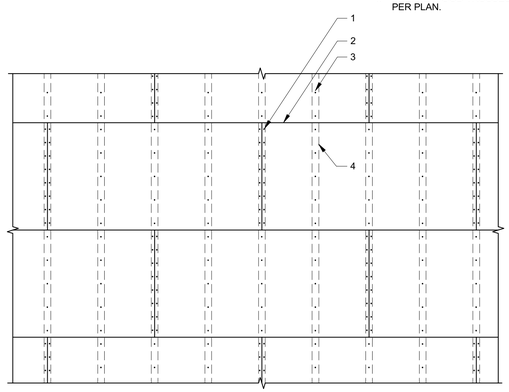
1. PREFAB WOOD TRUSS. NOTE: MANUFACTURER TO VERIFY TRUSS CAPACITY FOR MECHANICAL LOADS. MODIFY TOP AND BOTTOM CHORDS AS REQUIRED (MAINTAIN DEPTH).
2. DOUBLE 2X12 WITH SIMPSON HUS212-2TF HANGERS EACH END AND 16d AT 12" O.C. - STAGGERED.
3. TRUSS HANGER AS REQUIRED. MIN. HUTF TYPE HANGER.

C1 TYPICAL ROOF OPENING
NO SCALE
635-21



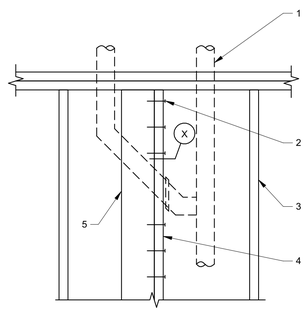
1. DOUBLE STUD AT SPLICE.
2. WOOD STUD WALL.
3. SIMPSON CMST12 STRAP (DOUBLE STRAP WHERE NOTED ON PLANS)
4. WOOD LEDGER.

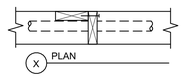
C2 TYPICAL LEDGER SPLICE
NO SCALE
614-04



1. EDGE NAILING - PER GSN.
2. PLYWOOD SHEATHING.
3. INTERMEDIATE NAILING - PER GSN.
4. PREFAB WOOD TRUSSES PER PLAN.

B1 TYPICAL ROOF PLYWOOD AT PREFAB WOOD TRUSSES
NO SCALE
695-01





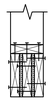
1. PLUMBING PIPE AT TOP PLATE PER TYPICAL DETAIL.
2. 16d NAILS AT 12" O.C.
3. 2x6 STUDS, TYP.
4. 2x6 STUDS WITH NOTCH/HOLE.
5. 2x6 STUD TURNED 90° - STRONG AXIS TO EXISTING STUD.

NOTE:
- JOIST/RIMBOARD/BLOCKING NOT SHOWN FOR CLARITY.

B2 ELEVATION - PIPE AT 2x6 WOOD STUD WALL
NO SCALE
614-05

1. WOOD STUD WALL AND PLATE.
2. 16d NAILS AT 12" O.C. STAGGERED BOTH SIDES.
3. 16d FACE NAILS THRU FIRST KING STUD INTO HEADER PER SCHEDULE.
4. (2) 16d TOENAILS - EACH SIDE, EACH END.
5. RUN VERTICAL STUD UP PAST HEADER AS SHOWN.
6. 16d NAILS AT 12" O.C.
7. WOOD HEADER WITH PLYWOOD.
8. DOUBLE TRIMMER STUDS UNDER HEADER BEARINGS FOR OPENING WIDTHS GREATER THAN 6'-0", PROVIDE TRIPLE TRIMMER STUDS.
9. ADDITIONAL FULL HEIGHT JAMB STUD PER BELOW WITH SAME NAILING PER NOTE #6.
10. BEARING STUDS TO TOP PLATE AT MULTI-STORY WALLS.

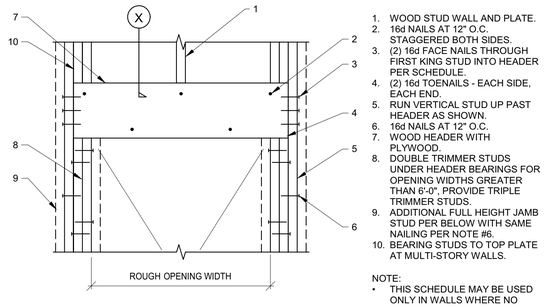
NOTE:
- THIS SCHEDULE MAY BE USED ONLY IN WALLS WHERE NO HEADER HAS BEEN CALLED OUT ON PLANS OR DETAIL.
- SINGLE 6x HEADERS MAY BE USED IN LIEU OF (3) 2x AT CONTRACTOR'S OPTION.



X SECTION

ROUGH OPENING WIDTH	HEADER AT BEARING OR SHEAR WALL	HEADER AT OTHER WALLS	ADD'L JAMB STUDS EACH SIDE	(#) OF FACE NAILS AT HEADER
0'-0" TO 3'-0"	(3) 2x6	(3) 2x4	--	(2)
3'-1" TO 6'-0"	(3) 2x8	(3) 2x6	(1)2x	(3)
6'-1" TO 8'-0"	(3) 2x12	(3) 2x8	(2) 2x	(4)
8'-1" TO 10'-0"	(3) 2x14	(3) 2x10	(3) 2x	(5)

A1 2x6 WOOD STUD WALL HEADER SCHEDULE
NO SCALE
682-22



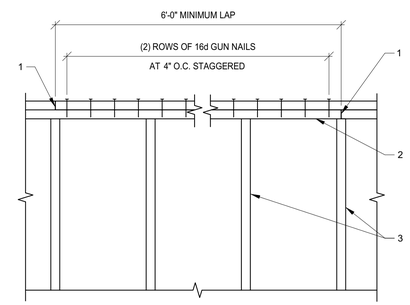
1. WOOD STUD WALL AND PLATE.
2. 16d NAILS AT 12" O.C. STAGGERED BOTH SIDES.
3. (2) 16d FACE NAILS THROUGH FIRST KING STUD INTO HEADER PER SCHEDULE.
4. (2) 16d TOENAILS - EACH SIDE, EACH END.
5. RUN VERTICAL STUD UP PAST HEADER AS SHOWN.
6. 16d NAILS AT 12" O.C.
7. WOOD HEADER WITH PLYWOOD.
8. DOUBLE TRIMMER STUDS UNDER HEADER BEARINGS FOR OPENING WIDTHS GREATER THAN 6'-0", PROVIDE TRIPLE TRIMMER STUDS.
9. ADDITIONAL FULL HEIGHT JAMB STUD PER BELOW WITH SAME NAILING PER NOTE #6.
10. BEARING STUDS TO TOP PLATE AT MULTI-STORY WALLS.

NOTE:
- THIS SCHEDULE MAY BE USED ONLY IN WALLS WHERE NO HEADER HAS BEEN CALLED OUT ON PLANS OR DETAIL.
- SINGLE 6x HEADERS MAY BE USED IN LIEU OF (3) 2x AT CONTRACTOR'S OPTION.

X SECTION

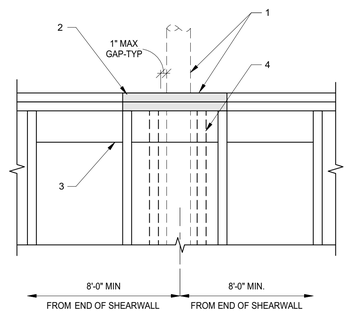
ROUGH OPENING WIDTH	HEADER AT BEARING OR SHEAR WALL	HEADER AT OTHER WALLS	ADD'L JAMB STUDS EACH SIDE	(#) OF FACE NAILS AT HEADER
0'-0" TO 3'-0"	(2) 2x6	(2) 2x4	--	(2)
3'-1" TO 6'-0"	(2) 2x8	(2) 2x6	(1)2x	(3)
6'-1" TO 8'-0"	(2) 2x12	(2) 2x8	(2) 2x	(4)
8'-1" TO 10'-0"	(2) 2x14	(2) 2x10	(3) 2x	(5)

A2 2x4 WOOD STUD WALL HEADER SCHEDULE
NO SCALE
682-21



1. TOP PLATE SPLICE OVER STUD ONLY.
2. DOUBLE TOP PLATE.
3. WOOD STUDS.

B3 TYPICAL SPLICE OF TOP PLATES
NO SCALE
614-03



1. OVERBORED (NOTCHED OR GAP) PLATED PIPE.
2. (2) SIMPSON CTS218 EACH SIDE.
3. 2x BLOCKING - ADD AT PIPE AND EACH STUD BAY EACH SIDE (3 BAYS MIN)
4. WHERE WALL IS A BEARING WALL, ADD STUD SUPPORT WITHIN 6" OF NOTCHED TOP PLATES.

NOTES:
- SEE PLANS FOR MORE INFORMATION AND NOTES NOT SHOWN.
- ATTACH SHEATHING MATERIAL TO TOP PLATES AND BLOCKING PER SHEARWALL NOTES AND DRAWINGS.

A3 PIPE IN SHEARWALL
NO SCALE
612-22

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SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE

Registered Professional Engineer (Structural) CERTIFICATE NO. 61523 DAVID GRAPSAS 3/6/2019 EXPIRES: 03/31/2022

Owner	JONATHAN PITT
Proj. Name	WANDERIST OFFICE & RETAIL

TYPICAL DETAILS

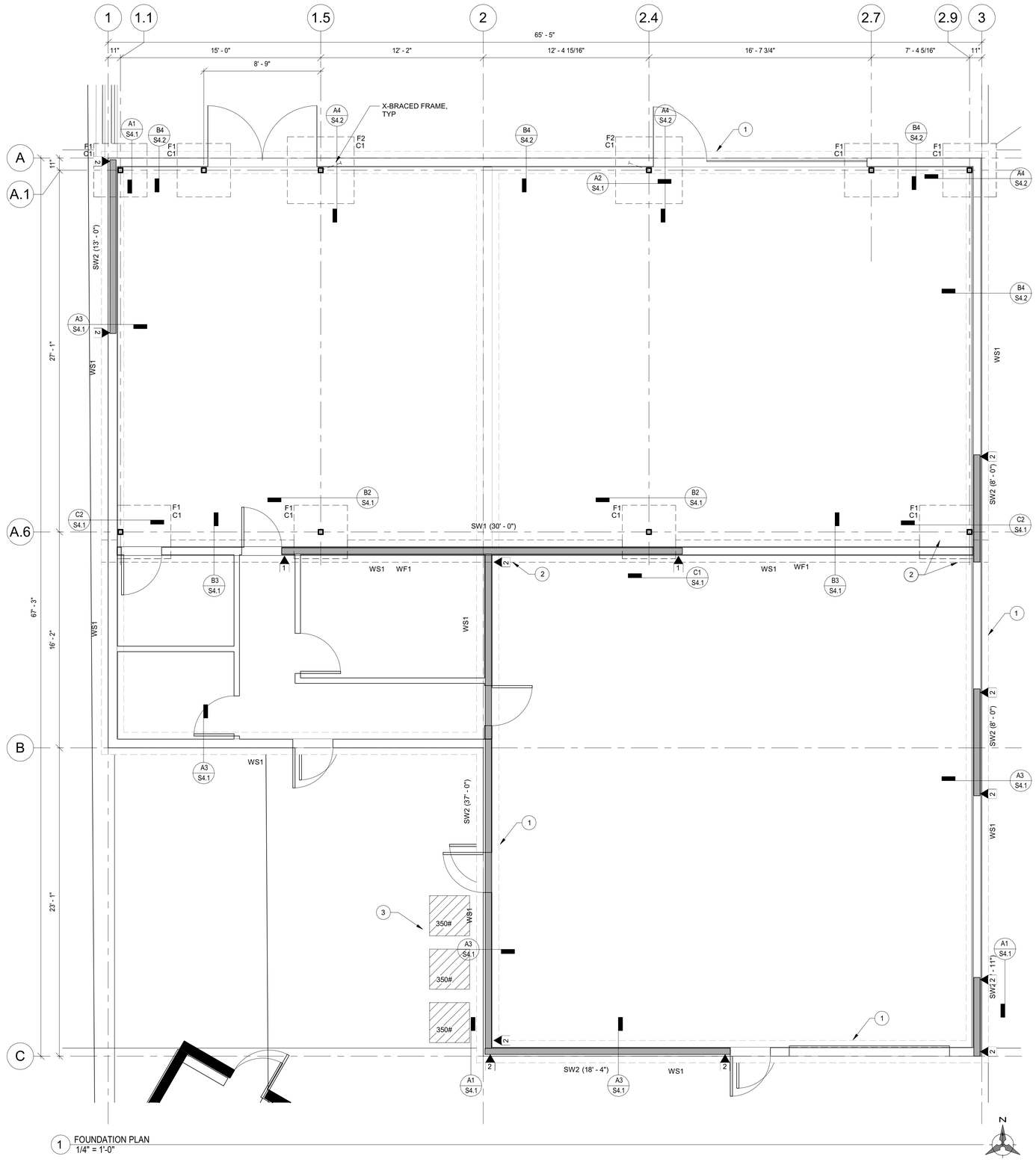
Date	03/06/2019

S1.5

Scale	As indicated

3/7/2019 8:55:18 AM

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19



1 FOUNDATION PLAN
1/4" = 1'-0"

FOUNDATION NOTES

A. VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION. DIMENSIONS, ELEVATIONS WHERE SHOWN ARE TO BE USED AS AN AID AND SHALL BE COORDINATED WITH THE ARCHITECT OR GENERAL CONTRACTOR PRIOR TO CONSTRUCTION. DO NOT USE CONC C.J. FOR LOCATING BUILDING ELEMENTS.
B. FOR ADDITIONAL INFORMATION, REFERENCE GENERAL STRUCTURAL NOTES
C. ANY REFERENCE TO ELEVATIONS ARE BASED ON A PROJECT DATUM OF 0'-0" AT FINISH FLOOR OF 1ST FLOOR. FOR MORE INFORMATION SEE ARCHITECTURAL DRAWINGS.
D. WF1, WF2, ETC. AS SHOWN ON PLAN INDICATES CONTINUOUS WALL FOOTING. SEE SCHEDULE ON SHEET S0.3. FOOTING SHALL BE CENTERED UNDER WALL U.N.O.
E. F1, F2, ETC. AS SHOWN ON PLAN INDICATES ISOLATED FOOTING. SEE SCHEDULE ON SHEET S0.3. FOOTING SHALL BE CENTERED UNDER COLUMN U.N.O.
F. FOUNDATION ELEVATIONS NOTED ON PLANS AND IN GSN ARE MINIMUMS. FOUNDATION CONTRACTOR SHALL COORDINATE WITH SOIL REPORT AND ALL TRADES TO ENSURE FOUNDATION ELEVATIONS ARE ADEQUATE. SEE TYPICAL DETAILS FOR ADDITIONAL REQUIREMENTS.
G. C1, C2, ETC. AS SHOWN ON PLAN INDICATES STEEL COLUMN. SEE SCHEDULE ON SHEET S0.3.
H. WS1, WS2, ETC. AS SHOWN ON PLAN INDICATES WALL TYPES. SEE SCHEDULE ON SHEET S0.3. SEE ARCHITECTURAL DRAWINGS FOR EXACT WALL LOCATIONS.
I. CONC. C.J. AS SHOWN ON PLAN INDICATES LOCATION OF CONCRETE CONTROL JOINT. CONTROL JOINTS MAY BE KEYED OR SAWCUT AT CONTRACTORS OPTION. CONC. C.J.'S SHALL BE PLACED WITHIN 24 HOURS OF FINISHING. SEE GSN AND TYPICAL DETAILS.
J. BUILDING CONCRETE SLAB ON GRADE SHALL BE AS NOTED ON PLAN. VERIFY EXACT SIZE AND LOCATION OF ALL DEPRESSED, RAISED, OR SLOPED CONCRETE SLABS WITH ARCHITECTURAL DRAWINGS. SEE GSN AND TYPICAL DETAILS FOR ADDITIONAL INFORMATION.

KEYNOTES

1 EXISTING CONTINUOUS 20" WIDE x 12" THICK CONCRETE FOOTING. TYPICAL AROUND PERIMETER OF BUILDING. BOTTOM OF EXISTING FOOTING AT 32" BELOW FINISHED FLOOR. CONTRACTOR TO FIELD VERIFY. TYP.
2 BOTTOM OF FOOTING TO MATCH BOTTOM OF EXISTING FOOTING. TYP.
3 APPROXIMATE LOCATION OF MECHANICAL EQUIPMENT. REFER TO TYPICAL DETAILS A4 & B4/S1.3 FOR EQUIPMENT PAD REQUIREMENTS.

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EXPIRES: 03/31/2022

Owner JONATHAN PITT
Proj. Name WANDERIST OFFICE & RETAIL

FOUNDATION PLAN

Date 03/06/2019

S2.1

Scale 1/4" = 1'-0"

SELF CERTIFIED BY: _____ DATE: 03/06/2019
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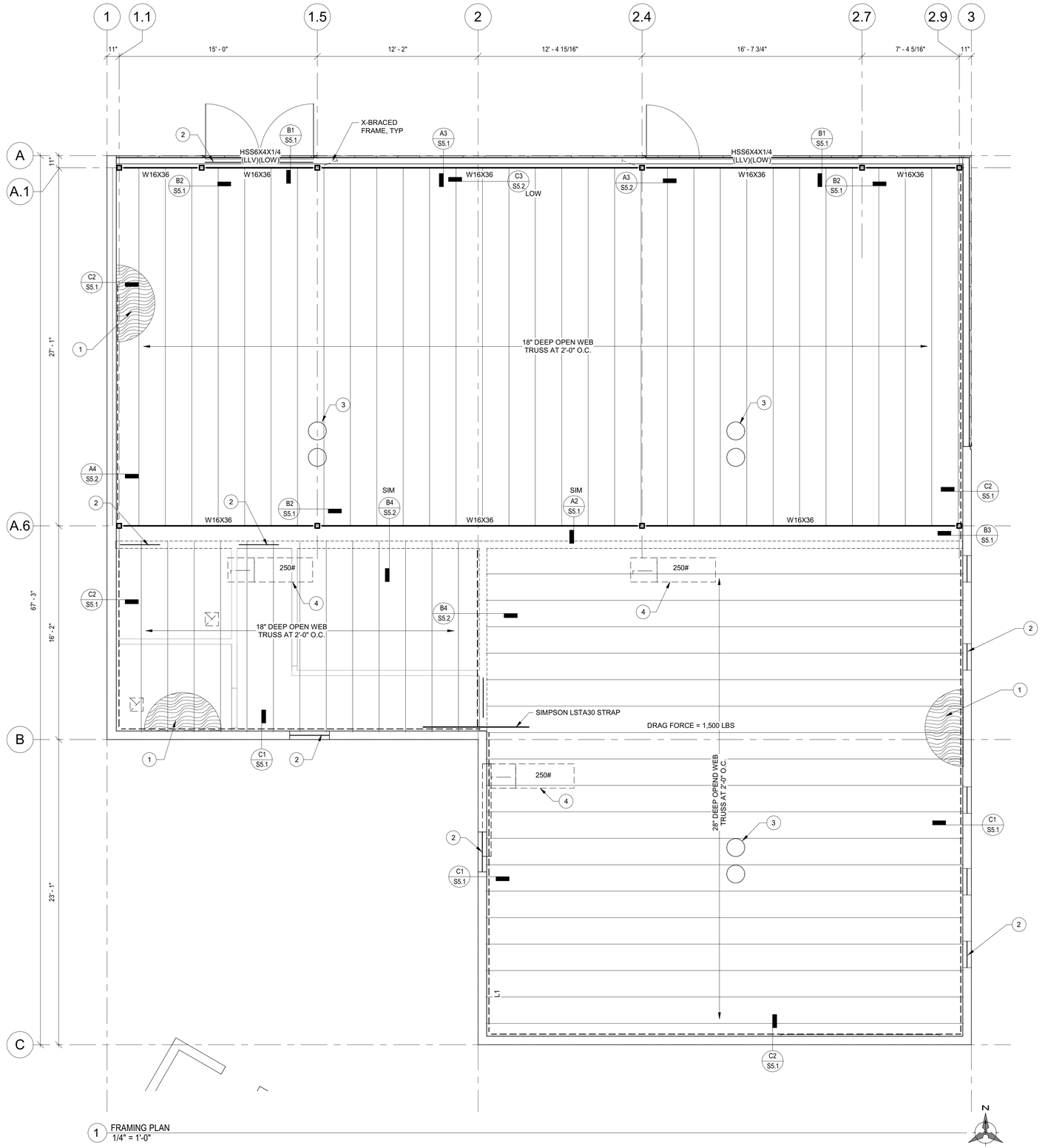

1 FRAMING PLAN
1/4" = 1'-0"

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ROOF FRAMING NOTES

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C. ANY REFERENCE TO ELEVATIONS ARE BASED ON A PROJECT DATUM OF 0'-0" AT FINISH FLOOR OF 1ST FLOOR. FOR MORE INFORMATION SEE ARCHITECTURAL DRAWINGS.
D. L1, L2, ETC. AS SHOWN ON PLAN INDICATES LEDGER. SEE SCHEDULE ON SHEET S.03
E. FOR CLARITY, ALL ROOF OPENINGS MAY NOT BE SHOWN ON FRAMING PLAN. FOR EXACT SIZE, NUMBER AND LOCATION SEE ARCHITECTURAL, MECHANICAL, AND PLUMBING DRAWINGS. FOR FRAMING AT OPENINGS, SEE TYPICAL DETAILS.
F. VERIFY EXACT SIZE, WEIGHT AND LOCATION OF MECHANICAL UNITS, EQUIPMENT AND SUPPORTS INDICATED ON PLAN WITH ARCHITECTURAL, MECHANICAL, ELECTRICAL, FIRE PROTECTION, AND PLUMBING DRAWINGS.
G. FOR CLARITY, DETAILS MAY ONLY SHOW ONE SIDE OF CONNECTION.

KEYNOTES

1 19/32" PLYWOOD SHEATHING ATTACH PER GSN.
2 OPENING IN WOOD STUD WALL. REFER TO TYPICAL WOOD STUD WALL HEADER SCHEDULE DETAIL A1/S1.5. REFER TO ARCHITECTURAL DRAWINGS FOR EXACT SIZE AND LOCATION.
3 APPROXIMATE LOCATION OF ROOF DRAINS. FOR FRAMING AROUND ROOF DRAINS, REFER TO TYPICAL DETAILS. REFER TO ARCHITECTURAL DRAWINGS FOR EXACT SIZE AND LOCATION.
4 APPROXIMATE LOCATION OF MECHANICAL UNIT. FOR FRAMING AT MECHANICAL UNITS REFER TO DETAIL A1/S5.1. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR EXACT SIZE AND LOCATION.

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FRAMING PLAN

Date 03/06/2019

S3.1

Scale 1/4" = 1'-0"

SELF CERTIFIED BY: _____ DATE: 03/06/2019
DONALD ANDREWS CERTIFICATE #45

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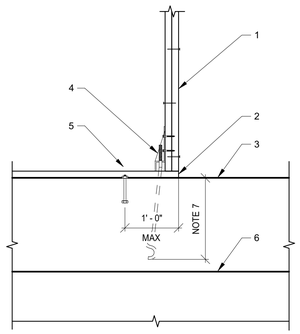
3/7/2019 8:55:20 AM

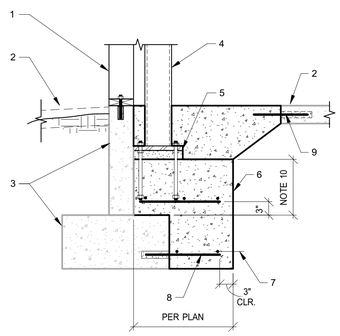
City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19
City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19



1. DOUBLE STUDS AT END OF PANEL. - NAIL AS POST WITH 16d AT 12" O.C. MINIMUM.
2. 2x CONTINUOUS WOOD PLATE.
3. CONCRETE STEM WALL AND FOOTING.
4. SIMPSON 'HTT' HOLDOWN.
5. ANCHOR BOLT.
6. TOP OF FOOTING.
7. EMBEDMENT PER HOLDOWN SCHEDULE.

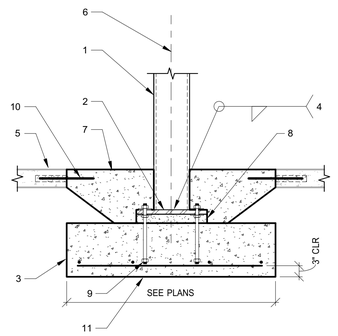
NOTE:
- ANCHOR BOLT FOR SIMPSON HOLDOWN NOT TO BE CONSIDER AS ANCHOR BOLT FOR SILL PLATE.

C1 SIMPSON 'HTT' HOLDOWN
NO SCALE 134-04 S2.1



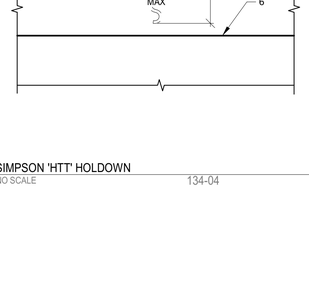
1. STUD WALL.
2. CONCRETE SLAB OR FINISHED GRADE AS OCCURS.
3. EXISTING CONCRETE FOOTING AND STEM WALL.
4. STEEL COLUMN PER PLAN.
5. STEEL BASEPLATE WITH ANCHOR RODS AND DOUBLE NUTS: REFER TO SCHEDULE.
6. CONCRETE FOOTING AND REINFORCEMENT PER SCHEDULE.
7. (2) #5 PARALLEL TO EXISTING WALL FOOTING.
8. DRILL AND EPOXY (3) #3 BARS INTO EXITING FOOTING; MIN. 6" EMBEDMENT.
9. 1/2"x16" LONG GREASED DOWEL, TYP.
10. FOOTING THICKNESS PER FOOTING SCHEDULE.

C2 NEW STEEL COLUMN FOOTING AT EXISTING WALL FOOTING
NO SCALE S2.1



1. STEEL COLUMN.
2. STEEL BASEPLATE WITH ANCHOR RODS AND DOUBLE NUTS. REFER TO SCHEDULE.
3. CONCRETE FOOTING PER PLANS.
4. FOR WELD SIZE - SEE TYPICAL STEEL COLUMN AT BASE PLATE DETAIL.
5. EXISTING SLAB ON GRADE.
6. CENTERLINE OF STEEL COLUMN, BASEPLATE AND FOOTING.
7. CONCRETE CLOSURE POUR. PROVIDE 3" COVER AROUND ALL STEEL BELOW GRADE.
8. ±1 1/2" DRYPACK.
9. HEAVY HEX NUT - TACK WELD.
10. 1/2"x16" LONG GREASED DOWEL, TYP.
11. BOTTOM OF FOOTING TO MATCH BOTTOM OF EXISTING FOOTING.

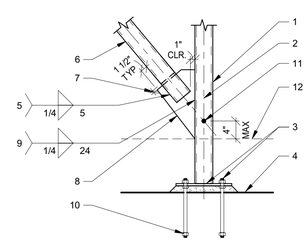
B2 INTERIOR STEEL COLUMN AT FOOTING
NO SCALE 912-11 S2.1



1. DOUBLE STUDS AT END OF PANEL. - NAIL AS POST WITH 16d AT 12" O.C. MINIMUM.
2. PROVIDE ADDITIONAL STUDS AT JAMBS TO COVER OF HOLDOWN BOLTS.
3. 2x CONTINUOUS WOOD PLATE.
4. EXSITING CONCRETE STEM WALL AND FOOTING.
5. SIMPSON 'HD' HOLDOWN.
6. ANCHOR BOLT.
7. EMBEDMENT PER HOLDOWN SCHEDULE.
8. TOP OF FOOTING.

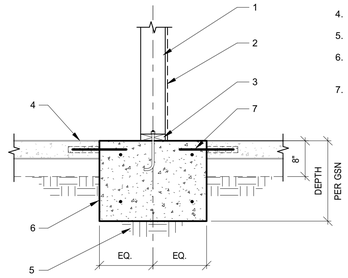
NOTE:
- ANCHOR BOLT FOR SIMPSON HOLDOWN NOT TO BE CONSIDER AS ANCHOR BOLT FOR SILL PLATE.

A1 SIMPSON 'HD' HOLDOWN
NO SCALE 134-03M S2.1



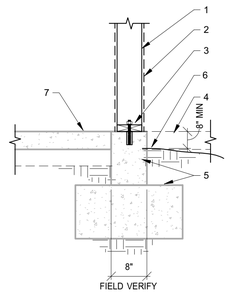
1. STEEL COLUMN.
2. CENTERLINE OF STEEL COLUMN.
3. FOR COLUMN BASE PLATE AND ANCHOR RODS, SEE COLUMN SCHEDULE.
4. TOP OF COLUMN FOOTING.
5. BRACE TO GUSSET PLATE. WELD EACH SIDE OF PLATE.
6. DIAGONAL BRACE MEMBER.
7. 1/2" MAXIMUM OVERSLOT
8. 1/2" THICK STEEL KNIFE PLATE ON CENTERLINE OF STEEL COLUMN.
9. GUSSET PLATE TO COLUMN.
10. HEAVY HEX NUT - TACK WELD.
11. WORKPOINT.
12. FINISHED FLOOR ELEVATION.

A2 BRACED FRAME CONNECTION AT STEEL COLUMN
NO SCALE 431-03M S2.1



1. WOOD STUD WALL.
2. SHEATHING MATERIAL AS OCCURS.
3. 2x CONTINUOUS PLATE WITH 1/2" DIA. ANCHOR BOLTS AT 48" O.C. - INSERT OR EXPANSION BOLTS MY BE USED IN LIEU OF ANCHOR BOLTS.
4. EXISTING CONCRETE SLAB ON GRADE.
5. FOR SUBASE REQUIREEMTNS SEE GSN.
6. CONCRETE FOOTING AND REINFORCEMENT PER PLAN AND SCHEDULE.
7. 1/2"x16" LONG GREASED DOWEL, TYP.

B3 TURNDOWN AT INTERIOR BEARING WALL
NO SCALE 134-13M S2.1



1. WOOD STUD WALL.
2. SHEATHING MATERIAL AND ATTACHEMNT AS OCCURS.
3. 2x CONTINUOUS WOOD PLATE WITH 1/2" DIA. ADHESIVE ANCHORS AT 48" O.C. - U.N.O.
4. CONCRETE SLAB WHERE OCCURS.
5. EXISTING CONCRETE STEM WALL AND FOOTING.
6. FINISHED GRADE.
7. EXISTING CONCRETE SLAB ON GRADE.

A3 EXTERIOR WOOD STUD WALL FOOTING
NO SCALE 134-01.1M S2.1

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(P) 512.900.7888

SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE

Registered Professional Engineer (Structural) CERTIFICATE NO. 61523 DAVID GRAPSAS Expires: 3/6/2019 STATE OF ARIZONA
EXPIRES: 03/31/2022

Owner JONATHAN PITT
Proj. Name WANDERIST OFFICE & RETAIL

FOUNDATION DETAILS

Date 03/06/2019

S4.1

Scale 3/4" = 1'-0"

3/7/2019 8:55:21 AM

City of Phoenix Plan #: 1901783-LPSC Date: 03/12/19

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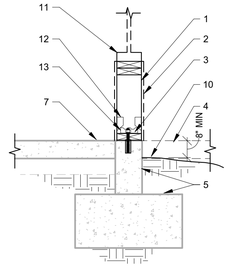
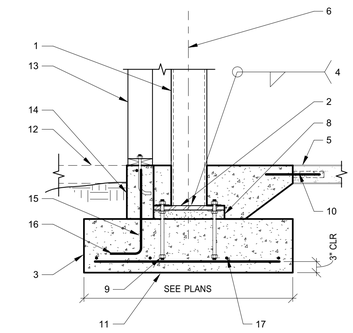


1. WOOD STUD WALL.
2. SHEATHING MATERIAL AND ATTACHEMNT AS OCCURS.
3. 2x CONTINUOUS WOOD PLATE WITH 1/2" DIA. ADHESIVE ANCHORS AT 16" O.C. - U.N.O.
4. CONCRETE SLAB WHERE OCCURS.
5. CONCRETE STEM WALL AND FOOTING.
6. #4 HOOKED DOWELS AT 48" O.C. - ALTERNATE BENDS.
7. CONCRETE SLAB ON GRADE.
8. (1) #4 CONTINUOUS.
9. THICKNESS OF CONCRETE STEM WALL TO MATCH NOMINAL THICKNESS OF WALL.
10. FINISHED GRADE.
11. CURTAIN WALL SYSTEM PER ARCH'L. DESIGNED BY OTHERS.
12. SIMPSON H3 EACH SIDE OF STUD AT EACH STUD.
13. SIMPSON A35 AT EACH STUD.

B4 WOOD STUD WALL FOOTING AT WINDOW SILL
NO SCALE 134-01.2 S2.1



1. STEEL COLUMN PER PLAN.
2. STEEL BASEPLATE WITH ANCHOR RODS AND DOUBLE NUTS. REFER TO SCHEDULE.
3. CONCRETE FOOTING PER PLANS.
4. FOR WELD SIZE - SEE TYPICAL STEEL COLUMN AT BASE PLATE DETAIL.
5. EXISTING SLAB ON GRADE.
6. CENTERLINE OF STEEL COLUMN, BASEPLATE AND FOOTING.
7. CONCRETE CLOSURE POUR. PROVIDE 3" COVER AROUND ALL STEEL BELOW GRADE.
8. ±1 1/2" DRYPACK.
9. HEAVY HEX NUT - TACK WELD.
10. 1/2"x16" LONG GREASED DOWEL, TYP.
11. BOTTOM OF FOOTING TO MATCH BOTTOM OF EXISTING FOOTING.
12. CONCRETE SLAB OR FINISHED GRADE AS OCCURS.
13. WOOD STUD WALL PER PLAN.
14. CONCRETE STEM WALL TO MATCH EXISTING. MINIMUM 8" WIDTH.
15. (1) #5 CONTINUOUS TOP AND BOTTOM. DRILL AND EPOXY INTO EXISTING CONCRETE STEM WALL. MINIMUM 5" EMBEDMENT.
16. #5 HOOK BARS. PROVIDE (1) EACH END AND AT 16" O.C. MAX. (2) MINIMUM.
17. DRILL AND EPOXY BARS EXISTING CONCRETE FOOTING . MINIMUM 5" EMBEDMENT.

A4 INTERIOR STEEL COLUMN AT FOOTING
NO SCALE 912-11M S2.1

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Owner JONATHAN PITT
Proj. Name WANDERIST OFFICE & RETAIL

FOUNDATION DETAILS

Date 03/06/2019

S4.2

Scale 3/4" = 1'-0"

3/7/2019 8:55:21 AM

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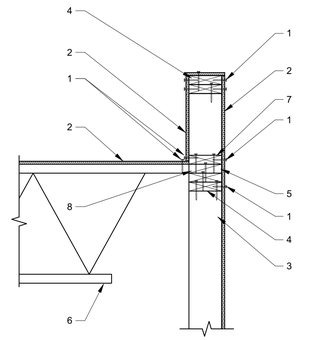
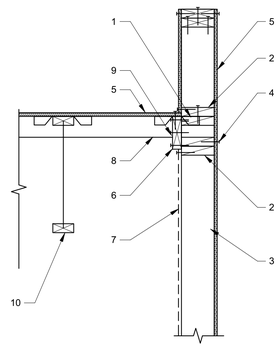


1. EDGE NAILING.
2. PLYWOOD SHEATHING; ATTACH PER GSN.
3. WOOD STUD WALL PER PLAN.
4. DOUBLE TOP PLATE.
5. PRE NOTCHED PLATE WITH 16d NAILS AT 12" O.C.
6. PREFABRICATED WOOD TJL TRUSS BY OTHERS.
7. 2x PLATE WITH 16d NAILS AT 12" O.C. - STAGGERED.
8. 2x BLOCKING WITH (3) 16d NAILS PER BLOCK.

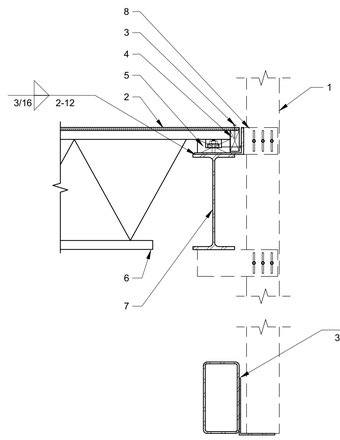
NOTE:
- FOR NUMBER OF REQUIRED PRE NOTCHED PLATES SEE DETAIL B3/S5.2.

C1 PREFAB WOOD TJL TRUSS AT WOOD STUD WALL
NO SCALE 628-06M S3.1



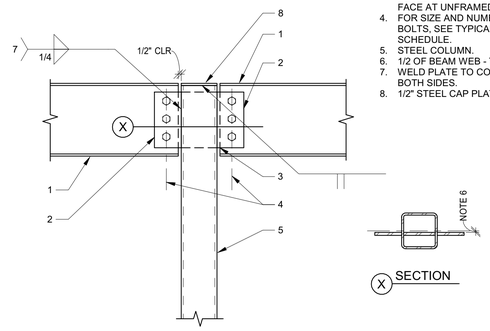
1. SIMPSON H2.5 AT EACH STUD.
2. (2) 2x BLOCKING WITH (3) 16d NAILS PER BLOCK.
3. WOOD STUD WALL PER PLAN.
4. EDGE NAILING.
5. PLYWOOD SHEATHING.
6. MIN 2x8 CONTINUOUS LEDGER WITH (2) 16d NAILS EACH STUD AND (3) EACH BLOCK.
7. SHEATHING MATERIAL AS OCCURS.
8. 2x BLOCKING AT 24" O.C. FOR 3 BAYS WITH EDGE NAILING TO ROOF SHEATHING AND H2.5 EACH END. ALIGN BLOCKING AT TRUSS CHORDS AND STAGGER H2.5 CLIPS.
9. MIN. SIMPSON ST6224 STRAP CENTERED AT LEDGER SPLICE LOCATIONS.
10. PREFAB WOOD TRUSS.

C2 PREFAB WOOD TJL TRUSS AT WOOD WALL
NO SCALE 637-21M S3.1



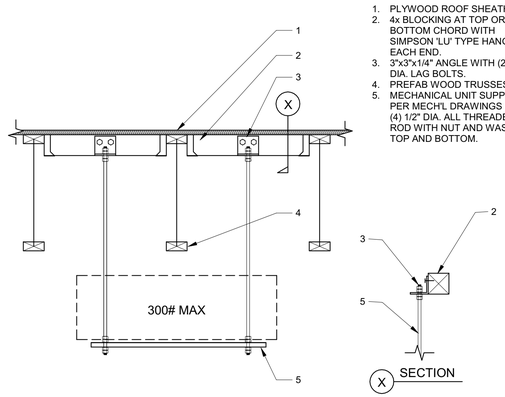
1. CURTAIN WALL SYSTEM BY OTHERS. PROVIDE DEFLECTION CLIPS AT STEEL BEAM AND BENT PLATE AS SHOWN.
2. PLYWOOD SHEATHING; ATTACH PER GSN.
3. EDGE ATTACHMENT.
4. CONTINUOUS 2x BLOCKING; ATTACH TO TOP PLATE WITH 16d NAILS AT 6" O.C.
5. CONTINUOUS 3x TOP PLATE WITH 3/4" DIA. THREADED STUDS AT 48" O.C. COUNTERSUNK.
6. PREFABRICATED WOOD TJL TRUSS BY OTHERS.
7. STEEL BEAM PER PLAN.
8. CONTINUOUS 1/4" STEEL BENT PLATE.

B1 PREFAB WOOD TRUSS AT STEEL BEAM
NO SCALE S3.1



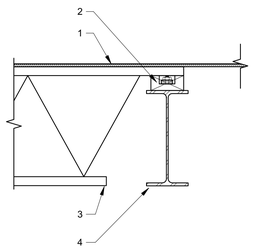
1. STEEL BEAM.
2. 3/8" STEEL KNIFE PLATE. 5/8" STEEL KNIFE PLATE WHERE "D"=27" OR MORE.
3. WHERE BEAM OCCURS 1 SIDE ONLY, EXTEND KNIFE PLATE 1/2" FROM COLUMN FACE AT UNFRAMED SIDE.
4. FOR SIZE AND NUMBER OF BOLTS, SEE TYPICAL BOLT SCHEDULE.
5. STEEL COLUMN.
6. 1/2 OF BEAM WEB - TYP.
7. WELD PLATE TO COLUMN BOTH SIDES.
8. 1/2" STEEL CAP PLATE.

B2 TYPICAL STEEL BEAM TO STEEL COLUMN
NO SCALE 415-17 S3.1



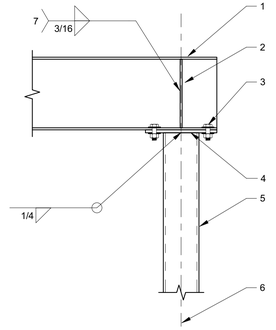
1. PLYWOOD ROOF SHEATHING.
2. 4x BLOCKING AT TOP OR BOTTOM CHORD WITH SIMPSON 'LU' TYPE HANGER EACH END.
3. 3"x3"x1/4" ANGLE WITH (2) 1/2" DIA. LAG BOLTS.
4. PREFAB WOOD TRUSSES.
5. MECHANICAL UNIT SUPPORTS PER MECH'L DRAWINGS MIN. (4) 1/2" DIA. ALL THREADED ROD WITH NUT AND WASHER TOP AND BOTTOM.

A1 MECHANICAL SUPPORT AT ROOF TRUSSES
NO SCALE 635-02M



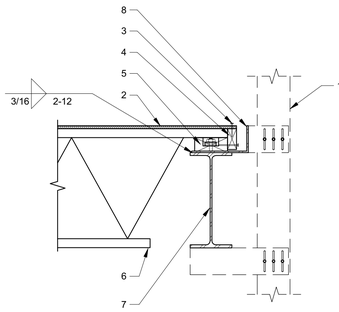
1. PLYWOOD SHEATHING; ATTACH PER GSN.
2. CONTINUOUS 3x TOP PLATE WITH 3/4" DIA. THREADED STUDS AT 48" O.C. COUNTERSUNK.
3. PREFABRICATED WOOD TRUSS BY OTHERS.
4. STEEL BEAM PER PLAN.

A2 PREFAB WOOD TJL TRUSS AT STEEL BEAM AT WOOD STUD WALL
NO SCALE 628-06M S3.1



1. STEEL BEAM.
2. 3/8" STEEL STIFFENER PLATE EACH SIDE OF BEAM - EXTEND TO EDGE OF FLANGE. TYP.
3. (4) 3/4" DIA BOLTS ON BEAM GAGE.
4. 1/2" STEEL CAP PLATE.
5. STEEL COLUMN.
6. CENTERLINE OF COLUMN.
7. WELD 3 SIDES - TYPICAL.

B3 STEEL BEAM CONNECTION STEEL COLUMN
NO SCALE 415-03 S3.1



1. CURTAIN WALL SYSTEM BY OTHERS. PROVIDE DEFLECTION CLIPS AT STEEL BEAM AND BENT PLATE AS SHOWN.
2. PLYWOOD SHEATHING; ATTACH PER GSN.
3. EDGE ATTACHMENT.
4. CONTINUOUS 2x BLOCKING; ATTACH TO TOP PLATE WITH 16d NAILS AT 6" O.C.
5. CONTINUOUS 3x TOP PLATE WITH 3/4" DIA. THREADED STUDS AT 48" O.C. COUNTERSUNK.
6. PREFABRICATED WOOD TJL TRUSS BY OTHERS.
7. STEEL BEAM PER PLAN.
8. CONTINUOUS 1/4" STEEL BENT PLATE.

A3 PREFAB WOOD TJL TRUSS AT STEEL BEAM AT WOOD STUD WALL
NO SCALE 628-06M S3.1

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STRUCTURAL
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SHEET ISSUE/REV:

NO.	DESCRIPTION	DATE

Registered Professional Engineer (Structural) CERTIFICATE NO. 61523 DAVID GRAPSAS 3/6/2019 ARIZONA U.S.A.
EXPIRES: 03/31/2022

Owner	JONATHAN PITT
Proj. Name	WANDERIST OFFICE & RETAIL

FRAMING DETAILS

Date	03/06/2019

S5.1

Scale	1" = 1'-0"

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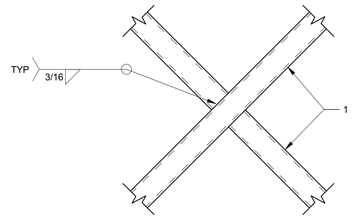
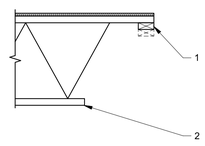
1. HSS BRACE



C3 HSS BRACE AT HSS BRACE
NO SCALE S3.1

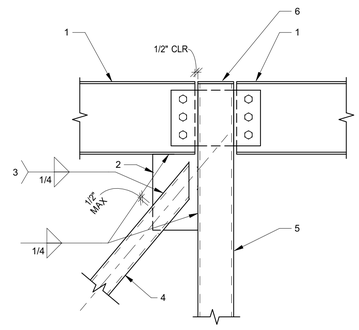
1. 2x PRENOTCHED PLATES.
2. PREFAB OPEN WEB TJL TRUSS.



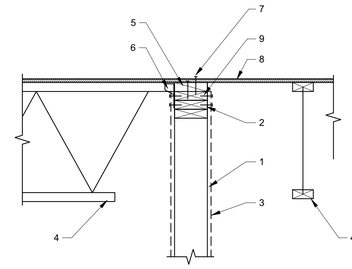
PRE-NOTCHED PLATE SCHEDULE		
NUMBER OF PRE-NOTCHED PLATES	PREFAB WOOD ROOF TRUSS DEPTH	PREFAB WOOD FLOOR TRUSS DEPTH
1	14" TO 22"	14" TO 18"
2	23" TO 34"	19" TO 34"
3	35" TO 40"	35" TO 40"

B3 TJL OPEN WEB TRUSS PRE-NOTCHED PLATES
NO SCALE

1. STEEL BEAM. SEE OTHER DETAILS FOR BEAM TO COLUMN CONNECTION.
2. 3/8" STEEL GUSSET PLATE.
3. BRACE TO GUSSET PLATE. WELD EACH SIDE OF PLATE. 5" MINIMUM EACH SIDE.
4. HSS4x4x1/4 DIAGONAL BRACE MEMBER. SLOT OVER GUSSET PLATE.
5. STEEL COLUMN.
6. 1/2" STEEL CAP PLATE.



A3 TYPICAL STEEL BEAM TO STEEL COLUMN
NO SCALE 415-17M S3.1

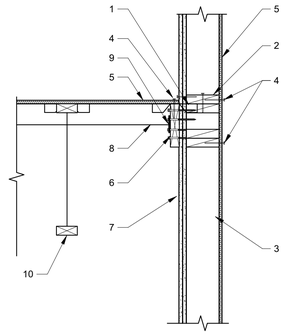


1. WOOD STUD WALL.
2. (2) 2x CONTINUOUS WOOD TOP PLATES.
3. SHEATHING MATERIAL AS OCCURS PER PLAN. FINISH PER ARCH'L.
4. PREFABRICATED WOOD TRUSS.
5. CONTINUOUS BLOCKING WITH 16d TOE NAILS AT 6" O.C. (3) MINIMUM BETWEEN TRUSSES.
6. SIMPSON H3 CONNECTOR AT EVERY OTHER STUD. CONNECTOR NOT REQUIRED IF PLYWOOD IS NAILED DIRECTLY TO CONTINUOUS TOP AND BOTTOM PLATES.
7. EDGE NAILING TO RIM BOARD.
8. PLYWOOD ROOF SHEATHING PER PLANS.
9. PRE-NOTCHED PLATE WITH 16d NAILS AT 12" O.C.

NOTE:
- FOR NUMBER OF REQUIRED PRE NOTCHED PLATES SEE DETAIL B3/S5.2.

B4 WOOD JOIST AT WOOD STUD WALL
NO SCALE 625-06 S3.1

1. SIMPSON H2.5 AT EACH STUD.
2. (2) 2x BLOCKING WITH (3) 16d NAILS PER BLOCK.
3. WOOD STUD WALL PER PLAN.
4. EDGE NAILING.
5. PLYWOOD SHEATHING.
6. MIN 3x8 CONTINUOUS LEDGER CONNECTED DIRECTLY TO STUDS WITH (2) 16d NAILS EACH STUD AND (3) 16d EACH BLOCK. AT FIRE RATED WALL PROVIDE 3x12 LEDGER ON FACE OF FIRE ASSEMBLY SHEATHING AS SHOWN WITH (4) 1/4"x4 1/2" SIMPSON SDS SCREWS AT EACH STUD.
7. SHEATHING MATERIAL AS OCCURS. REFER TO ARCHITECTURAL DRAWINGS FOR FIRE RATING REQUIREMENTS.
8. 2x BLOCKING AT 24" O.C. FOR 3 BAYS WITH EDGE NAILING TO ROOF SHEATHING AND H2.5 EACH END. ALIGN BLOCKING AT TRUSS CHORDS AND STAGGER H2.5 CLIPS.
9. MIN. SIMPSON ST6224 STRAP CENTERED AT LEDGER SPLICE LOCATIONS.
10. PREFAB WOOD TJL TRUSS.



A4 CONTINUOUS WOOD LEDGER AT WOOD STUD WALL
NO SCALE 637-02M S3.1

SELF CERTIFIED BY: _____ DATE: 03/06/2019
DONALD ANDREWS CERTIFICATE #45

- PLANS WERE PREPARED BY OR UNDER THE DIRECT SUPERVISION OF, OR REVIEWED BY THE SELF-CERTIFIED PROFESSIONAL,
- PLANS ARE COMPLETE,
- THE PLANS ARE, AS OF THE DATE OF SUBMISSION, IN ACCORDANCE WITH THE REQUIREMENTS OF THE PHOENIX BUILDING CONSTRUCTION CODE AND ALL OTHER APPLICABLE LAWS.

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ERWIN | ARCHITECTURE DEVELOPMENT

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S5.2

Scale	As indicated

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