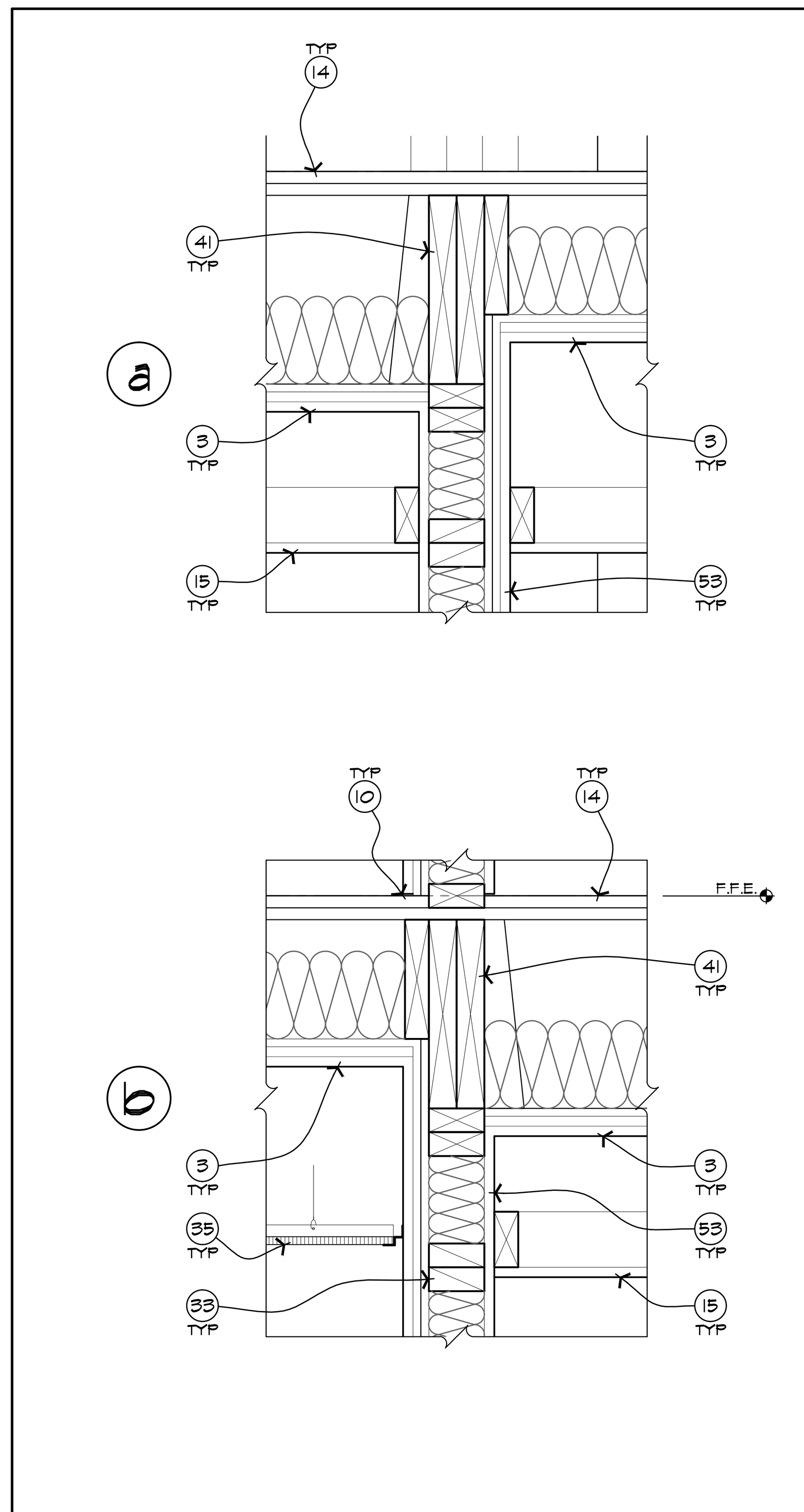
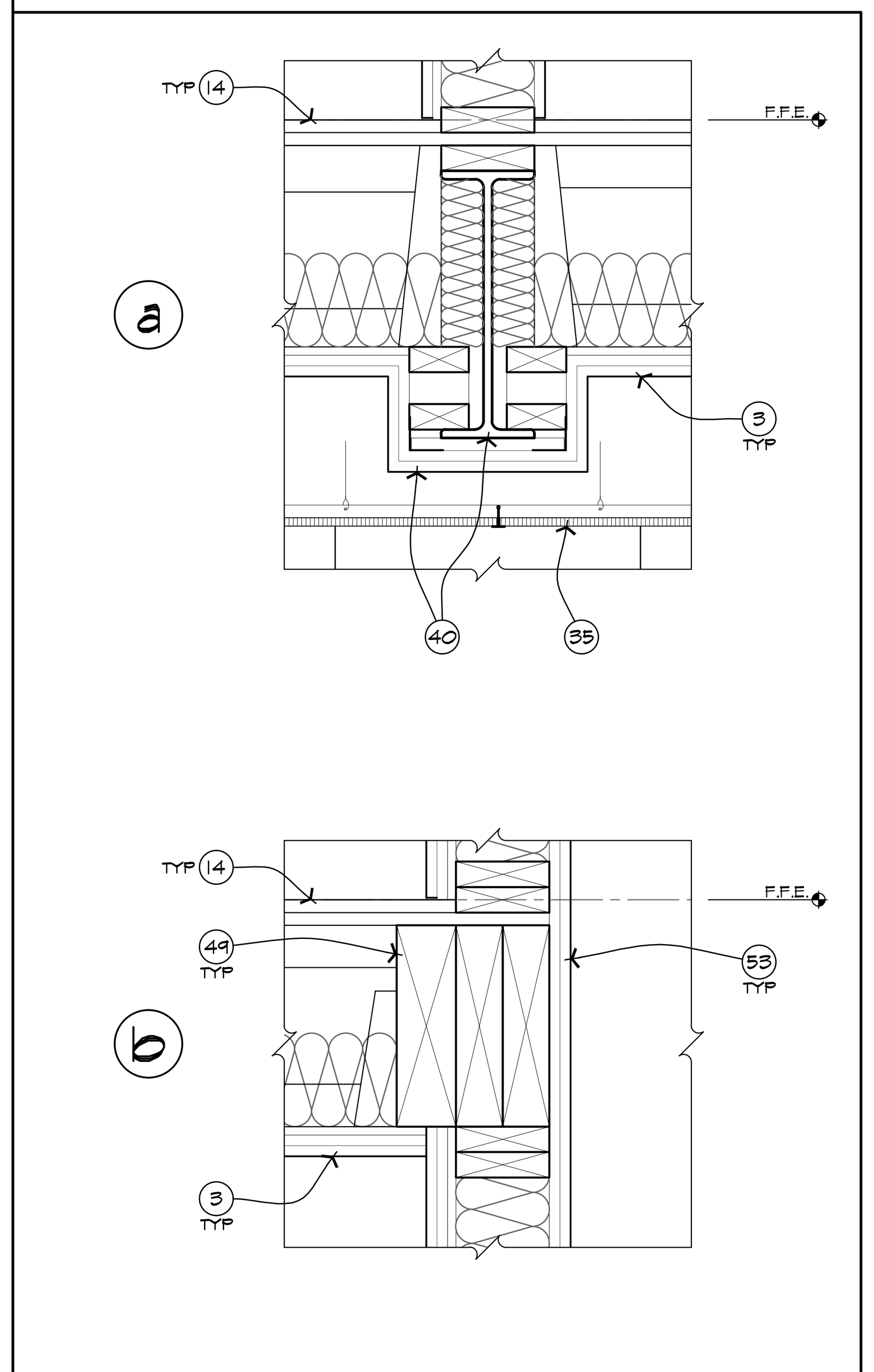
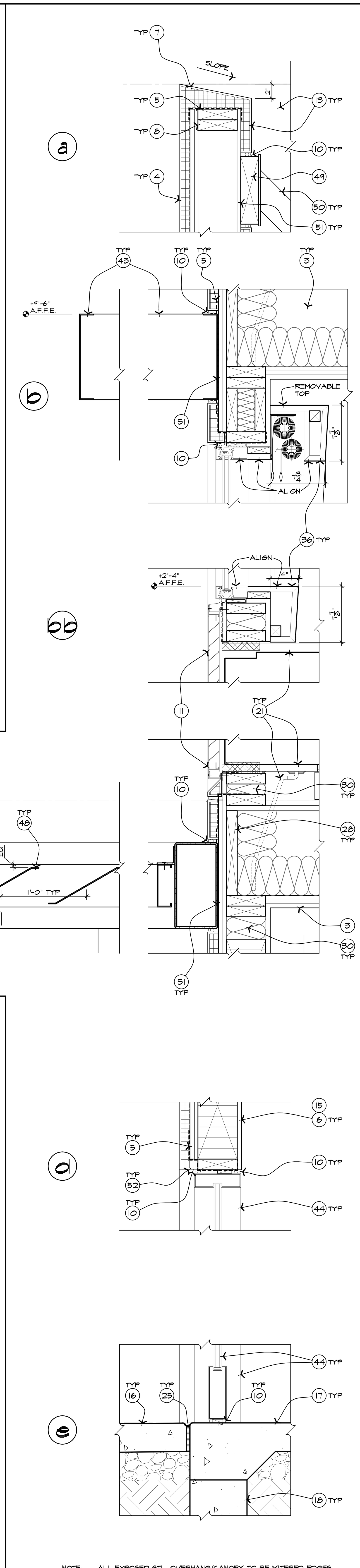


1 TYP DETAILS
SCALE: 1 1/2" = 1'-0"

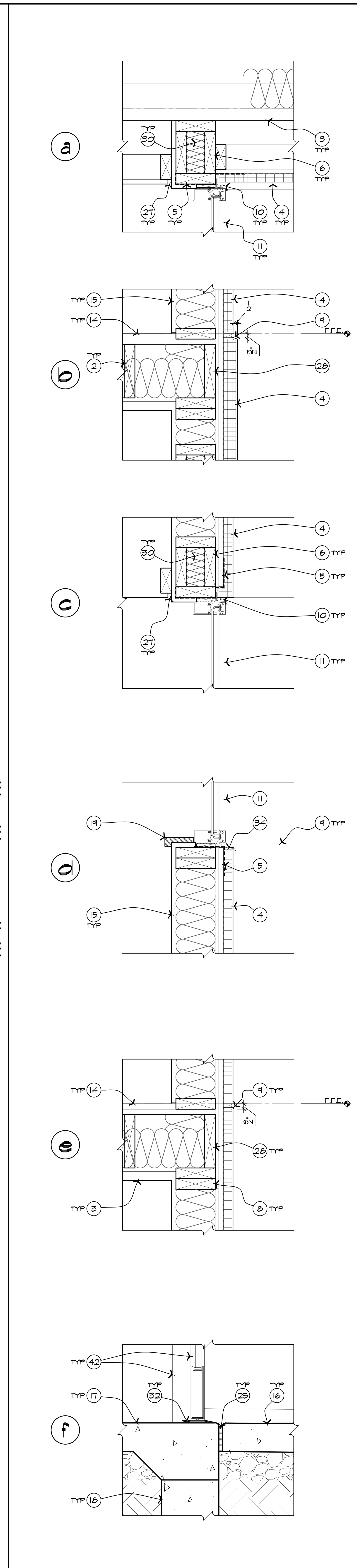


2 TYP DETAILS
SCALE: 1 1/2" = 1'-0"

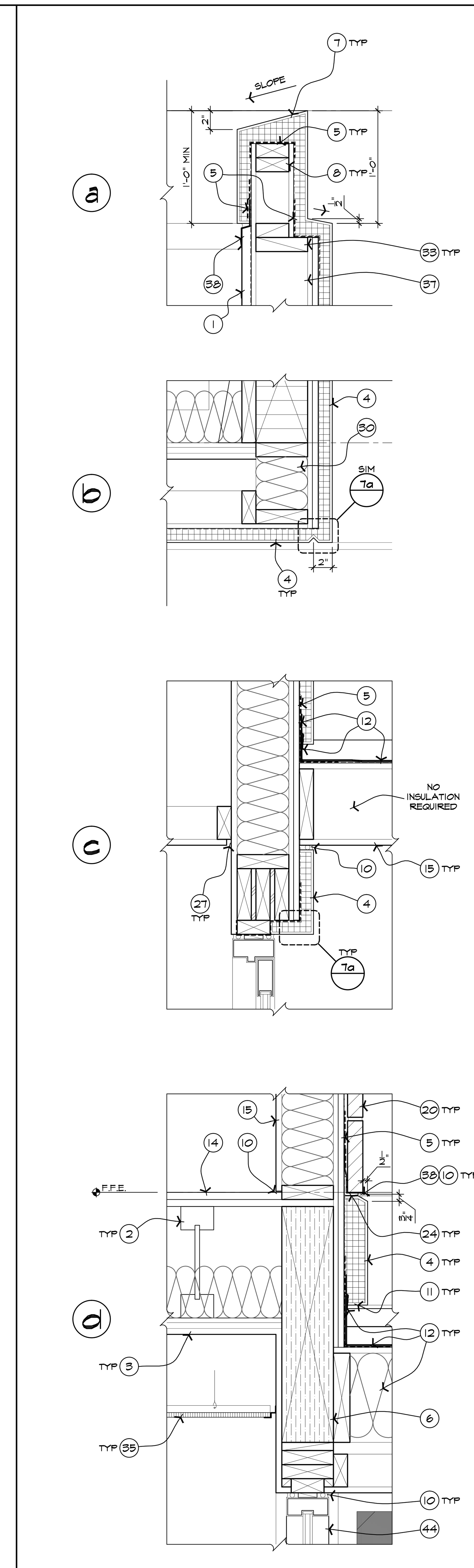


NOTE: ALL EXPOSED STL. OVERHANG/CANOPY TO BE MITERED EDGES, GRIND ALL WELDS SMOOTH, PRIMED & PAINTED.

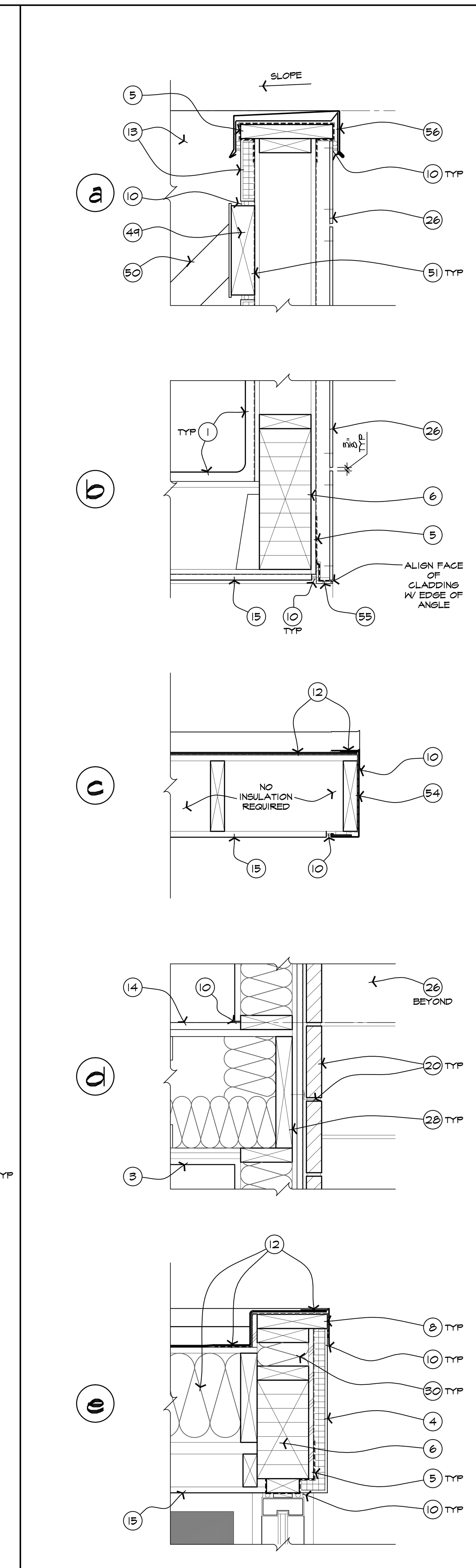
3 TYP DETAILS
SCALE: 1 1/2" = 1'-0"



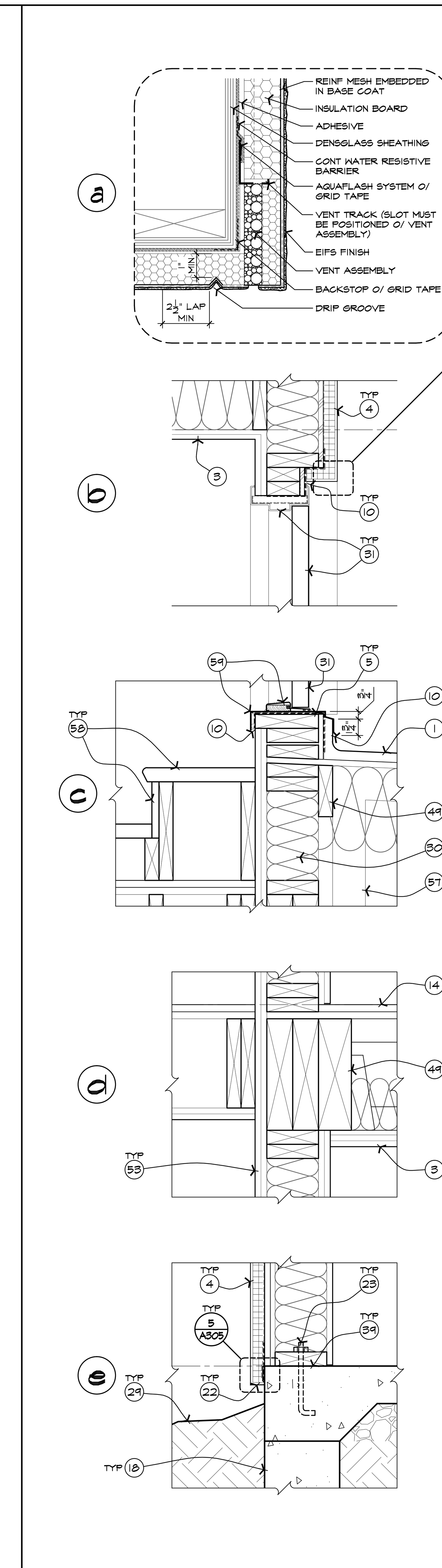
4 TYP DETAILS
SCALE: 1 1/2" = 1'-0"



5 TYP DETAILS
SCALE: 1 1/2" = 1'-0"



6 TYP DETAILS
SCALE: 1 1/2" = 1'-0"



7 TYP DETAILS
SCALE: 1 1/2" = 1'-0"

KEYNOTES

1. P.U. FOAM ROOF SYSTEM OVER EXT. GRADE PLYWD ROOF DECKING, TAPERED FOAM AT ROOF TO DRAIN & BRING FOAM UP ON THE PARAPET WALL TO UNDERSIDE OF FLASHING AT PARAPET CAP. PROVIDE R-30 BATT INSULATION IN ROOF FRAMING.
2. STRUCT WOOD MEMBER W/ ACOUSTICAL BATT INSULATION IN FIRE RATED FLOOR/CEILING ASSEMBLY.
3. FIRE RATED FLOOR/CEILING/ROOF ASSEMBLY - SEE SHEET A502-4.
4. EIFS O/ MIN 1 1/2" THK XPS FOAM BD (TYP - U.N.O. AS INDICATED ON DWGS) ON WEATHER RESISTIVE BARRIER OVER EXTERIOR GRADE PLYWD SHEATHING & 2x6 WOOD STUD FRAMING @ 16" O.C. W/ R-19 BATT INSULATION IN CAVITY. OVERLAP WEATHER RESISTIVE BARRIER @ FLASHING TAPE AS REQUIRED PER MANUFACTURER'S INSTRUCTIONS. PROVIDE ADDITIONAL LAYER OF 5/8" THK TYPE 'X' GYP BD O/ PLYWD SHEATHING @ FIRE RATED WALL, SEE WALL TYPE DESIGNATION ON FLOOR PLANS.
5. APPLY SELF ADHERED FLASHING TAPE AT TOP OF PARAPET, ALL DOOR/WINDOW OPENINGS AT HEAD/JAMB/SILL & ALL ADJOINING MATERIALS - PROVIDE LAPPING PER MANUFACTURER'S RECOMMENDATION.
6. BEAM/HEADER, SEE STRUCT. DWGS.
7. PARAPET CAP.
8. DBL 2x WD PLATE.
9. EIFS REVEAL AS OCCURS BETWEEN FLOORS & EIFS CONTROL JOINT AS OCCURS AT EACH FLOOR LINE, SEE WALL SECTIONS AND EXTERIOR ELEVATIONS FOR ACTUAL LOCATIONS.
10. CONT SEALANT - W/ BACKER ROD WHERE REQUIRED.
11. ALUMINUM WINDOW W/ INTEGRATED MECH GRILLE BELOW WHERE OCCURS, SEE DOOR & WINDOW SCHEDULES.
12. SINGLE PLY ROOFING SYSTEM W/ MTL EDGE, TERMINATION BAR, AND COUNTER FLASHING AS REQUIRED PER MANUFACTURER'S INSTRUCTION O/ EXT. GRADE PLYWD ROOF DECKING ON 2x WD JOISTS, SEE STRUCT. DWGS. PROVIDE R-30 BATT INSULATION IN ROOF FRAMING (U.N.O.).
13. EIFS ON THE BACK SIDE OF PARAPET.
14. 3/4" THK. LIGHTWEIGHT CONC. TOPPING PER ESR #1153.
15. 5/8" THK GYP BD FINISH OVER FRAMING. PROVIDE FIRE RATED GYP BD WHERE REQUIRED & M.R. OR GLASS MAT FACED GYP BD AT WET/DAMP AREAS.
16. CONC SIDEWALK AS OCCURS.
17. CONC TURNDOWN, SEE STRUCT DWGS
18. CONC FOOTING, SEE STRUCT DWGS.
19. SOLID SURFACE SILL - SHIM TO LEVEL, MASTIC APPLIED W/ CONT SEALANT ALL SIDES, SEE ALSO I.D. DWGS.
20. ARCHITECTURAL MASONRY UNITS W/ ACCENT BANDS SET IN MORTAR O/ MTL. LATH & WEATHER RESISTIVE BARRIER ON 2x WD STUD FRAMING W/ CONTROL JOINT @ FLOOR LINES - PROVIDE ANCHORS & TIES AS REQUIRED, TO BE INSTALLED PER MANUFACTURER'S INSTRUCTION.
21. PTAC UNIT W/ REQUIRED ACCESSORIES AS OCCURS. RUN CONDENSATED DRAIN PIPE WITHIN FLR/CLG ASSEMBLY & TURN/RUN PIPE DOWN IN BETWEEN WALL CAVITY.
22. DRAINAGE STRIP OR WEEP SCREED TRACK, INSTALLED PER MANUFACTURER'S REQUIREMENT.
23. SILL PLATE W/ ANCHOR BOLT, SEE STRUCT DWGS.
24. STL. SUPPORT ANGLE PER MANUFACTURER'S INSTRUCTION.
25. EXPANSION JOINT FILLER.
26. COMPOSITE CLADDING O/ 2x WD NAILING STRIPS & WEATHER RESISTIVE BARRIER O/ EXTERIOR SHEATHING - TO BE INSTALLED PER MANUFACTURER'S INSTRUCTION.
27. 1/2" REVEAL - PAINTED BLACK.
28. 2x WD RIM BD.
29. FINISH GRADE WHERE OCCURS, SEE CIVIL DWGS.
30. INFILL VOID W/ BATT INSULATION.
31. PAINTED HOLLOW MTL DOOR W/ STL FRAME, SEE DOOR SCHEDULE.
32. ALUM THRESHOLD.
33. WD BLOCKING AS REQUIRED.
34. SILL PAN FLASHING W/ TURNED UP SIDES, WATERTIGHT SEAMS W/ CONT. SEALANT.
35. SUSPENDED CLG GRID WHERE OCCURS, SEE REFL CLG PLAN.
36. P. LAM. FACED SHADE/DRAPERY SURROUND O/ 3/4" PLYWD - TYP AT ALL GUEST ROOM WINDOWS.
37. NOTCHED 2x6 WD STUD TO 2x4 @ TOP OF PARAPET.
38. PAINTED GALV MTL FLASHING.
39. COMPRESSED SILL GASKET.
40. STL. BEAM AS OCCURS - CONT. 1 HR FIRE RATED ASSEMBLY WRAPPING @ U/S OF BEAM W/ 2 LAYERS OF TYPE 'X' GYP BD & BLOCKING AS REQUIRED.
41. DBL. WD RIM BD, SEE STRUCT. DWGS.
42. ALUM DOOR & FRAME AS OCCURS, SEE DOOR SCHEDULE.
43. INVERTED PAINTED 2" x 12" COLD-FORMED STL. STUD FRAMING OVERHANG - ATTACHED TO WALL, SEE STRUCT DWGS.
44. ALUM STOREFRONT SYSTEM SET IN CONT BEAD OF CAULK, SEE WINDOW SCHEDULE.
45. PAINTED 6" x 12" TUBE STL. CANOPY BEAM, SEE STRUCT DWGS.
46. PAINTED 2 1/2" x 2 1/2" STL. ANGLE BRACKET, SEE STRUCT DWGS.
47. PAINTED 2" x 6" STL CHANNEL BORDER, SEE STRUCT DWGS.
48. PAINTED 6" DEEP ALUM. LOUVER INSERT SUNSCREEN.
49. WD LEDGER, SEE STRUCT DWGS.
50. PARAPET BRACING AS OCCURS, SEE STRUCT DWGS.
51. CONT. UNINTERRUPTED WEATHER RESISTIVE BARRIER O/ EXT. SHEATHING AT WALL.
52. EIFS DRIP EDGE.
53. FIRE RATED WALL ASSEMBLY, SEE WALL TYPE DESIGNATIONS ON FLOOR PLANS, SHEET A101-4 & THEN WALL TYPES ON SHEET A501.
54. PAINTED MTL FASCIA W/ DRIP EDGE TO MATCH ROOF EDGE.
55. PAINTED 16 GA MTL ANGLE BOTTOM EDGE W/ CONT SEALANT & WEEP HOLES @ 24" O.C.
56. PAINTED GALV. MTL PARAPET COPING W/ CONT. CLEAT ON DBL 2x WD PLATE TO MATCH ADJACENT EIFS COLOR.
57. PREFABRICATED WD TRUSS, SEE STRUCT. DWGS.
58. 2x WD TREAD W/ ROUNDED NOSING & 3/4" THK PLYWD RISER O/ 2x WD FRAMING.
59. PEMKO 8451-Q SET IN BED OF CAULK O/ 1/8" MTL BENT PLATE O/ SHIM.

GKI
GERALD R. KESLER, INC.
ARCHITECTS
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Fax: 602-466-2160
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REGISTERED ARCHITECT
CERTIFICATE NO. 13452
GERALD KESLER
Date Signed 7-15
ARIZONA, U.S.A.
EXP. 6-30-18

INN CODE # 16771-PHXCT
1170 S. Arizona Ave.
Chandler, Arizona, 85286

Holiday Inn Express & Suites

JOB NUMBER
1401

DATE
07-24-2015

REVISION

SHEET TITLE
TYPICAL DETAILS

SHEET NUMBER
A505

3042

DOOR SCHEDULE - PUBLIC AREAS & BOH

DOOR #	ROOM #	ROOM NAME	DOOR TYPE	(NOMINAL) DOOR SIZE W (WIDTH) X H (HEIGHT)	DOOR THICKNESS	DOOR		FRAME		FIRE RATED	HARDWARE SET	STC RATING	REMARKS
						MATERIAL	FINISH	TYPE	FINISH				
GROUND FLOOR													
100	100	STAIR #1-2	D	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	90	5	-	
100A	100A	ELEC	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	45	6	47	
100B	100	STAIR #1-2	D	3'-0" X 7'-0"	1 3/4"	HM	PT	A	PT	90	7	-	PANIC DEVICE
100C	100A	ELEC	B	PR 3'-0" X 7'-0"	1 3/4"	HM	PT	A	PT	-	9	-	PANIC DEVICE
101	101	UTILITY	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	45	6	47	
101A	101A	RISER	B	3'-0" X 7'-0"	1 3/4"	HM	PT	A	PT	-	8	-	
101B	101	UTILITY	B	PR 3'-0" X 7'-0"	1 3/4"	HM	PT	A	PT	-	9	-	
103	103	LAUNDRY	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	45	10	47	
103A	103A	UNISEX R.R.	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	-	11	-	
103B	103	LAUNDRY	B	1'-6" X 6'-8"	1 3/4"	HC WD	PT	WD	PT	-	12	-	
103C	103	LAUNDRY	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	45	13	-	
104	104	PANTRY	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	45	6	-	
105	105	EMPLOYEE	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	45	10	-	
106	106	BREAKFAST BAR	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	45	6	-	
107	107	WORK STATION	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	-	14	-	
107A	107A	SALES	C	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	-	15	-	
107B	107B	MANAGER	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	-	15	-	
107C	107C	PBX	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	-	16	-	
107D	107D	LUGGAGE	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	-	16	-	
108	108	MEETING	H	5'-2" X 8'-0"	1 3/4"	SC WD	PT	A	PT	-	4	-	
108A	108	MEETING	F	3'-0" X 7'-0"	1 3/4"	AL/GL	ANOD	AL	ANOD	-	17	-	
110	110	DINING	F	3'-0" X 7'-0"	1 3/4"	AL/GL	ANOD	AL	ANOD	-	17	-	
111	111	LOBBY	E	PR 2'-6" X 7'-0"	-	GL	ANOD	AL	ANOD	-	18	-	AUTOMATIC DOOR W/ BREAK AWAY FEATURE
111A	111A	VESTIBULE	E	PR 2'-6" X 7'-0"	-	GL	ANOD	AL	ANOD	-	-	-	AUTOMATIC DOOR W/ BREAK AWAY FEATURE
112	112	WOMEN R.R.	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	-	19	-	
112A	112A	MEN R.R.	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	-	19	-	
112B	112B	CORRIDOR	F	3'-0" X 7'-0"	1 3/4"	AL/GL	ANOD	D	ANOD	-	20	-	PANIC DEVICE
114A	114A	EXERCISE	C	3'-0" X 7'-0"	1 3/4"	WD/GL	PT	B	PT	-	21	-	
116A	116A	GUEST LAUNDRY	C	3'-0" X 7'-0"	1 3/4"	WD/GL	PT	A	PT	-	21	-	
134	134	STAIR #1-1	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	90	5	-	
134A	134	STAIR #1-1	B	3'-0" X 7'-0"	1 3/4"	HM	PT	A	PT	-	7	-	PANIC DEVICE
135	135	CORRIDOR	F	3'-0" X 7'-0"	1 3/4"	AL/GL	ANOD	C	ANOD	-	20	-	PANIC DEVICE
135A	135A	CORRIDOR	F	3'-0" X 7'-0"	1 3/4"	AL/GL	ANOD	C	ANOD	-	20	-	PANIC DEVICE
SECOND, THIRD, AND FOURTH FLOOR													
200 300 400	200 300 400	STAIR #-2	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	90	22	-	PANIC DEVICE
200A 300A 400A	200A 300A 400A	STORAGE	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	45	16	-	
202A 302A 402A	202A 302A 402A	CHUTE	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	45	23	-	
214B 314B 414B	214B 314B 414B	ELEC	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	45	6	-	
216 316 416	216 316 416	HOUSEKEEPING	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	45	23	-	
234 334 434	234 334 434	STAIR #-1	B	3'-0" X 7'-0"	1 3/4"	SC WD	PT	A	PT	90	22	-	PANIC DEVICE

DOOR HARDWARE SETS

SET #1 GUEST ROOM ENTRY DOOR:
1 1/2 PR BB HINGES
CARD LOCK - WING LEVER
DOOR CLOSER, 90° STOP
PERIMETER SEAL - PEMKO S773BL
THRESHOLD - PEMKO EV2325BL
DOOR SHOE - PEMKO 2113 DV
PRIVACY DR LATCH - PEMKO PDL
PEEP VIEWER - (2) @ ACC ROOMS

SET #2 BATHROOM DOOR:
1 HING
1 PR SPRING HINGES
PRIVACY LATCH SET
STONE THRESHOLD

SET #3 CONNECTING DOORS: (2 EA)
1 1/2 PR BB HINGES
PRIVACY LATCH
DEAD BOLT
PRIVACY PUSH BUTTON
PERIMETER SEAL - PEMKO S773BL
THRESHOLD - PEMKO ADJ232V8BL
DR SHOE - PEMKO 2113 AVUN
STOPS (WALL TYPE)

SET #4 BEDROOM DOOR:
SLIDING DR - PEMKO W80 BSS
RECESSED FINGER PULLS

SET #5
1 1/2 PR BB HINGES
PASSAGE SET
CLOSER
PERIMETER SEAL
THRESHOLD
DR SHOE

SET #6
1 ½ PR BB HINGES
STORE ROOM LOCK
DOOR CLOSER
PERIMETER SEAL
THRESHOLD
DOOR SHOE
KICK PLATE
WALL STOP

SET #7
1 ½ PR BB HINGES
PANIC BAR
DOOR CLOSER
PERIMETER SEAL
THRESHOLD
DOOR SHOE
KICK PLATE

SET #8
1 ½ PR BB HINGES
STORE ROOM LOCK
DOOR CLOSER
PERIMETER SEAL
THRESHOLD - ALUM.
DR SHOE
KICK PLATE

SET #9
1 1/2 PR BB HINGES
STORE ROOM LOCK
DOOR CLOSER
PERIMETER SEAL
THRESHOLD - ALUM.
DOOR SHOE - PEMKO 210 DV
KICK PLATE
FLUSH BOLT TOP & BOTTOM

SET #10
1 ½ PR BB HINGES
KEY PAD LOCK
DOOR CLOSER
PERIMETER SEAL
THRESHOLD
FLOOR STOP

SET #11
1 ½ PR BB HINGES
PRIVACY LOCK
DOOR CLOSER
SILENCERS
STONE THRESHOLD

SET #12
1 PR HINGES
LATCH SET
PERIMETER SEAL

SET #13
1 ½ PR BB HINGES
PASSAGE LOCK
DOOR CLOSER
PERIMETER SEAL
THRESHOLD
DOOR SHOE
KICK PLATE

SET #14
1 1/2 PR BB HINGES
KEY PAD LOCK
SILENCERS
DOOR CLOSER
WALL STOP

SET #15
1 1/2 PR BB HINGES
OFFICE LOCK
DOOR CLOSER
PERIMETER SEAL
FLOOR STOP

SET #16
1 1/2 PR BB HINGES
STORE ROOM LOCK
SILENCERS

SET #17
1 1/2 PR BB HINGES
PANIC BAR W/ LATCH SET
DOOR CLOSER
PERIMETER GASKET
THRESHOLD - ALUM
DOOR SHOE

SET #18
CARD READER
INTERCOM TO FRONT DESK

SET #19
1 1/2 PR BB HINGES
PRIVACY LOCK
DOOR CLOSER
PERIMETER SEAL
THRESHOLD - STONE
KICK PLATE

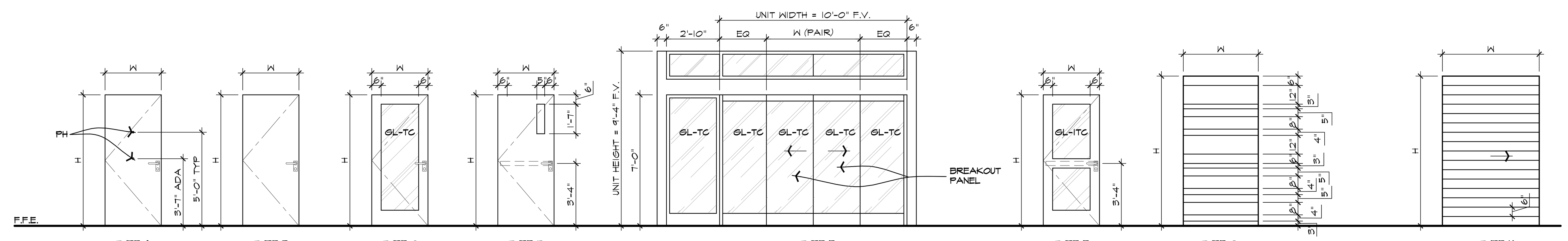
SET #20
1 1/2 PR BB HINGES
PANIC BAR W/ LATCH SET
KEY CARD READER
DOOR CLOSER
PERIMETER GASKET
THRESHOLD - ALUM
DOOR SHOE

SET #21
1 1/2 PR BB HINGES
KEY CARD LOCK
CLOSER
PERIMETER SEAL
WALL STOP
KICK PLATE

SET #22
1 1/2 PR BB HINGES
PANIC BAR / PASSAGE LATCH
DOOR CLOSER
PERIMETER STRIP
DOOR SHOE
THRESHOLD
WALL STOP

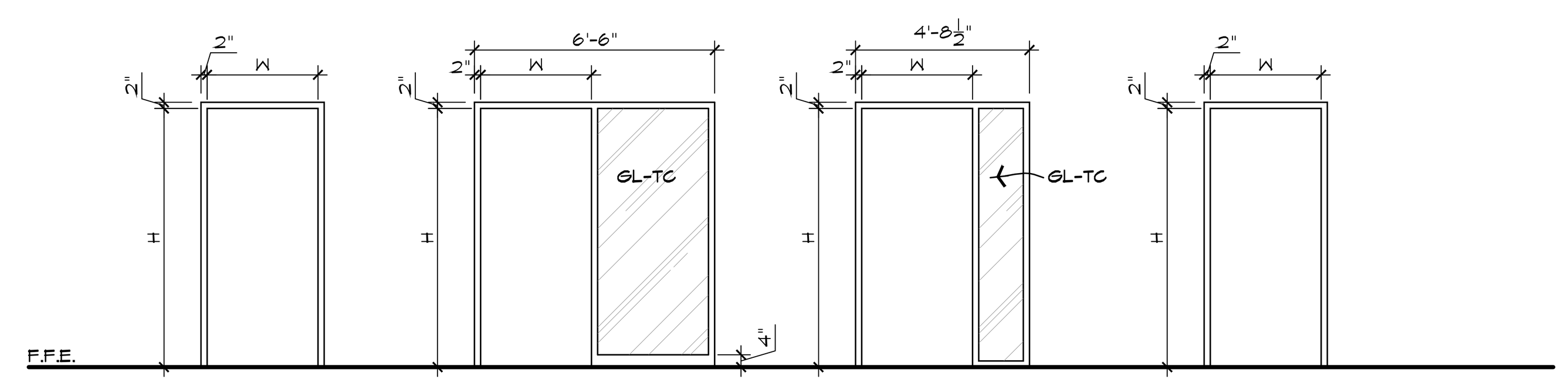
SET #23
1 ½ PR BB HINGES
KEY PAD LOCK
DOOR CLOSER
PERIMETER SEAL
THRESHOLD
DOOR SHOE
KICK PLATE

DOOR TYPES



TYPE A — FLUSH GUEST RM.
TYPE B — FLUSH DOOR
TYPE C — FLUSH W/ GL-TC
TYPE D — FLUSH W/ GL-TC
TYPE E — AUTOMATIC SLIDING
TYPE F — AL. W/ GL-ITC
TYPE G — T&G WD. TILT-UP
TYPE H — T&G WOOD FACED DOOR BOTH SIDES.

FRAME TYPES



TYPE A — HOLLOW METAL (HM)
TYPE B — HOLLOW METAL (HM)
TYPE C — ALUMINUM (AL)
TYPE D — ALUMINUM (AL)

DOOR SCHEDULE - GUEST ROOMS

TYPICAL ALL GUEST ROOMS, ALL FLOORS

DOOR #	ROOM #	ROOM NAME	DOOR TYPE	(NOMINAL) DOOR SIZE W X H	DOOR THICKNESS	DOOR		FRAME		FIRE RATED	HARDWARE SET	STC RATING	REMARKS
						MATERIAL	FINISH	MATERIAL	FINISH				
DA	-	GUEST - ENTRY	C	3'-0" x 7'-0"	1 3/4"	SC WD	PT	A	-	20 MIN	1	47	
DB	-	GUEST - BATHROOM	C	3'-0" x 7'-0"	1 3/4"	SC WD	PT	A	-	0	2		UNDER CUT DOOR BOTTOM 1"
DC	-	GUEST - CONNECTING	C	3'-0" x 7'-0"	1 3/4"	SC WD	PT	A	-	45 MIN	3	47	PROVIDE TWO DOORS, BACK TO BACK 180° SWING
DD	-	GUEST - BEDROOM	H	4-4" x 7'-0"	1 3/4"	SC WD	PT	WD	PT	0	4		SLIDING DOOR

DOOR ABBREVIATIONS

MATERIALS:
AL ALUMINUM
GL GLASS, SEE DOOR & WINDOW TYPES
GL-T GLASS - TEMPERED
GL-TC GLASS - TEMPERED CLEAR
GL-IT GLASS - 1" INSULATED TEMPERED
GL-ITC GLASS - 1" INSULATED TEMPERED CLEAR
GL-W WIRE GLASS
HM HOLLOW METAL
T TIMELY ALUMINUM FRAME
WD WOOD (SOLID CORE U.N.O.)
PH PEEP HOLE

FINISHES:
ANOD ANODIZED
CL CLEAR
EXP EXPOSED TO ABOVE
PC POWDER COAT
PT PAINT - COLOR TO BE SELECTED

OTHERS:
F.V. FIELD VERIFY
MFR PER MANUFACTURER

GENERAL NOTES

A. FIELD VERIFY ALL DOOR & WINDOW OPENING DIMENSIONS PRIOR TO FABRICATION.
B. DOOR & WINDOW ELEVATIONS VIEWED FROM EXTERIOR SIDE, U.N.O.
C. SEE FINISH LEGEND, SHEET A201 - 202 FOR EXTERIOR DOOR/WINDOW FRAMES COLORS & GLAZING COLORS.
D. ALL EXTERIOR GLAZING SHALL BE "LOW-E" TYPE, U.N.O.
E. ALL DOOR AND WINDOW HARDWARE REQUIREMENTS TO BE VERIFIED WITH THE OWNER AND ARCHITECT FOR OPERABLE FUNCTIONS PRIOR TO ACQUISITION.
F. ALL HOLLOW METAL DOORS AND METAL FRAMES TO RECEIVE PRIMER AND PAINT.
G. ALL DOOR LOCKS, LATCHES, LEVER TYPE HANDLES SHALL COMPLY WITH ALL ADA REQUIREMENTS.
H. LOCKS OR LATCHES SHALL COMPLY WITH THE IFC, THE IBC AND ANSI. EGRESS DOORS SHALL BE READILY OPENABLE FROM THE EGRESS SIDE WITHOUT A KEY OR SPECIAL KNOWLEDGE OR EFFORT.
I. ALL EXTERIOR DOORS SHALL HAVE TAMPER RESISTANT HINGES AND PICK RESISTANT LOCKS.
J. THRESHOLD HEIGHT @ DOORS SHALL NOT EXCEED ½" MAX.
K. "TOTAL DOOR", ELECTROMAGNETIC HOLD OPEN @ FIRE RATED ELEVATOR DOORS SHALL BE INTEGRATED WITH BUILDING FIRE ALARM SYSTEM.
L. DOOR TO REMAIN UNLOCK DURING DAYTIME BUSINESS HOURS. ACCESS FROM THE OUTSIDE DURING NIGHTTIME BETWEEN 10 PM TO 6 AM WILL REQUIRE CARD ACCESS. ACCESS FROM THE INSIDE DURING NIGHTTIME TO BE OPENABLE BY THE PANIC DEVICE.
M. ALL CARD ACCESS POINTS SHALL BE COMPLIED WITH THE REACH RANGES PER 2010 ADA STANDARD, SEE DTL. 14 & 15/G012.
N. ACCESS FROM THE OUTSIDE WILL REQUIRED CARD ACCESS AT ALL TIMES. ACCESS FROM THE INSIDE TO BE OPENABLE BY THE PANIC DEVICE.
P. SAFETY GLAZING SHALL BE TESTED IN ACCORDANCE W/ CPSC 16CFR 1021.
Q. FIRE DOOR W/ VISION PANEL ASSEMBLY SHALL BE PROVIDED W/ FIRE RATED SAFETY GLAZING. CONTRACTOR TO PROVIDE LISTING REPORTS TO THE INSPECTOR FOR THE DOOR ASSEMBLY.
R. 5" W X 1'-5" H, MAX. 85 SQ.IN. WIRED GLASS PANEL FOR 90 MIN. STAIR DOORS.
S. 7" W X 1'-10" H, MAX. 155 SQ.IN. WIRED GLASS PANEL FOR DOORS IN A 45 MIN. DOOR PANEL TYPE IN A 1 HOUR RATED WALL.
T. DOOR & SIDE LITE GLASS IN 1 HOUR RATED WALLS TO BE 45 MIN. RATED FIRELITE NT GLASS.
U. ALL GLASS IN INTERIOR DOORS AND SIDELIGHTS ARE TO BE TEMPERED OR LAMINATED SAFETY GLASS.
V. ALL GLASS IN EXTERIOR DOORS AND SIDELIGHTS ARE TO BE TEMPERED OR LAMINATED SAFETY GLASS.
W. ALL EXTERIOR FLUSH DOORS ARE TO BE INSULATED HOLLOW METAL.

GKI
GERALD R. KESLER, INC.
ARCHITECTS
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REGISTERED ARCHITECT CERTIFICATE NO. 13452 GERALD KESLER

INN CODE # 16711-PHXCT
1170 S. Arizona Ave.
Chandler, Arizona, 85286

Holiday Inn Express & Suites

JOB NUMBER
1401
DATE
07-24-2015
REVISION
1 — 1st City Comments 09-11-2015
2 — Elec Door Relo 01-20-2017
SHEET TITLE
DOOR SCHEDULE
SHEET NUMBER
A601

3042

ROOM FINISH SCHEDULE

ROOM #	ROOM NAME	FLOOR		BASE		WALL		CEILING		REMARKS
		MATERIAL	FINISH	MATERIAL	FINISH	MATERIAL	FINISH	MATERIAL	FINISH	
GROUND FLOOR										
100	STAIR #1-2	R	-	R	-	G.B.	PT	G.B.	PT	
100A	ELEC	CONC	S	R	-	G.B.	PT	G.B.	PT	
101	UTILITY	CONC	S	R	-	G.B.	PT	G.B.	PT	
101A	RISER	CONC	S	R	-	G.B.	PT	G.B.	PT	
103	LAUNDRY	VCT	W	R	-	G.B.	PT	G.B.	PT	
103A	UNISEX R.R.	VCT	W	R	-	G.B.	PT	G.B.	PT	
104	PANTRY	VCT	W	R	-	G.B.	PT	G.B.	PT	
105	EMPLOYEE	VCT	W	R	-	G.B.	PT	G.B.	PT	
106	BREAKFAST BAR	CT	-	WD	PT	G.B.	PT	G.B./ACT	PT/-	
107	WORK STATION	CPT	-	R	-	G.B.	PT	ACT	-	
107A	SALES	CPT	-	R	-	G.B.	PT	ACT	-	
107B	MANAGER	CPT	-	R	-	G.B.	PT	ACT	-	
107C	PBX	VCT	W	R	-	G.B.	PT	G.B.	PT	
107D	LUGGAGE	VCT	W	R	-	G.B.	PT	G.B.	PT	
108	MEETING	CPT	-	WD	PT	G.B.	PT	ACT	-	
109	RECEPTION	CPT/CT	-	WD	PT	G.B.	PT	G.B./ACT/WD	PT/-/-	
110	DINING	CPT/CT	-	WD	PT	G.B.	PT	G.B./ACT/WD	PT/-/-	
111	LOBBY	CPT/CT	-	WD	PT	G.B.	PT	G.B./ACT/WD	PT/-/-	
111A	VESTIBULE	CT	-	-	-	GL	-	G.B.	PT	
112	WOMEN R.R.	CT	-	CT	-	CT/VWC	-	G.B.	PT	
112A	MEN R.R.	CT	-	CT	-	CT/VWC	-	G.B.	PT	
112B	CORRIDOR	CPT	-	WD	PT	VWC	-	G.B./ACT	PT/-	
113	BUSINESS	CPT/CT	-	WD	PT	G.B.	PT	G.B./ACT/WD	PT/-/-	
114	ELEVATOR LOBBY	CT	-	WD	PT	VWC	-	G.B./ACT	PT/-	
114A	EXERCISE	R	-	R	-	VWC	-	ACT	-	
116A	GUEST LAUNDRY	CT	-	R	-	VWC	-	ACT	-	
134	STAIR #1-1	R	-	R	-	G.B.	PT	G.B.	PT	
135	CORRIDOR	CPT/CT	-	WD/CPT	PT/-	VWC	-	G.B./ACT	PT/-	
135A	CORRIDOR	CPT/CT	-	WD/CPT	PT/-	VWC	-	G.B./ACT	PT/-	
SECOND, THIRD, AND FOURTH FLOOR										NOTE: "#" INDICATED TYPICAL FLOOR LEVELS - 2nd, 3rd, 4th FLOOR
#00	STAIR #-2	R	-	R	-	G.B.	PT	G.B.	PT	
#00A	STORAGE	VCT	W	R	-	G.B.	PT	G.B.	PT	
#02A	CHUTE	VCT	W	R	-	G.B.	PT	G.B.	PT	
#14	ELEVATOR LOBBY	CPT	-	CPT	-	VWC	-	ACT.	-	
#14A	VENDING	CT	-	CT	-	VWC	-	ACT.	-	
#14B	ELEC	VCT	W	R	-	PT	-	G.B.	PT	
#16	HOUSEKEEPING	VCT	W	R	-	PT	-	G.B.	PT	
#34	STAIR #-1	R	-	R	-	G.B.	PT	G.B.	PT	
#35	CORRIDOR	R	-	R	-	G.B.	PT	G.B.	PT	

ROOM FINISH LEGEND

ABBRV.	MATERIAL	NOTES	DESCRIPTION
CONC	CONCRETE	-	
LWC	LIGHTWEIGHT CONCRETE	-	.
CPT	CARPET	-	TO BE SELECTED
CT	CERAMIC TILE	-	MISC SIZES, THIN SET
VCT	VINYL COMPOSITE TILES	-	COLOR TO BE SELECTED
R	RUBBER	-	COLOR TO BE SELECTED
WD	WOOD	-	FINISH AS DIRECTED
W	WAXED		
S	SEALED	-	.
PT	PAINT	SEE SPEC	.
G.B.	GYPSUM WALL BOARD	-	
VWC	VINYL WALL COVERING	-	.
ACT-1	ACOUSTIC CLG TILES IN CLG GRID	-	4' x 4' GRID
ACT-2	ACOUSTIC CLG TILES IN CLG GRID	-	TEGULAR EDGE, 2' x 2' THIN LINE GRID
ACT-3	ACOUSTIC CLG TILES IN CLG GRID	-	2' x 4' STANDARD GRID
WD-CLG	WOOD PANEL CEILING		SUSPENDED

ROOM FINISH NOTES

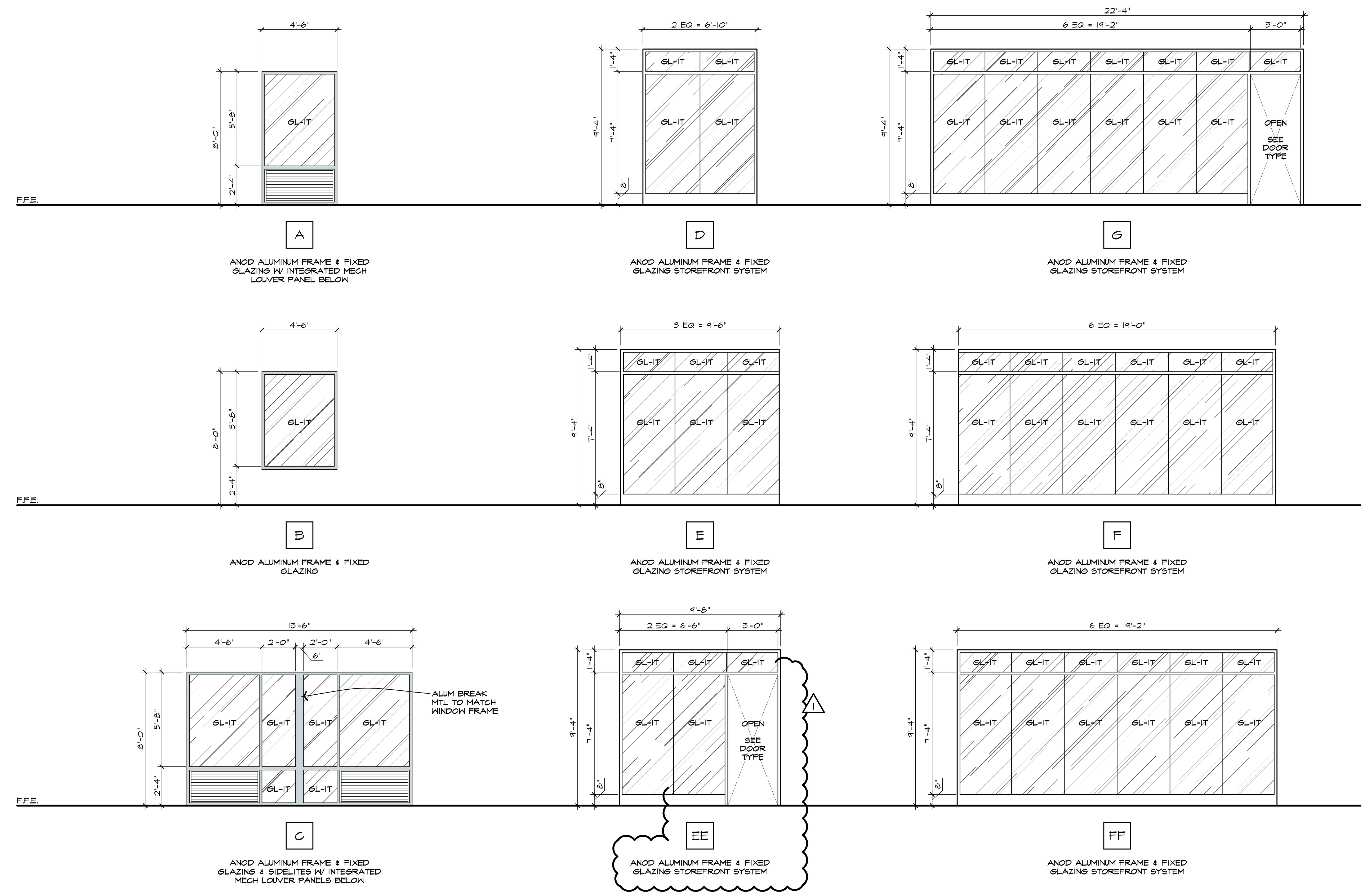
1. ALL ROOM FINISH MATERIALS SHALL BE SUPPLIED & INSTALLED BY THE CONTRACTOR, U.N.O.

GENERAL WINDOW NOTES

A. SEE ARCH. SPECS FOR ALL EXTERIOR GLAZING SYSTEMS AT WINDOWS AND STOREFRONTS.
B. DIMENSIONS SHOWN ARE FOR REFERENCE ONLY. CONTRACTOR SHALL FIELD VERIFY ALL WINDOW AND STOREFRONT OPENING DIMENSIONS PRIOR TO FABRICATIONS & INSTALLATIONS.
C. SEE ALSO WINDOW RELATED INFORMATION IN GENERAL NOTES ON DOOR SCHEDULE, SHEET A601.

WINDOW TYPES

NOTE: FIELD VERIFY ALL WINDOW OPENING DIMENSIONS PRIOR TO FABRICATION.



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REGISTERED ARCHITECT
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1401
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1 — 1st City Comments 09-11-2015

SHEET TITLE
ROOM FINISH & WINDOW SCHEDULES
SHEET NUMBER
A602

GENERAL STRUCTURAL NOTES

(APPLY UNLESS NOTED OTHERWISE)

GENERAL NOTES:

1. THE STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH THE ARCHITECTURAL, CIVIL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS, AND THE SPECIFICATIONS. THE CONTRACTOR SHALL VERIFY THE REQUIREMENTS OF OTHER TRADES AS TO SLEEVES, CHASES, HANGERS, INSERTS, ANCHORS, HOLES, AND ADDITIONAL ITEMS TO BE PLACED OR SET IN THE STRUCTURAL WORK.

2. THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE PROVISIONS OF THE INTERNATIONAL BUILDING CODE (IBC) 2012 EDITION WITH LOCAL AMENDMENTS.

3. THE WORK AS OUTLINED IN THE SPECIAL INSPECTION TABLES ON SHEET S061 IS SUBJECT TO SPECIAL INSPECTIONS AS DESCRIBED IN CHAPTER 17 OF THE INTERNATIONAL BUILDING CODE.

   THE LATERAL-LOAD-RESISTING SYSTEM FOR THIS STRUCTURE CONSISTS PRIMARILY OF THE FOLLOWING:

   WOOD DIAPHRAGMS AND LIGHT FRAMED SHEAR WALLS SHEATHED WITH STRUCTURAL PANELS OR OTHER MATERIALS.

   THESE ELEMENTS PROVIDE FOR LATERAL STRENGTH AND STABILITY IN THE COMPLETED STRUCTURE.

4. THE CONTRACTOR SHALL PROVIDE TEMPORARY SHORING AND BRACING REQUIRED TO ERECT AND HOLD THE STRUCTURE IN PROPER ALIGNMENT UNTIL PERMANENT SUPPORTS AND LATERAL BRACING ARE IN PLACE. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS, METHODS, TECHNIQUES, SEQUENCES FOR PROCEDURE OF CONSTRUCTION, OR THE SAFETY PRECAUTIONS.

5. LIVE LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE AS FOLLOWS:

   LIVE LOAD REDUCTION OF THE UNIFORMLY DISTRIBUTED FLOOR LIVE LOADS HAS BEEN UTILIZED IN CONFORMANCE WITH IBC.

   SLAB-ON-GRADE---------------------------------------100 PSF

   SUPPORTED FLOORS

   LOBBIES--100 PSF
   CORRIDOR---100 PSF
   STAIRWAY---100 PSF
   DWELLING UNITS--------------------------------------40 PSF
   STORAGE--125 PSF
   MECHANICAL SPACE-----------------------------------150 PSF
   ELEVATOR MACHINE ROOM-------------------------------50 PSF

   LIVE LOAD REDUCTION OF THE UNIFORMLY DISTRIBUTED ROOF LIVE LOADS HAS BEEN UTILIZED IN CONFORMANCE WITH IBC.

   ROOF--20 PSF

6. SUPERIMPOSED DEAD LOADS USED IN THE DESIGN OF THIS STRUCTURE ARE AS FOLLOWS:

   FLOOR JOISTS--30 PSF
   ROOF JOISTS---15 PSF

7. WIND DESIGN CRITERIA

   WIND LOAD (ULTIMATE)-------------------------------115 MPH
   WIND LOAD (NOMINAL)---------------------------------90 MPH
   WIND LOAD IMPORTANCE FACTOR (IW)-------------------1.00
   WIND EXPOSURE CATEGORY-----------------------------C
   RISK CATEGORY--------------------------------------II
   INTERNAL PRESSURE COEFFICIENT----------------------±0.18
   COMPONENT AND CLADDING PRESSURES:
   ROOF, ZONE 1---------------------------+16 PSF, 31 PSF
   ROOF, ZONE 2---------------------------+16 PSF, 44 PSF
   ROOF, ZONE 3---------------------------+16 PSF, 68 PSF
   WALLS, ZONE 4-------------------------------32 PSF
   WALLS, ZONE 5-------------------------------35 PSF
   PAPAPET, ZONE 4-----------------------------45 PSF
   PARAPET, ZONE 5-----------------------------60 PSF

8. SEISMIC DESIGN CRITERIA

   SEISMIC LOAD IMPORTANCE FACTOR(IE)-----------------1.0
   DESIGN SPECTRAL RESPONSE ACCELERATION
   SS......0.177g   S1......0.058g
   SDS.....0.189g   SD1.....0.093g
   RISK CATEGORY--------------------------------------II
   SEISMIC SOIL SITE CLASS----------------------------D
   RESPONSE MODIFICATION FACTOR (R)-------------------3.5
   DEFLECTION AMPLIFICATION FACTOR (Cd)---------------2.5
   SEISMIC RESPONSE COEFFICIENT (CS)------------------0.063
   ANALYSIS PROCEDURE------EQUIVALENT LATERAL FORCE PROCEDURE

   LATERAL LOADS INDICATED ARE WORKING STRESS LOADS

9. CONSTRUCTION MATERIAL LOADS SHALL BE PLACED SUCH THAT THE LOADS DO NOT EXCEED THE LIVE LOADS SPECIFIED ON THE CONTRACT DOCUMENTS.

10. SECTIONS AND DETAILS SHALL BE INCORPORATED AT ALL APPROPRIATE LOCATIONS, WHETHER SPECIFICALLY CUT OR NOT. TYPICAL DETAILS ARE NOT CUT ON PLANS, BUT SHALL APPLY AT ALL APPROPRIATE LOCATIONS.

11. THESE DOCUMENTS REPRESENT THE COMPLETED STRUCTURE. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE CONTRACTOR'S MEANS AND METHODS OR THE SAFETY PRECAUTIONS REQUIRED. NOR SHALL THE STRUCTURAL ENGINEERS INSPECTIONS OR OBSERVATIONS PERFORMED DURING CONSTRUCTION INCLUDE ANY RESPONSIBILITY FOR THESE ITEMS.

FOUNDATION NOTES:

1. FOUNDATIONS FOR THIS STRUCTURE HAVE BEEN DESIGNED IN ACCORDANCE WITH THE RECOMMENDATIONS IN THE GEOTECHNICAL EXPLORATION REPORT, PREPARED BY R.A.M.M., DATED JUNE 20, 2007. PROJECT NUMBER G14781.

2. FOUNDATIONS HAVE BEEN DESIGNED FOR A NET ALLOWABLE SOIL BEARING PRESSURE OF 2000 PSF.

3. FOUNDATIONS SHALL BEAR ON COMPACTED FILL MATERIAL 36" IN DEPTH. COMPACT EACH 8" LAYER OF BACKFILL OR FILL SOIL MATERIAL TO A MINIMUM OF 95% MODIFIED PROCTOR IN ACCORDANCE WITH ASTM D698. PRIOR TO PLACING FILL, CONTACT THE GEOTECHNICAL ENGINEER TO EVALUATE THE EXPOSED SUBGRADE.

4. THE MINIMUM DEPTH OF THE TOP OF FOOTINGS BELOW THE FINISHED GRADE SHALL BE 12".

5. REFER TO GEOTECHNICAL REPORT FOR SHORING & EXCAVATION REQUIREMENTS.

6. PRIOR TO PLACING FOUNDATION CONCRETE, ALL FOUNDATION EXCAVATIONS SHALL BE INSPECTED BY THE OWNER'S TESTING AGENCY TO EXPLORE THE EXTENT OF LOOSE, SOFT OR OTHERWISE UNSATISFACTORY SOIL MATERIAL AND TO VERIFY DESIGN BEARING PRESSURE. THE OWNER'S TESTING AGENCY SHALL REPORT FINDINGS TO THE STRUCTURAL ENGINEER OF RECORD AND DIRECTION FOR CORRECTIVE ACTION WILL BE PROVIDED WHERE REQUIRED.

7. NO UNBALANCED BACKFILL SHALL BE PLACED AGAINST MASONRY OR CONCRETE WALLS UNLESS WALLS ARE SECURELY BRACED AGAINST OVERTURNING, EITHER BY TEMPORARY CONSTRUCTION BRACING OR BY PERMANENT CONSTRUCTION.

8. PROVIDE A 2% MIN. SLOPE FOR PROPER DRAINAGE WITHIN 5'-0" OF THE BUILDING EXTERIOR TO ACCOUNT FOR SURFACE RUN-OFF OF THE BUILDING AND ADJACENT SURFACES.

9. REFER TO ARCHITECTURAL AND CIVIL DRAWINGS FOR EXTERIOR SLABS NOT SHOWN.

10. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR ANY GEOTECHNICAL ASPECTS OF THIS PROJECT. ALL INFORMATION PROVIDED WAS TAKEN FROM THE GEOTECHNICAL REPORT PROVIDED.

CAST-IN-PLACE CONCRETE NOTES:

1. ALL CONCRETE WORK SHALL CONFORM WITH THE REQUIREMENTS OF ACI 301 AND ACI 318.

2. ALL CONCRETE WORK SHALL COMPLY WITH THE FOLLOWING:

   CEMENT PER ASTM C150, TYPE II.
   AGGREGATE PER ASTM C33.
   CONCRETE SHALL BE NORMAL WEIGHT.
   CONCRETE SHALL BE READY MIXED IN ACCORDANCE WITH ASTM C94
   WATER / CEMENT RATIO = 0.5 MAX BY WEIGHT

3. CONCRETE SHALL BE NORMAL WEIGHT AND SHALL BE DESIGNED FOR A MINIMUM 28 DAY COMPRESSIVE STRENGTH AS FOLLOWS:

   SLABS ON GRADE ------------------------------------3,000 PSI*
   FOUNDATIONS ---------------------------------------3,000 PSI*

   *DESIGNED FOR 2500 PSI

4. FLY ASH IF PERMITTED BY ARCHITECT - SHALL CONFORM TO ASTM C618, CLASS F AND SHALL BE LIMITED TO 25% OF CEMENTITIOUS MATERIALS.

5. ALL CONCRETE SHALL HAVE A MAXIMUM SLUMP OF 4". IF A SUPER PLASTICIZER IS USED, AN 8" MAXIMUM SLUMP IS ALLOWED AT THE POINT OF PLACEMENT. ALL MIX DESIGNS SHALL BE DESIGNED BY THE CONCRETE PRODUCTION FACILITY IN ACCORDANCE WITH ACI 301. MIX DESIGNS FOR POST-TENSIONING CONCRETE SHALL BE PROPORTIONED SO AS TO MINIMIZE SHRINKAGE CRACKING.

6. ALL REINFORCING STEEL AND EMBEDDED ITEMS SUCH AS ANCHOR BOLTS AND WELD PLATES SHALL BE ACCURATELY PLACED IN THE POSITIONS SHOWN AND ADEQUATELY TIED AND SUPPORTED BEFORE CONCRETE IS PLACED TO PREVENT DISPLACEMENT BEYOND PERMITTED TOLERANCES.

7. MECHANICALLY VIBRATE ALL CONCRETE PER ACI.

8. ALL CONCRETE SLABS ON GRADE SHALL BE BOUND BY CONTROL JOINTS AS SHOWN ON THE FOUNDATION PLAN. KEYED CONTROL JOINTS ARE REQUIRED AT EXPOSED EDGES DURING POURING, AND SUCH THAT NO SINGLE POUR IS LARGER THAN 4000 SQUARE FEET WITHOUT APPROVAL, ALL OTHER JOINTS MAY BE SAW CUT. EXPANSION JOINT MATERIAL OF 1/2" IS REQUIRED WHEREVER THE SLAB ON GRADE ABUTS A VERTICAL SURFACE.

9. ALL CONCRETE PLACEMENT SHALL COMPLY WITH THE HOT AND COLD WEATHER REQUIREMENTS OF ACI UNLESS APPROVED BY THE ARCHITECT. ALL CONCRETE SHALL BE CURED IN ACCORDANCE WITH PER ACI 301.

10. ALL CONCRETE SHALL BE TESTED FOR COMPRESSIVE STRENGTH AND SLUMP PER ASTM C31, C39 AND C143. PROVIDE A MINIMUM OF 4 CYLINDERS FOR EACH DAY'S PLACEMENT OR FOR EVERY 50 CUBIC YARDS, WHICHEVER IS GREATER. A QUALIFIED TESTING LABORATORY SHALL TEST ONE CYLINDER AT 7 DAYS AND TWO AT 28 DAYS AND SHALL HAVE 1 ADDITIONAL CYLINDER AS A SPARE.

REINFORCING STEEL:

1. REINFORCING STEEL SHALL CONFORM TO THE LATEST ACI AND CRSI DETAILING MANUAL AND THE FOLLOWING:

   #4 AND LARGER DEFORMED BARS----------ASTM A615 (Fy = 60 KSI)
   #3 AND SMALLER DEFORMED BARS---------ASTM A615 (Fy = 40 KSI)
   ALL WELDED REINFORCING BARS------------------------ASTM A706 WELDED
   WIRE FABRIC-------------------------------ASTM A185
   WIRE---ASTM A82

2. ACCURATELY SUPPORT ALL REINFORCING, INCLUDING WELDED WIRE FABRIC, WITH GALVANIZED METAL CHAIRS, SPACERS OR HANGERS FOR THE FOLLOWING CLEAR CONCRETE COVERAGES UNO:
   CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH ----------- 3"
   EXPOSED TO EARTH OR WEATHER
   #6 OR LARGER -- 2"
   #5 AND SMALLER ------------------------------------ 1 1/2"
   COLUMNS (TO TIES) --------------------------------- 1 1/2"

   ALL OTHER CLEARANCES SHALL COMPLY WITH THE LATEST EDITION OF ACI 318.

3. LAP SPLICES, UNLESS NOTED OTHERWISE, SHALL BE CLASS "B" TENSION LAP SPLICES PER LATEST EDITION OF ACI 318. LAP SPLICES IN CONCRETE COLUMNS SHALL BE STANDARD COMPRESSION LAP SPLICES. STAGGER SPLICES A MINIMUM OF ONE LAP LENGTH.

4. LAP SPLICES IN WELDED WIRE FABRIC SHALL BE A MINIMUM OF 8". REMOVE CROSS LINK OF WELDED WIRE FABRIC AT EACH SAW JOINT.

5. ALL SPLICE LOCATIONS SUBJECT TO APPROVAL BY THE STRUCTURAL ENGINEER. SPLICED BARS SHALL BE PLACED AT THE SAME EFFECTIVE DEPTH U.N.O. ALL REINFORCING NOTED AS "CONTINUOUS" SHALL BE FULLY CONTINUOUS AND SPLICED. PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZONTAL BARS AT ALL CORNERS AND INTERSECTIONS PER TYPICAL DETAILS.

6. REINFORCING BAR SPACING GIVEN ARE MAXIMUM ON CENTERS. SKEW HOOKS AS REQUIRED TO MAINTAIN CONCRETE COVER. SECURELY TIE ALL BARS IN PLACE PRIOR TO PLACEMENT OF CONCRETE.

EPOXY BOLTS OR DOWELS AND EXPANSION BOLTS:

1. EPOXY DOWELS IN CONCRETE SHALL BE REINFORCING STEEL INSTALLED WITH ONE OF THE FOLLOWING:
   a. HILTI HIT-RE 500-SD ADHESIVE ANCHOR (ICC-ES REPORT ESR-2322)
   b. SIMPSON STRONG-TIE SET-EX EPOXY ADHESIVE (ICC-ES REPORT ESR-2508)
   c. POWERS PE 1000+ EPOXY ADHESIVE (ICC-ES REPORT ESR-2583)

2. EPOXY BOLTS IN CONCRETE SHALL BE A THREADED ROD INSTALLED WITH ONE OF THE FOLLOWING:
   a. HILTI HIT-RE 500-SD ADHESIVE ANCHOR (ICC-ES REPORT ESR-2322)
   b. SIMPSON STRONG-TIE SET-EX EPOXY ADHESIVE (ICC-ES REPORT ESR-2508)
   c. POWERS PE 1000+ EPOXY ADHESIVE (ICC-ES REPORT ESR-2583)

3. EXPANSION BOLTS IN CONCRETE SHALL BE ONE OF THE FOLLOWING:
   a. HILTI HSL-3 CARBON STEEL HEAVY DUTY EXPANSION ANCHOR (ICC-ES REPORT ESR-1545)
   b. HILTI HDA CARBON AND STAINLESS STEEL UNDERCUT ANCHOR (ICC-ES REPORT ESR-1546)
   c. HILTI KWIK BOLT TZ CARBON AND STAINLESS STEEL ANCHORS (ICC-ES REPORT ESR-1917)
   d. POWERS POWER-STUD+SD2 ANCHOR (ICC-ES REPORT ESR-2502)
   e. POWERS POWER-STUD+SD1 (ICC-ES REPORT ESR-2818)
   f. SIMPSON STRONG-TIE STRONG-BOLT ANCHOR (ICC-ES REPORT ESR-1771)

4. THE CONTRACTOR MAY NOT USE SUBSTITUTES FOR EPOXY OR EXPANSION ANCHORS WITHOUT PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.

5. FOR MINIMUM EMBEDMENT LENGTH SEE TYPICAL POST-INSTALLED ANCHOR AND REINFORCING SCHEDULE. INSTALL ALL BOLTS AS OUTLINED IN MANUFACTURER'S SPECIFICATIONS INCLUDING UTILIZING PROPER SIZE AND TYPE OF DRILL, CLEANING HOLE, DRIVING AND TIGHTENING BOLT.

STRUCTURAL STEEL NOTES:

1. ALL STEEL CONSTRUCTION SHALL COMPLY WITH THE LATEST EDITION OF THE AISC HANDBOOK. THIS STRUCTURE WAS DESIGNED USING THE MANUAL OF STEEL CONSTRUCTION- ALLOWABLE STRESS DESIGN.

2. STRUCTURAL STEEL SHALL COMPLY WITH THE FOLLOWING SPECIFICATIONS:

   STRUCTURAL STEEL SHAPES, PLATES AND BARS (EXCEPT W-SHAPES) ASTM A 36, Fy =36 KSI
   STRUCTURAL STEEL W-SHAPES - ASTM A 992/A 572, GRADE 50, Fy = 50 KSI
   HOLLOW STRUCTURAL SHAPES (HSS)
   ASTM A 500, GRADE B, Fy = 46 KSI FOR SQUARE AND RECTANGULAR SHAPES
   ANCHOR RODS - ASTM F 1554, GRADE 36
   HIGH-STRENGTH BOLTS - ASTM A325
   WASHERS - ASTM F 436
   NUTS - ASTM A 563

3. UNLESS OTHERWISE NOTED, ALL CONNECTIONS SHALL BE AISC TYPE 2 "STANDARD FRAMED BEAM CONNECTIONS" WITH ASTM A 325 BOLTS.

4. HIGH STRENGTH BOLTS MAY BE TIGHTENED TO THE "SNUG TIGHT" CONDITION IN LIEU OF FULL PRETENSIONING.

5. HIGH STRENGTH BOLTS SHALL BE INSTALLED WITH LOAD INDICATOR WASHERS.

6. WELDING SHALL BE IN ACCORDANCE WITH AWS D1.1, "STRUCTURAL WELDING CODE - STEEL". WELD ELECTRODES SHALL BE E70XX - LOW HYDROGEN OR EQUIVALENT. UNLESS OTHERWISE NOTED, PROVIDE CONTINUOUS FILLET WELDS WITH MINIMUM SIZE REQUIRED BY TABLE J2.4 OF THE "MANUAL OF STEEL CONSTRUCTION - ALLOWABLE STRESS DESIGN". ALL WELDING SHALL BE PERFORMED BY WELDERS HOLDING VALID CERTIFICATES AND HAVING CURRENT EXPERIENCE IN THE TYPE OF WELD SHOWN ON THE DRAWINGS OR NOTES. CERTIFICATES SHALL BE THOSE ISSUED BY AN ACCEPTED TESTING AGENCY.

7. ALL STEEL EXPOSED TO WEATHER (INCLUDING LINTELS IN EXTERIOR WALLS) SHALL BE HOT DIPPED GALVANIZED.

8. SHOP PAINT ALL STEEL SURFACES WITH FABRICATOR'S STANDARD RUST-INHIBITING PRIMER EXCEPT AT SURFACES ENCASED IN CONCRETE, SURFACES TO RECEIVE FIREPROOFING, OR SURFACES ENCLOSED WITHIN THE BUILDING FINISHES. BEAMS, COLUMNS AND BRACES SHALL NOT BE SPLICED WITHOUT THE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER.

9. NON-SHRINK GROUT SHALL BE 5,000 PSI, FIVE STAR, SIKA 212 OR AN APPROVED EQUIVALENT. INSTALL NON-SHRINK GROUT UNDER BEARING PLATES BEFORE FRAMING MEMBER IS INSTALLED. AT COLUMNS, INSTALL NON-SHRINK GROUT UNDER BASEPLATES AFTER COLUMN HAS BEEN PLUMBED BUT PRIOR TO FLOOR OR ROOF INSTALLATION.

ROUGH CARPENTRY NOTES:

1. ROUGH CARPENTRY FOR THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE LATEST VERSION OF WFCM SPECIFICATIONS FOR WOOD CONSTRUCTION."

2. ALL FRAMING PER SHALL CONFORM TO THE LATEST EDITION OF THE IBC. MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 19%. ALL SAWN LUMBER SHALL BE STAMPED WITH THE GRADE MARK OF AN APPROVED LUMBER GRADING AGENCY. ALL SAWN LUMBER SHALL BE DOUGLAS FIR-LARCH WITH THE FOLLOWING MINIMUM PROPERTIES U.N.O.:

   JOISTS
   2 X 4 OR LARGER------------------------------ D.F. #2

   BEAMS, LEDGERS AND TOP PLATES
   WIDTH 4" OR LESS ---------------------------- D.F. #2
   WIDTH GREATER THAN 4" ----------------------- D.F. #1

   STUDS, PLATES AND BLOCKING
   2 X 4 OR LARGER------------------------------ D.F. #1

   POSTS
   4 X 4 OR LARGER------------------------------ D.F. #2
   6 X 6 OR LARGER ----------------------------- D.F. #1

3. ALL PLYWOOD SHALL CONFORM TO PS-1 OR APA PRP-108 AND SHALL HAVE AN EXTERIOR OR EXPOSURE 1 CLASSIFICATION AND BEAR THE STAMP OF AN APPROVED TESTING AGENCY. ALL NAILS ARE COMMON NAILS UNLESS OTHERWISE NOTED. WHERE SCREWS ARE INDICATED FOR WOOD TO WOOD ATTACHMENTS, USE WOOD SCREWS. ALL PLYWOOD SHALL BE OF THE FOLLOWING NOMINAL THICKNESS, SPAN/INDEX RATIO AND SHALL BE ATTACHED AS FOLLOWS UNLESS NOTED OTHERWISE:

USE	THICK	SPAN/INDEX RATIO	EDGE ATTACHMENT	INTERMEDIATE ATTACHMENT
ROOF	5/8"	20/40	10d@4"O.C.	10d AT 12" O.C.
FLOOR	3/4" T & G	48/24	SCREWS@4"O.C.	SCREWS AT 12" O.C.
WALL	1/2"	32/16	10d@6"O.C.	10d@12"O.C.
SHEAR WALL	SEE SCHEDULE			

4. SCREWS AT FLOOR SHEATHING SHALL BE #8 X 2 1/2" LONG FOR SHEATHING LESS THAN 1" THICK. ALL FLOOR SHEATHING SHALL BE GLUED TO JOISTS WITH AN APA AFG-01 QUALIFIED GLUE.

5. ALL WOOD FRAMING MEMBERS PERMANENTLY EXPOSED TO THE WEATHER AND ALL SILL PLATES AROUND THE BUILDING PERIMETER SHALL BE PRESERVATIVE-TREATED.

6. STEEL PLATE CONNECTORS SHALL COMPLY WITH ASTM A36 SPECIFICATIONS. BOLTS CONNECTING WOOD MEMBERS SHALL COMPLY WITH ASTM A 307 COMMON STEEL BOLTS, AND SHALL BE 3/4" DIAMETER UNLESS OTHERWISE SPECIFIED.

7. METAL FRAMING ANCHORS SHALL COMPLY WITH ASTM A 653 GRADE A STRUCTURAL QUALITY. METAL FRAMING ANCHORS SHALL BE AS MANUFACTURED BY SIMPSON STRONG-TIE COMPANY, INC. OR OTHER MANUFACTURER WITH CURRENT AND EQUIVALENT ICC APPROVAL.

8. PROVIDE BRIDGING FOR ALL FLOOR AND ROOF JOISTS. MAXIMUM SPACING SHALL BE 8'-0" UNLESS OTHERWISE NOTED.

9. PROVIDE DOUBLE JOISTS UNDER ALL PARTITIONS WHICH RUN PARALLEL WITH JOISTS, AND UNDER ALL CONCENTRATED LOADS FROM FLOORS ABOVE. PROVIDE MULTIPLE STUDS WHERE INDICATED ON THE PLANS.

10. PROVIDE HEADERS OF THE SAME CROSS SECTION AS JOISTS OR RAFTERS TO FRAME AROUND ALL OPENINGS TO SUPPORT SHEATHING UNLESS OTHERWISE NOTED OR DETAILED ON THE DRAWINGS.

11. UNLESS OTHERWISE NOTED, ATTACH BLOCKING AND NAILERS TO STEEL FRAMING USING 3/16" DIAMETER POWDER ACTUATED FASTENERS AT 24" ON CENTER OR 1/2" DIAMETER BOLTS AT 48" ON CENTER. STAGGER FASTENERS TO ALTERNATE SIDES OF BEAM WEB.

12. WHERE MULTIPLE FRAMING MEMBERS ARE INDICATED, SCAB CONTINGENT MEMBERS TOGETHER WITH 16d NAILS AT 12" ON CENTER, ALTERNATING AT 2 INCHES FROM EACH EDGE.

13. AMERICAN PLYWOOD ASSOCIATION PERFORMANCE RATED SHEATHING MAY BE USED AS AN ALTERNATE TO PLYWOOD WITH PRIOR WRITTEN APPROVAL OF ARCHITECT. WHERE ROOF IS TO BE GUARANTEED, IT MAY NOT BE USED WITHOUT PRIOR APPROVAL FROM BUILT-UP ROOF SYSTEM MANUFACTURER. RATED SHEATHING SHALL COMPLY WITH ICC, HAVE AN EXTERIOR OR EXPOSURE 1 CLASSIFICATION, AND SHALL HAVE A SPAN RATING EQUIVALENT TO OR BETTER THAN THE PLYWOOD IT REPLACES. ATTACHMENT AND THICKNESS (WITHIN 1/32") SHALL BE THE SAME AS THE PLYWOOD IT REPLACES.

14. STUD WALLS SHALL BE 2 X 6 AT 16" O.C. U.N.O. ON PLANS. CONNECT BOTTOM PLATE AT CONCRETE FOUNDATIONS WITH 1/2" DIAMETER ANCHOR BOLTS PLACED AT 4'-0" O.C. MAXIMUM U.N.O. ANCHOR BOLTS SHALL BE PLACED AT ALL JAMBS, CORNERS, INTERSECTIONS AND ENDS OF WALLS.

15. ALL NAILING NOT NOTED SHALL BE ACCORDING TO TABLE 2304.9.1 OF THE INTERNATIONAL BUILDING CODE. ALL NAILS SHALL BE WITH COMMON NAILS.

STRUCTURAL GLUED LAMINATED LUMBER NOTES:

1. STRUCTURAL GLUED LAMINATED UNITS FOR THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AMERICAN INSTITUTE OF TIMBER CONSTRUCTION (AITC) "STRUCTURAL GLUED LAMINATED TIMBER" AITC A 190.1.

2. GLUED-LAMINATED BEAMS SHALL BE DOUGLAS FIR LARCH WITH THE FOLLOWING MINIMUM PROPERTIES: Fb = 2,400 PSI, Fv = 240 PSI, Fc (PERPENDICULAR) = 650 PSI, E =1,800,000 PSI. BEAMS CANTILEVERING OVER SUPPORTS SHALL HAVE THE SPECIFIED MINIMUM PROPERTIES TOP AND BOTTOM.

3. CAMBER AS SHOWN ON DRAWINGS. IF NO CAMBER IS SPECIFIED, PROVIDE STANDARD CAMBER USING A RADIUS OF 3500 FEET.

4. ALL BEAMS SHALL BE FABRICATED USING WATERPROOF GLUE. FABRICATION AND HANDLING PER LATEST AITC AND WCLA STANDARDS. BEAMS SHALL BEAR GRADE STAMP AND AITC STAMP AND CERTIFICATE. APPEARANCE GRADE BEAMS SHALL BE USED IF INDICATED ON ARCHITECTURAL DRAWINGS.

5. STEEL PLATE CONNECTORS SHALL COMPLY WITH ASTM A36 SPECIFICATIONS. BOLTS CONNECTING WOOD MEMBERS SHALL COMPLY WITH ASTM A 307 COMMON STEEL BOLTS WITH NUTS AND WASHERS.

WOOD STAIRS:

1. WOOD STAIRS SHALL BE ASSEMBLED AND FABRICATED PER IBC. MAXIMUM MOISTURE CONTENT SHALL NOT EXCEED 19%.

2. WOOD STAIR STRINGERS SHALL COMPLY WITH ONE OF THE FOLLOWING MINIMUM SIZE AND SPECIES:

   ENGINEERED WOOD STRINGERS: SHALL HAVE A MINIMUM Fb = 2730 PSI AND MINIMUM MODULUS OF ELASTICITY OF 1,800,000 PSI.

3. MAXIMUM NOTCH MEASURED PERPENDICULAR TO THE STRINGER PLANE SHALL NOT EXCEED 5 1/2" FOR THE STRINGER SPANS SHOWN ABOVE.

4. STAIR TREADS SHALL COMPLY WITH ONE OF THE FOLLOWING MINIMUM PROPERTIES:

   3/4" PLYWOOD 48/24 SPAN RATING
   1X DOUGLAS FIR #2 FLAT WOOD DECKING.

5. ATTACHMENT TO ADJACENT WOOD WALLS SHALL BE PER TYPICAL DETAILS. COORDINATE ATTACHMENT WITH ARCHITECTURAL FOR SOUND TRANSMISSION PRIOR TO CONNECTING STAIR STRINGERS TO ADJACENT WALLS.

6. FOR STAIR DIMENSIONS, INCLUDING RISE AND RUN AND NON-SLIP SURFACES, SEE ARCHITECTURAL DRAWINGS.

WOOD I-JOIST (TJI TYPE):

1. THE JOIST MANUFACTURER SHALL BE RESPONSIBLE FOR THE COMPLETE DESIGN, FABRICATION AND ERECTION PROCEDURES OF ALL JOISTS, BRIDGING AND/OR BLOCKING PANELS, ETC. FOR A COMPLETE INSTALLATION OF THE JOIST SYSTEM. ALL JOISTS AND CONNECTORS SHALL HAVE A CURRENT ICC APPROVAL. CONNECTIONS AND BEARING MATERIAL THAT ARE SHOP CONNECTED TO JOISTS SHALL BE DESIGNED BY JOIST FABRICATOR.

2. JOIST SIZES ARE INDICATED ON PLANS AND SCHEDULES.

   THE UNIFORM LOADS DO NOT INCLUDE SPECIAL OR ADDITIONAL LOADS NOTED ON THE PLANS OR DETAILS.

3. FOR ROOF JOISTS LIMIT TOTAL LOAD DEFLECTIONS TO SPAN/180 AT SIMPLE SPANS U.N.O. LIMIT LIVE LOAD DEFLECTIONS TO SPAN/240 AT SIMPLE SPANS U.N.O. FOR FLOOR JOISTS LIMIT TOTAL LOAD DEFLECTIONS TO SPAN/240 AT SIMPLE SPANS U.N.O. LIMIT LIVE LOAD DEFLECTIONS TO SPAN/360 AT SIMPLE SPANS U.N.O.

4. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS PREPARED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF ARIZONA FOR THE DESIGN OF WOOD CHORD OR STEEL WEB JOISTS INCLUDING DESIGN LOADINGS AND REACTIONS APPLIED TO THE SUPPORTING STRUCTURE.

5. ADDITIONAL JOISTS SHALL BE SUPPLIED AS REQUIRED TO SUPPORT ADDITIONAL AND SPECIAL LOADS INCLUDING MECHANICAL EQUIPMENT. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS.

6. ALL FABRICATION SHALL BE PERFORMED ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVED TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION.

PREFABRICATED METAL-PLATE-CONNECTED WOOD TRUSS NOTES:

1. PREFABRICATED METAL-PLATE-CONNECTED WOOD TRUSSES FOR THIS STRUCTURE SHALL BE DESIGNED IN ACCORDANCE WITH THE NATIONAL FOREST PRODUCTS ASSOCIATION (NFPA) "NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION", THE TRUSS PLATE INSTITUTE (TPI) AND THE "DESIGN SPECIFICATION FOR METAL-PLATE-CONNECTED WOOD TRUSSES".

2. THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS PREPARED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF ARIZONA FOR THE DESIGN OF PREFABRICATED METAL-PLATE-CONNECTED WOOD TRUSSES, INCLUDING DESIGN LOADINGS AND REACTIONS APPLIED TO THE SUPPORTING STRUCTURE. SECONDARY BENDING STRESSES IN TRUSS TOP AND BOTTOM CHORDS DUE TO LOADS SHALL BE CONSIDERED IN THE DESIGN.

3. WOOD TRUSS FRAMING MEMBERS SHALL COMPLY WITH PS 20 "AMERICAN SOFTWOOD LUMBER STANDARD" AND THE FOLLOWING MINIMUM REQUIREMENTS:

   MOISTURE CONTENT - SEASONED, WITH 19 PERCENT.
   GRADE - NO. 2.
   SPECIES - DOUGLAS FIR.

4. METAL FRAMING ANCHORS SHALL COMPLY WITH ASTM A 653 GRADE A STRUCTURAL QUALITY.

5. WOOD TRUSS DESIGN LOADS SHALL BE AS FOLLOWS:
   TOP CHORD LOADING:
   LIVE LOAD = AS INDICATED IN "GENERAL NOTES"
   DEAD LOAD = 9 PSF
   WIND LOAD = AS INDICATED IN "GENERAL NOTES"

   BOTTOM CHORD LOADING:
   LIVE LOAD = 0 P.S.F. DEAD LOAD = 6 PSF

6. FOR ROOF TRUSSES, LIMIT TOTAL LOAD DEFLECTIONS TO SPAN/240 AT SIMPLE SPANS U.N.O. LIMIT LIVE LOAD DEFLECTIONS TO SPAN/360 AT SIMPLE SPANS U.N.O. FOR FLOOR TRUSSES, LIMIT TOTAL LOAD DEFLECTIONS TO SPAN/360 AT SIMPLE SPANS U.N.O. LIMIT LIVE LOAD DEFLECTIONS TO SPAN/480 AT SIMPLE SPANS U.N.O. ALL TRUSSES SHALL BE CAMBERED FOR 0.8 TIMES THE DESIGN DEAD LOAD UNLESS THE CAMBER WOULD BE LESS THAN 3/4".

7. ALL TRUSS TO TRUSS CONNECTORS SHALL BE DESIGNED BY THE TRUSS MANUFACTURER AND SHALL HAVE CURRENT ICC APPROVAL. MULTIPLE TRUSS MEMBERS SHALL BE FASTENED TOGETHER TO ALLOW TRANSFER OF SHEAR AND TENSION FORCES (MINIMUM 200 PLF) AT PLYWOOD SHEATHING JOINTS AND TO PREVENT CROSS GRAIN BENDING OF TOP CHORDS.

8. IN ADDITION TO THE TRUSS BRACING SHOWN, THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY AND PERMANENT BRACING AS REQUIRED FOR SAFE ERECTION OF THE TRUSSES, OR AS RECOMMENDED BY THE MANUFACTURER. THE GUIDELINES SET FORTH IN THE TRUSS PLATE INSTITUTE PUBLICATION "BRACING WOOD TRUSSES, COMMENTARY AND RECOMMENDATIONS" SHALL BE CONSIDERED AS MINIMUM REQUIREMENTS.

9. TRUSS MANUFACTURER MAY USE ALTERNATIVE TRUSS WEB CONFIGURATIONS SUBJECT TO APPROVAL OF THE ENGINEER OF RECORD.

SHOP DRAWINGS:

1. SHOP DRAWINGS SHALL BE SUBMITTED FOR ALL STRUCTURAL ITEMS IN ADDITION TO ITEMS REQUIRED BY ARCHITECTURAL SPECIFICATIONS. CONSTRUCTION DOCUMENTS SHALL NOT BE REPRODUCED FOR USE AS SHOP DRAWINGS. NO MORE THAN THREE SETS OF BLUELINES AND ONE SET OF REPRODUCIBLES WILL BE REVIEWED FOR ANY SUBMITTAL.

2. THE CONTRACTOR SHALL REVIEW ALL SHOP DRAWINGS PRIOR TO SUBMITTAL. ITEMS NOT IN ACCORDANCE WITH CONTRACT DOCUMENTS SHALL BE FLAGGED UPON HIS REVIEW. VERIFY ALL DIMENSIONS WITH ARCHITECT.

3. ANY CHANGES, SUBSTITUTIONS, OR DEVIATIONS FROM CONTRACT DOCUMENTS SHALL BE CLOUDED BY MANUFACTURER OR FABRICATOR. ANY ITEMS WHICH ARE NOT CLOUDED OR FLAGGED BY SUBMITTING PARTIES, SHALL NOT BE CONSIDERED APPROVED UNLESS NOTED ACCORDINGLY BY THE ENGINEER OF RECORD.

4. THE SHOP DRAWINGS DO NOT REPLACE THE CONTRACT DOCUMENTS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO MAKE SURE ITEMS ARE CONSTRUCTED TO CONTRACT DOCUMENTS.

5. THE ADEQUACY OF ENGINEERING DESIGNS AND LAYOUT PERFORMED BY OTHERS RESTS WITH THE DESIGNING OR SUBMITTING AUTHORITY.

6. REVIEWING IS INTENDED ONLY AS AN AID TO THE CONTRACTOR IN OBTAINING CORRECT SHOP DRAWINGS. RESPONSIBILITY FOR CORRECTNESS AND COMPLETENESS SHALL REST WITH THE CONTRACTOR.

7. ALLOW FIVE WORKING DAYS FOR REVIEW OF SHOP DRAWINGS BY THE STRUCTURAL ENGINEER.

DEFERRED SUBMITTALS: (PER 2009 IBC 106.3.4.2)

1. FOR THE PURPOSES OF THIS SECTION, DEFERRED SUBMITTALS ARE DEFINED AS THOSE PORTIONS OF THE DESIGN THAT ARE NOT SUBMITTED AT THE TIME OF THE APPLICATION AND WHICH ARE TO BE SUBMITTED TO THE BUILDING OFFICIAL WITHIN A SPECIFIED PERIOD.

2. DEFERRAL OF ANY SUBMITTAL ITEMS SHALL HAVE PRIOR APPROVAL OF THE BUILDING OFFICIAL. THE CONTRACTOR SHALL SUBMIT THE DEFERRED SUBMITTAL DOCUMENTS FOR REVIEW BY THE BUILDING OFFICIAL AND THE ENGINEER OF RECORD.

3. SUBMITTAL DOCUMENTS FOR DEFERRED SUBMITTAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT OR ENGINEER OF RECORD IN A ENOUGH TIME FOR REVIEW PRIOR TO FABRICATION. THE DOCUMENTS SHALL BE REVIEWED FOR GENERAL CONFORMANCE WITH THE DRAWINGS. A COPY OF THE DEFERRED SUBMITTAL DOCUMENTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL WITH A NOTATION INDICATING THAT THE DEFERRED SUBMITTAL DOCUMENTS HAVE BEEN REVIEWED. THE DEFERRED SUBMITTAL ITEMS SHALL NOT BE INSTALLED UNTIL THEIR DESIGN AND SUBMITTAL DOCUMENTS HAVE BEEN APPROVED BY THE BUILDING OFFICIAL.

4. DEFERRED SUBMITTAL ITEMS:

   PREFABRICATED METAL-PLATE CONNECTED WOOD TRUSSES
   WOOD I-JOIST

SPECIAL INSPECTION:

1. PER IBC CHAPTER 17, SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING ITEMS:

   SOILS: PER IBC SECTION 1704.7

   CONCRETE CONSTRUCTION: PER IBC SECTION 1704.4 & TABLE 1704.4. NOT REQUIRED FOR SLABS ON GRADE.

   STEEL CONSTRUCTION: PER IBC SECTION 1704.3 AND TABLE 1704.3 INCLUDING WELDING AND HIGH STRENGTH BOLTING.

   MASONRY CONSTRUCTION: PER IBC SECTION 1704.5 AND TABLE 1704.5.1

   EPOXY AND EXPANSION ANCHORS: REVIEW INSTALLATION PROCEDURES PER SPECIFIED ICC REPORT.

   DUTIES AND RESPONSIBILITIES OF THE SPECIAL INSPECTOR:

   THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DESIGN DRAWINGS AND SPECIFICATION. THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL IF REQUESTED, AND TO THE ENGINEER OR ARCHITECT OF RECORD. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE DESIGN AUTHORITY AND THE BUILDING OFFICIAL. UPON COMPLETION OF THE ASSIGNED WORK THE ENGINEER OR ARCHITECT SHALL COMPLETE AND SIGN THE APPROPRIATE FORMS CERTIFYING THAT TO THE BEST OF HIS KNOWLEDGE THE WORK IS IN CONFORMANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CODE.

   IBC SPECIAL INSPECTION TABLES ARE INCLUDED FOR REFERENCE ON SHEET S061.

STRUCTURAL OBSERVATION:

1. PER IBC CHAPTER 17, STRUCTURAL OBSERVATION IS REQUIRED FOR THIS PROJECT. STRUCTURAL OBSERVATION IS THE VISUAL OBSERVATION OF THE ELEMENTS AND CONNECTION OF THE STRUCTURAL SYSTEM AT SIGNIFICANT CONSTRUCTION STAGES AND THE COMPLETE STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS. STRUCTURAL OBSERVATION DOES NOT WAIVE THE RESPONSIBILITY FOR ANY SPECIAL INSPECTIONS REQUIRED.

   THE OWNER SHALL EMPLOY AN ENGINEER OR ARCHITECT (OR DESIGNEE APPROVED BY THE ENGINEER OF RECORD) TO PERFORM THE STRUCTURAL OBSERVATION. IT IS RECOMMENDED THAT THE ENGINEER OF RECORD BE RETAINED SINCE THEY ARE THE MOST FAMILIAR WITH THE STRUCTURAL SYSTEM AND CONTRACT DOCUMENTS.

   THE ENGINEER OR ARCHITECT RESPONSIBLE FOR THE STRUCTURAL OBSERVATION, THE CONTRACTOR, AND APPROPRIATE SUBCONTRACTORS SHALL HOLD A PRE-CONSTRUCTION MEETING TO REVIEW THE DETAILS OF THE STRUCTURAL SYSTEM TO BE OBSERVED. THE MEETING SHALL BE COORDINATED BY THE GENERAL CONTRACTOR PRIOR TO CONSTRUCTION.

   THE STRUCTURAL OBSERVER SHALL BE NOTIFIED AND SHALL PERFORM SITE VISITS AT THE STAGES IN THE PROGRESS OF THE WORK THAT ALLOW FOR CORRECTION OF DEFICIENCIES WITHOUT SUBSTANTIAL EFFORT OR UNCOVERING OF THE WORK INVOLVED. AT A MINIMUM, THE FOLLOWING SIGNIFICANT CONSTRUCTION STAGES REQUIRE A SITE VISIT AND AN OBSERVATION REPORT FROM THE STRUCTURAL OBSERVER.

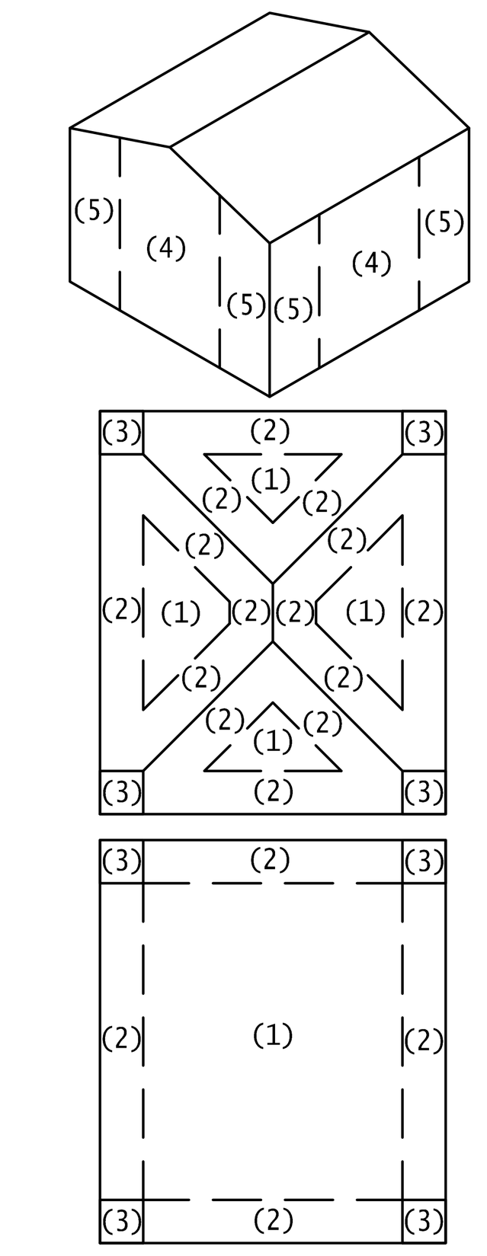
   HORIZONTAL DIAPHRAMS AND SHEARWALLS

   DEFICIENCIES SHALL BE REPORTED IN WRITING TO THE OWNER AND THE BUILDING OFFICIAL ON APPROVED JURISDICTION REPORTS. AT THE CONCLUSION OF THE WORK INCLUDED IN THE PERMIT, THE STRUCTURAL OBSERVER SHALL SUBMIT TO THE BUILDING OFFICIAL A WRITTEN STATEMENT THAT THE SITE VISITS HAVE BEEN MADE AND IDENTIFY ANY REPORTED DEFICIENCIES WHICH, TO THE BEST OF THE STRUCTURAL OBSERVER'S KNOWLEDGE, HAVE NOT BEEN RESOLVED.

STRUCTURAL SHEET INDEX

SHEET RANGE	DESCRIPTION
S001-S002	GENERAL STRUCTURAL NOTES
S011	TYPICAL FOUNDATION SECTIONS
S012	TYP FND, SITE & MASONRY DETAILS
S031	TYPICAL WALL FRAMING SECTIONS
S061-S062	SSI TABLES & SCHEDULES
S101	FOUNDATION PLAN
S102-S104	FLOOR FRAMING PLANS
S201	FOUNDATION SHEAR WALL PLAN
S202-S204	FLOOR SHEAR WALL PLANS
S301	STAIR PLANS & DETAILS
S401	FOUNDATION SECTIONS
S501-S503	FLOOR FRAMING SECTIONS
S601-S699	ROOF FRAMING SECTIONS

WIND LOADING DIAGRAMS



ABBREVIATIONS

A.B.C.	AGGREGATE BASE COURSE
A.F.F.	ABOVE FINISHED FLOOR
ALT.	ALTERNATE
A.B.	ANCHOR BOLT
B.F.F.	BELOW FINISHED FLOOR
BM	BEAM
B.O.D.	BOTTOM OF DECK
B.O.F.	BOTTOM OF FOOTING
B.O.S.	BOTTOM OF STEEL
BOT.	BOTTOM
BRG	BEARING
C.I.P.	CAST IN PLACE
CJ	CONTROL JOINT
CL	CENTERLINE
CLR	CLEAR
CMU	CONCRETE MASONRY UNIT
COL	COLUMN
CONT.	CONTINUOUS
DL	DEAD LOAD
E.F.	EACH FACE
EQ	EQUAL
EXP.	EXPANSION
E.W.	EACH WAY
F.F.	FINISHED FLOOR
F.F.E.	FINISHED FLOOR ELEVATION
F.G.	FINISHED GRADE
GA	GAGE
GSN	GENERAL STRUCTURAL NOTES
GLB	GLUED LAMINATED BEAM
GLULAM	GLUED LAMINATED BEAM
HOR	HORIZONTAL
I.B.C.	INTERNATIONAL BUILDING CODE
KIP	1000 POUNDS
LBS.	POUNDS
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
MCJ	MASONRY CONTROL JOINT
N.T.S.	NOT TO SCALE
O.C.	ON CENTER
OPP	OPPOSITE HAND
P.A.F.	POWDER ACTUATED FASTENERS
PC	PRECAST CONCRETE
PCF	POUNDS PER CUBIC FOOT
PEMB	PRE-ENGINEERED METAL BUILDING
PL	PLATE
PLF	POUNDS PER LINEAR FOOT
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
SIM	SIMILAR
S.I.P.	STRUCTURAL INSULATED PANEL
SP	SPACES
SSI	STRUCTURAL SPECIAL INSPECTION
STD	STANDARD
T&B	TOP AND BOTTOM
TL	TOTAL LOAD
T.O.B.	TOP OF BEAM
T.O.C.	TOP OF CONCRETE
T.O.D.	TOP OF DECK
T.O.F.	TOP OF FOOTING
T.O.L.	TOP OF LEDGER
T.O.M.	TOP OF MASONRY
T.O.P.	TOP OF PLATE
T.O.S.	TOP OF STEEL
T.O.W.	TOP OF WALL
TYP	TYPICAL
U.N.O.	UNLESS NOTED OTHERWISE
VERT.	VERTICAL
W.W.F.	WELDED WIRE FABRIC
W/	WITH
W/O	WITHOUT

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Registered Professional Engineer (Structural)
CERTIFICATE NO. 46644
W. DAVID BIXLER
DATE SIGNED 10/20/15
ARIZONA, U.S.A.
Expires 6/30/16

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DATE
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09-11-2015

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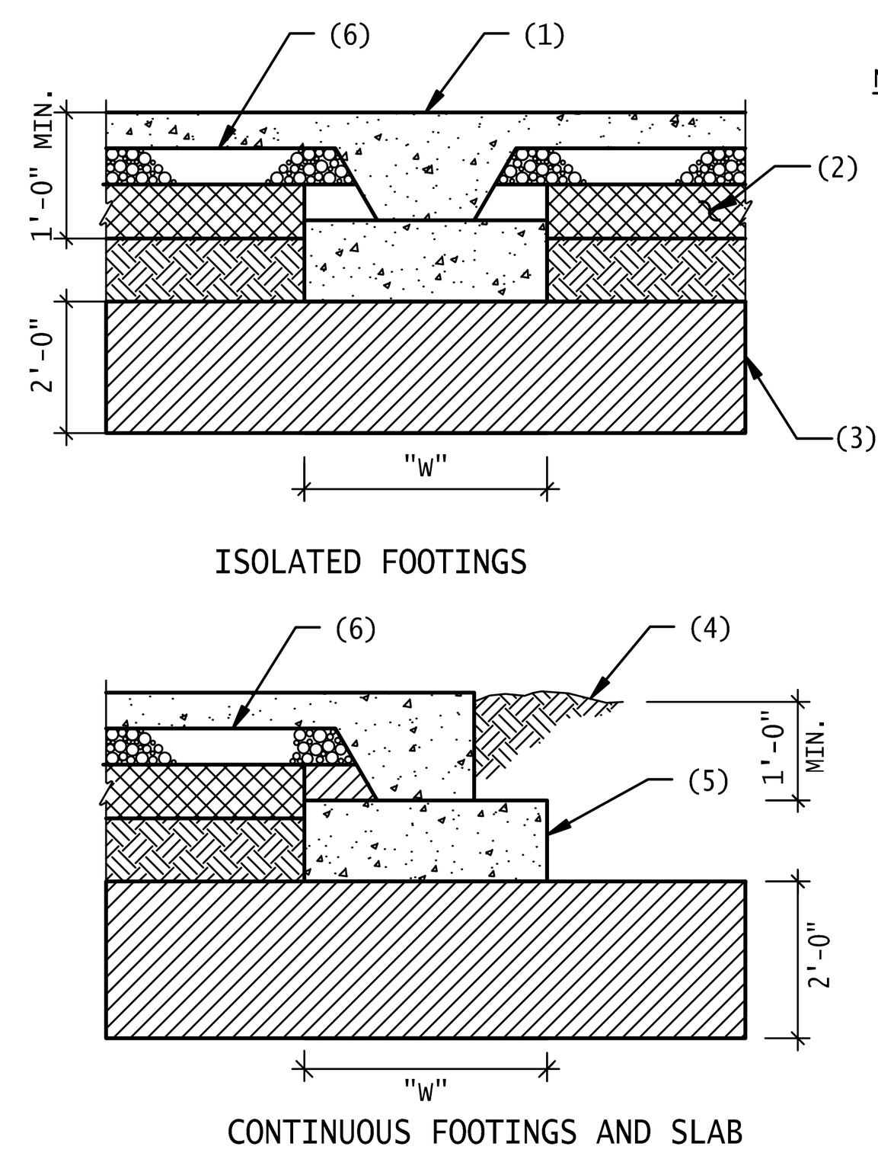
Proj. No. 15-049

GENERAL STRUCTURAL NOTES

S001

3042



ISOLATED FOOTINGS

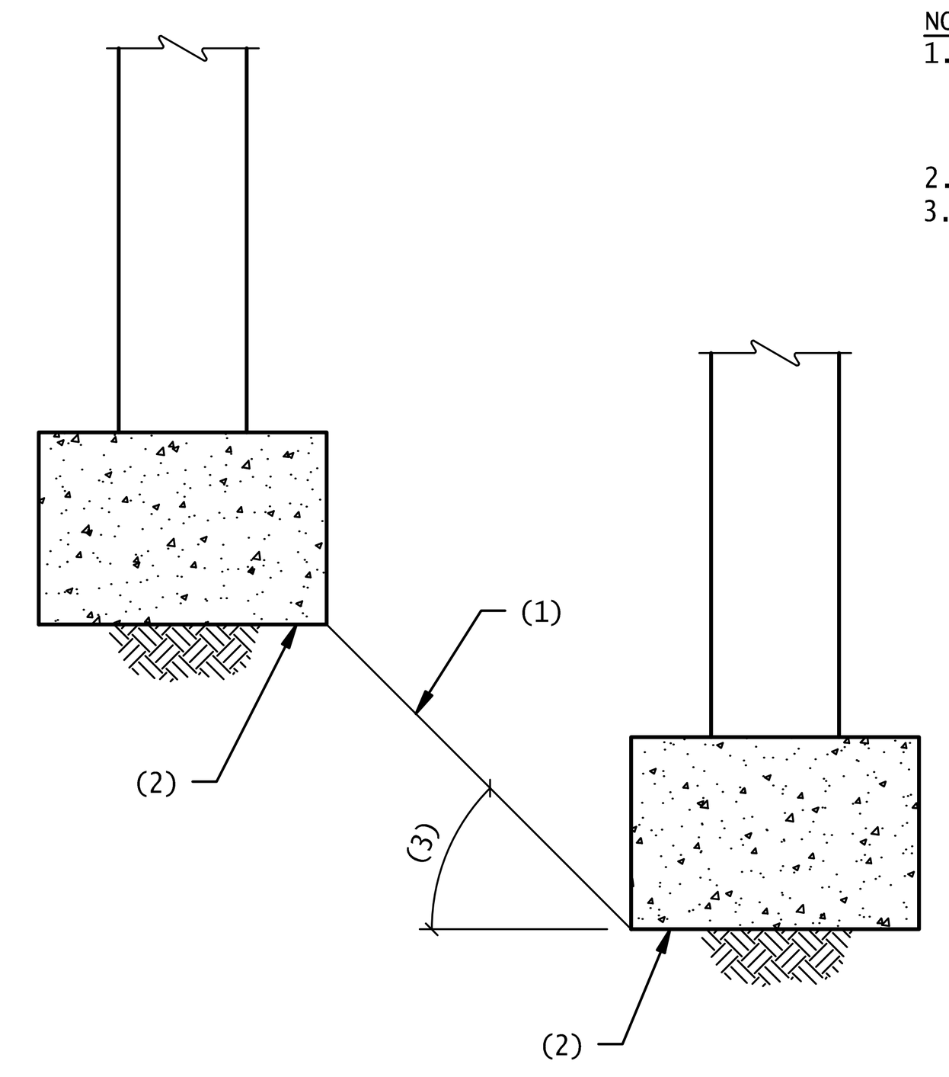
CONTINUOUS FOOTINGS AND SLAB

NOTES:
1. CONCRETE SLAB ON GRADE.
2. SCARIFY, MOISTEN, OR DRY AS REQUIRED, AND COMPACT ALL SUB GRADE SOILS BENEATH SLABS TO A MINIMUM DEPTH OF 6".
3. COMPACTED ENGINEERED FILL. AFTER OVER EXCAVATION HAS BEEN ACCOMPLISHED, CONTACT THE THE GEOTECHNICAL ENGINEER TO EVALUATE THE EXPOSED SUBGRADE AND PROVIDE ANY ADDITIONAL REMEDIATION REQUIRED. REQUIRED, AND COMPACTED TO A MINIMUM DEPTH OF 12".
4. FINISHED GRADE.
5. SEE PLANS AND GSN FOR TOP OF FOOTING ELEVATION.
6. 4" AGGREGATE BASE COURSE.

NOTE:
SEE G.S.N. FOR ADDITIONAL INFORMATION.

TYPICAL EARTHWORK DETAIL

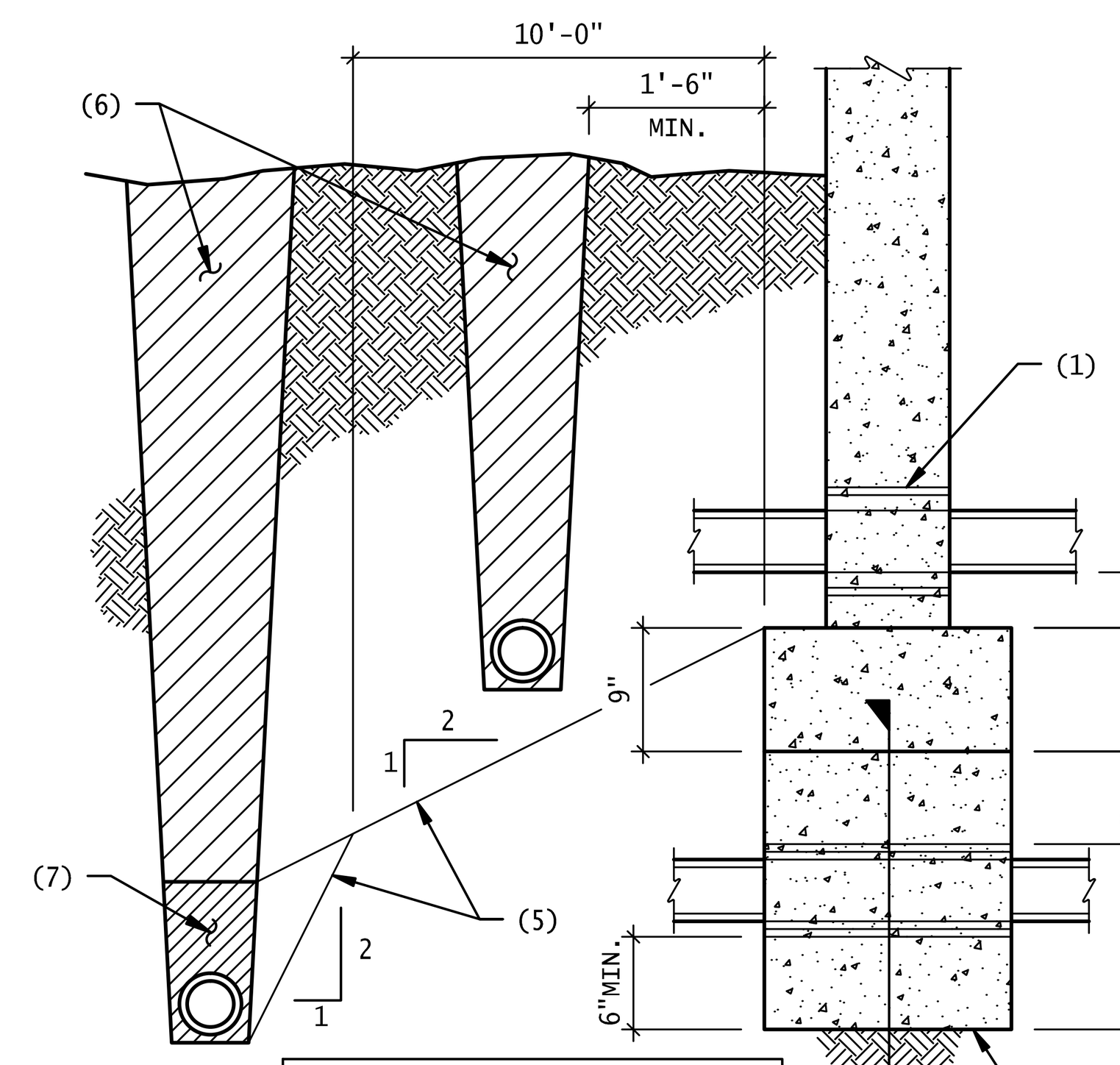
N.T.S.



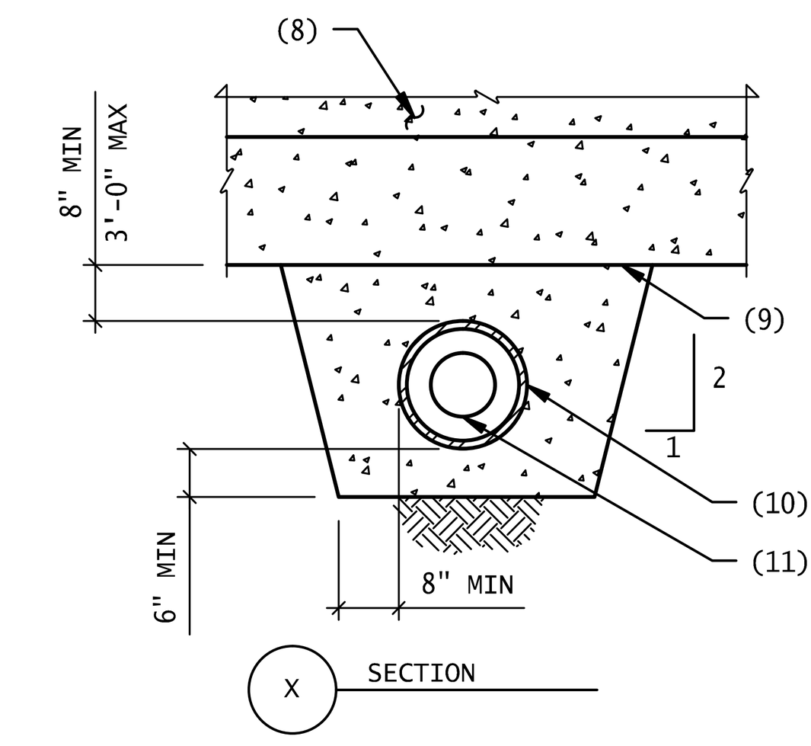
NOTES:
1. ADJACENT FOOTING SHALL NOT BE LOCATED BELOW LINE EXTENDING FROM BOTTOM OF FOOTING.
2. BOTTOM OF CONCRETE FOOTING.
3. 30° MAX IN GRANULAR SOILS, 45° MAX IN OTHER SOILS U.N.O.

MAXIMUM SLOPE BETWEEN ADJACENT FOOTING

N.T.S.



NOTE:
PIPES LOCATED 3'-0" OR GREATER BELOW FOOTING BASE MAY BE BACKFILLED WITH COMPACTED FILL PER SPECIFICATIONS.



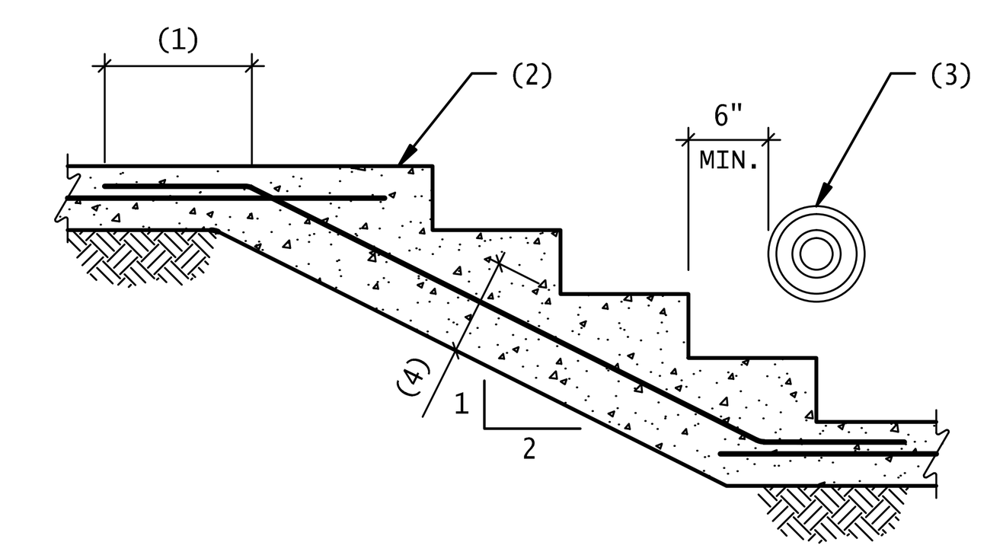
SECTION

NOTES:
1. CAST IRON SLEEVE SHALL BE 2" LARGER THAN PIPE.
2. NO SLEEVES PERMITTED IN THIS AREA OF FOOTING.
3. 3'-0" MAXIMUM - STEP FOOTING IF NECESSARY.
4. FILL PIPE TRENCH WITH CONCRETE.
5. EXCAVATION BELOW THESE LINES NOT PERMITTED.
6. BACK FILL.
7. COMPACT TO 90% DENSITY (ASTM D-698.)
8. STEM WALL.
9. CONCRETE FOOTING.
10. SLEEVE - PROVIDE 1/2" MINIMUM CLEARANCE AROUND PIPE OR CONDUIT.
11. PIPE OR CONDUIT.

NOTES:
A. PIPES SHALL NOT PASS UNDER COLUMN FOOTINGS.
B. FOR TRENCHES GREATER THAN 3'-6" BELOW BOTTOM OF FOOTING, SEE TYPICAL PIPE BELOW WALL FOOTING DETAIL.

TYPICAL PIPE BELOW FOOTING AND AT TRENCH

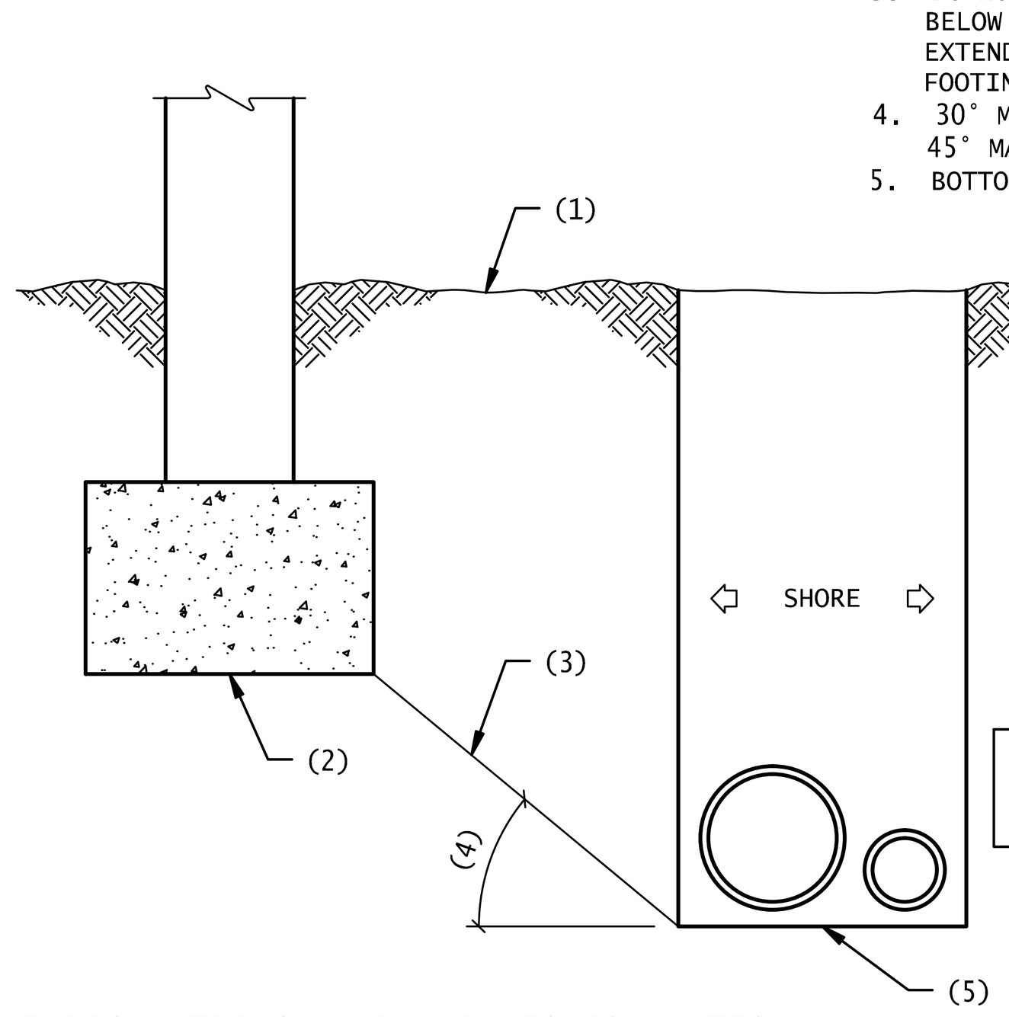
N.T.S.



NOTES:
1. 24" LAP.
2. WALL FOOTING.
3. PIPE THROUGH STEM WALL.
4. FOOTING THICKNESS.

TYPICAL STEP IN CONCRETE FOOTING

N.T.S.

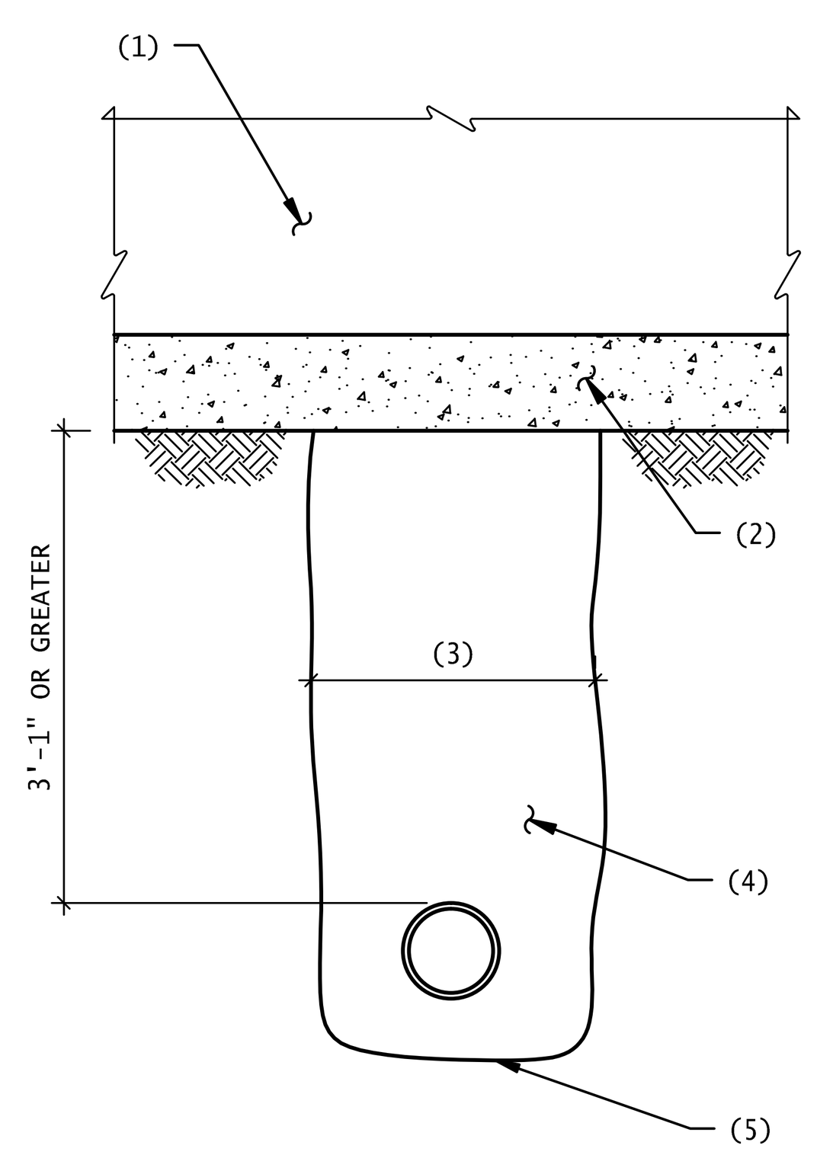


NOTES:
1. FINISHED GRADE WHERE OCCURS.
2. BOTTOM OF CONCRETE FOOTING.
3. DO NOT EXCAVATE A TRENCH BELOW IMAGINARY LINE EXTENDING FROM BOTTOM OF FOOTING OR FOUNDATION.
4. 30° MAX IN GRANULAR SOILS. 45° MAX IN OTHER SOILS, U.N.O.
5. BOTTOM OF TRENCH.

NOTE:
FOR ADDITIONAL INFORMATION, SEE PLANS AND DETAILS.

TYPICAL TRENCH PARALLEL TO FOUNDATION

N.T.S.

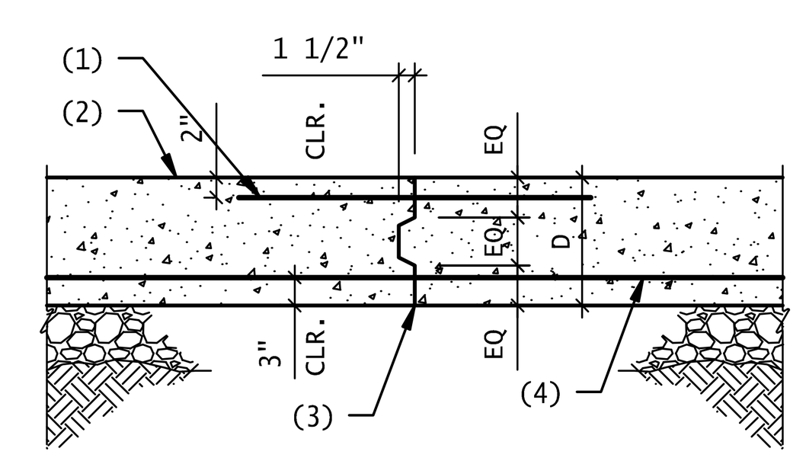


NOTES:
1. STEM WALL.
2. CONCRETE FOOTING.
3. 1'-6" MAXIMUM WIDTH - WHERE TRENCH EXCEEDS 1'-6" NOTIFY ENGINEER OF RECORD PRIOR TO PLACEMENT OF FOOTING.
4. BACKFILL AND RECOMPACT TRENCH.
5. BOTTOM OF TRENCH.

NOTE:
FOR TRENCH DEPTHS OF 3'-6" OR LESS, SEE TYPICAL PIPE BELOW FOOTING AND AT TRENCH DETAIL.

PIPE PASSING BELOW WALL FOOTING IN DEEP TRENCH

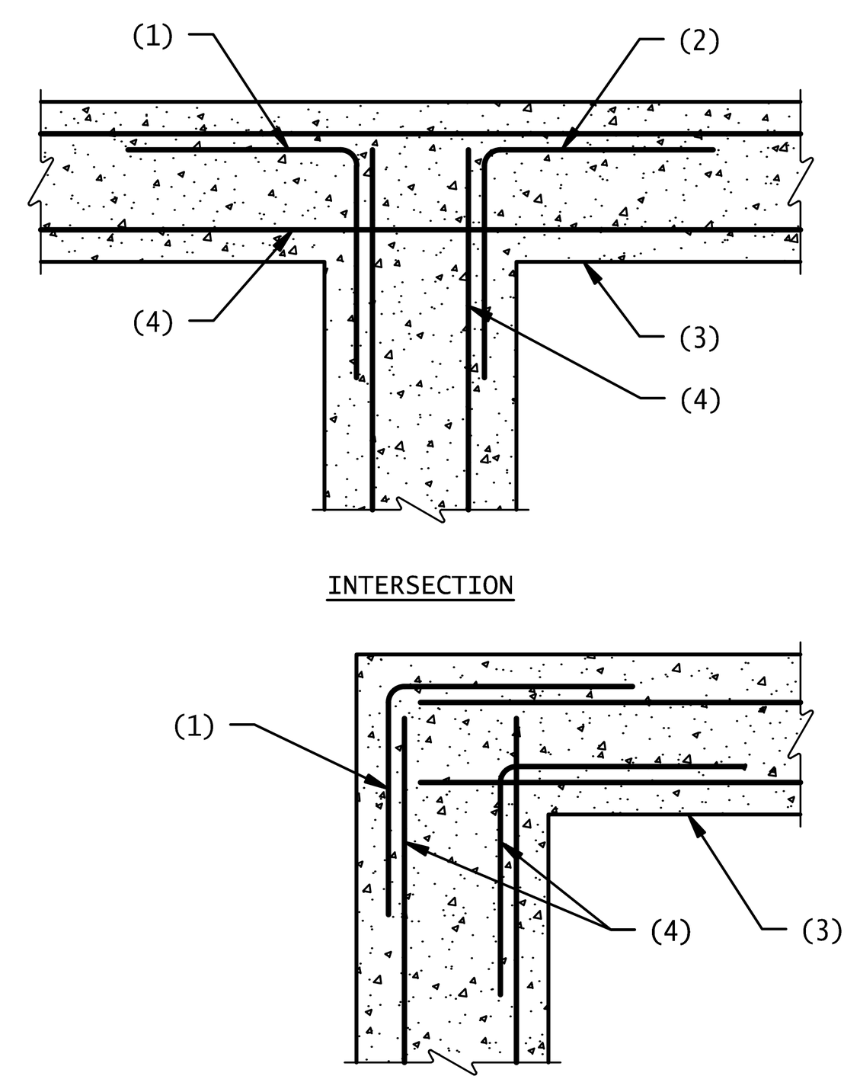
N.T.S.



NOTES:
1. 2 - #5 x 4'-0" CENTERED ON JOINT.
2. CONCRETE FOOTING.
3. PROVIDE KEY FULL WIDTH OF FOOTING.
4. BOTTOM REINFORCING IS CONTINUOUS THROUGH JOINT.

TYPICAL KEY IN CONCRETE

N.T.S.



INTERSECTION

CORNER

NOTES:
1. CORNER BARS SAME SIZE AND SPACING AS HORIZONTAL REINFORCING. LAP 24" MINIMUM.
2. ALTERNATE HOOKS.
3. CONCRETE STEM WALL OR FOOTING.
4. REINFORCING PER PLANS AND/OR G.S.N.

TYPICAL REINFORCING IN CONCRETE FOOTING AND/OR STEM WALL CORNERS

N.T.S.

	TENSION SPLICE LENGTHS (CLASS B)												COMP. BARS	
CONC PSI	f'c ≤ 3,000 PSI				f'c ≤ 4,000 PSI				f'c ≤ 5,000 PSI				ALL f'c	
BAR LOCATION	REGULAR		TOP		REGULAR		TOP		REGULAR		TOP		STD LAP	ENCLOSED W/ SPIRAL TIES
SIZE \ SPACING	≥2db	OTHER	≥2db	OTHER	≥2db	OTHER	≥2db	OTHER	≥2db	OTHER	≥2db	OTHER		
#3	24"	36"	31"	46"	19"	28"	25"	37"	17"	25"	22"	33"	12"	12"
#4	32"	47"	41"	61"	25"	37"	33"	49"	23"	34"	29"	44"	15"	12"
#5	39"	59"	51"	77"	31"	47"	41"	61"	28"	42"	36"	54"	19"	15"
#6	47"	71"	61"	92"	37"	56"	49"	73"	34"	50"	44"	65"	23"	18"
#7	69"	103"	89"	134"	54"	81"	71"	106"	49"	73"	63"	95"	27"	21"
#8	78"	117"	102"	153"	62"	93"	81"	121"	56"	83"	72"	108"	30"	23"
#9	88"	132"	115"	172"	70"	105"	91"	136"	63"	94"	81"	122"	34"	26"
#10	100"	149"	129"	194"	79"	118"	102"	153"	71"	106"	92"	137"	39"	30"
#11	110"	165"	143"	215"	87"	131"	114"	170"	78"	117"	102"	152"	43"	33"

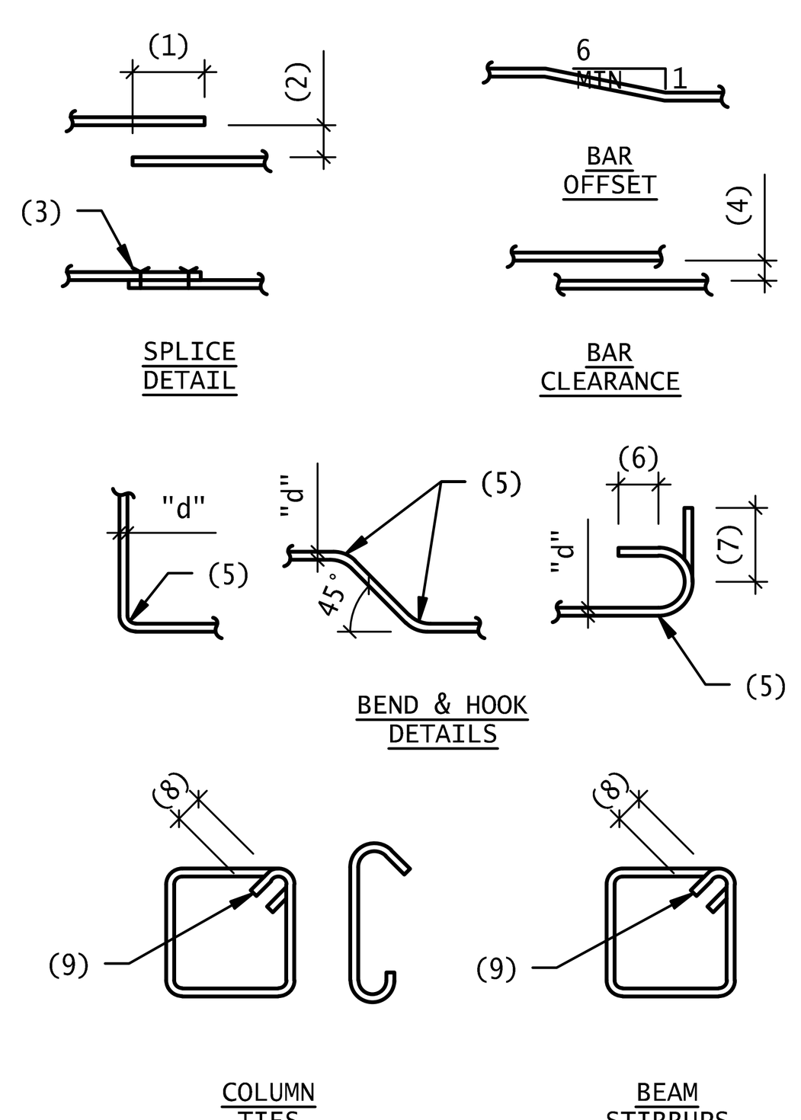
NOTES:
1. TOP BARS ARE ANY HORIZONTAL BARS PLACED WITHIN 8" OF THE TOP OF THE MEMBER.

MINIMUM REINFORCING BAR SPLICE LENGTHS IN CAST IN PLACE CONCRETE

END HOOKS				
BAR SIZE	FINISHED HOOK DIA. "D", IN.	180° HOOKS		90° HOOKS
		"A" OR "G", IN.	"J", IN.	"A" OR "G", IN.
#3	2.25	5	3	6
#4	3	6	4	8
#5	3.75	7	5	10
#6	4.5	8	6	12
#7	5.25	10	7	14
#8	6	11	8	16
#9	9.5	15	11.75	19
#10	10.75	17	13.25	22
#11	12	19	14.75	24
#14	18.25	27	21.75	31
#18	24	36	28.5	41

TYPICAL REINFORCING HOOK SCHEDULE

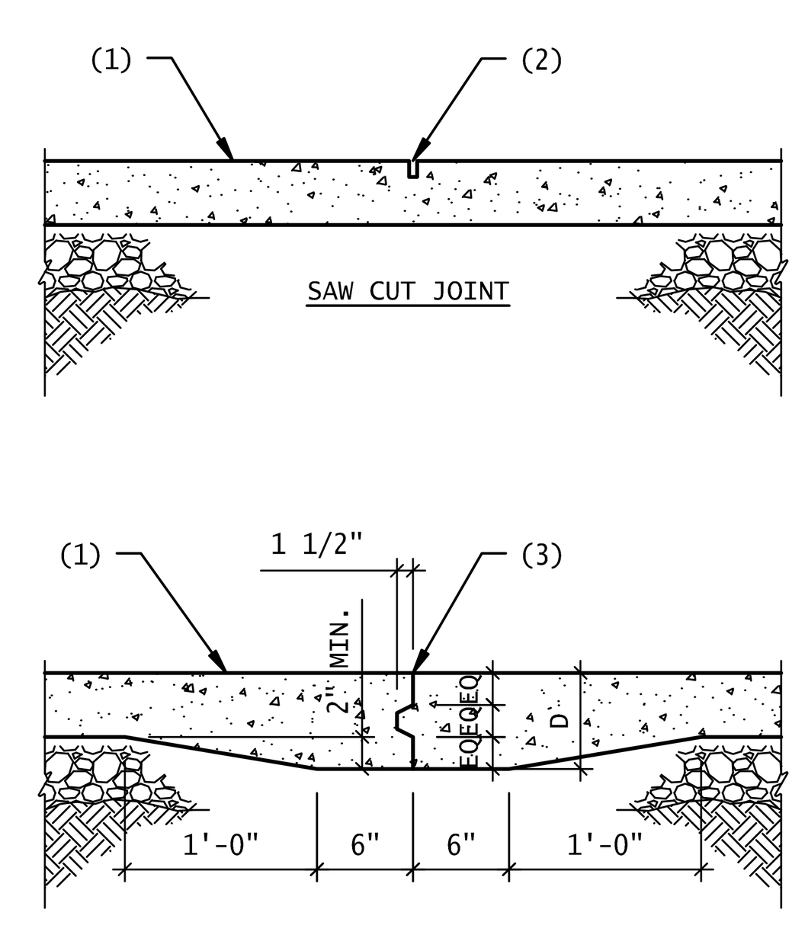
N.T.S.



NOTES:
1. LAP PER G.S.N.
2. MAXIMUM DISTANCE 1/5 OF REQUIRED LAP LENGTH BUT NOT MORE THAN 6".
3. WIRE TIES.
4. 1d OR 1", WHICHEVER IS GREATER.
5. RADIUS=3d FOR BARS LESS THAN #9; 4d FOR #9 TO #11 BARS; 5d FOR #14 AND #18 BARS. 5d FOR ALL GRADE 40 BARS WITH 180 DEGREE HOOK.
6. 4d OR 4", WHICHEVER IS GREATER.
7. 12d (90 DEGREE HOOK.)
8. 6d OR 4", WHICHEVER IS GREATER.
9. 135 DEGREE HOOK.

TYPICAL CONCRETE REINFORCING BAR DETAILS

N.T.S.

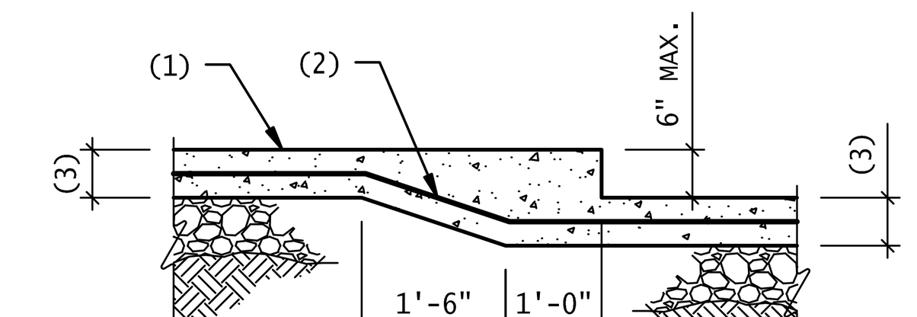


NOTES:
1. CONCRETE SLAB ON GRADE.
2. SAWCUT, 1/8" WIDE x 1/4 SLAB THICKNESS IN DEPTH. CUT SHALL BE MADE AS SOON AS POSSIBLE AFTER POUR TO PREVENT SHRINKAGE CRACKING, BUT AFTER THE CONCRETE HAS CURED SUFFICIENTLY TO PREVENT SPALLING OF THE CONCRETE WHILE SAWING. FILL JOINT WITH A FLEXIBLE SEALANT.
3. CONTINUOUS KEY.

NOTE:
KEYED CONSTRUCTION JOINTS NEED ONLY OCCUR AT EXPOSED EDGES DURING PLACEMENT UNLESS SPECIFICALLY NOTED (KCJ) ON THE PLANS.

TYPICAL CONTROL JOINTS IN CONCRETE SLAB ON GRADE

N.T.S.



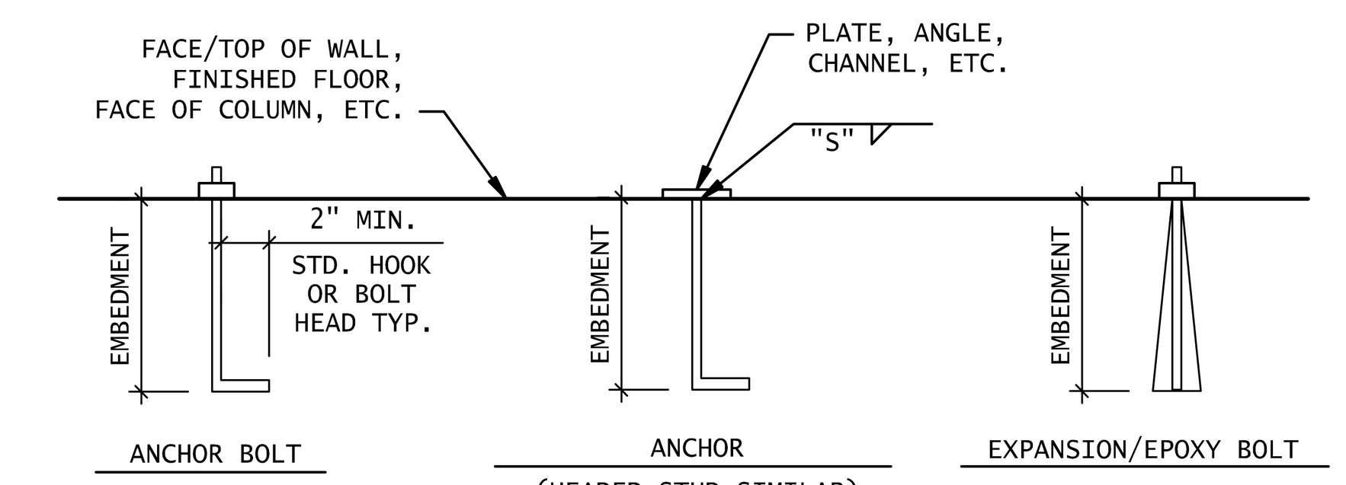
NOTES:
1. CONCRETE SLAB ON GRADE.
2. REINFORCING CONTINUOUS THROUGH SLAB DEPRESSION. SEE PLAN FOR REINFORCING.
3. SLAB THICKNESS PER PLAN.

TYPICAL SLAB DEPRESSION

N.T.S.

BOLT DIAMETER	VERT BOLT EMBEDMENT LENGTH	HORIZ BOLT EMBEDMENT LENGTH	FILLET WELD SIZE, "S".
1/2"	7"	4"	1/4"
5/8"	7"	4"	5/16"
3/4"	7"	5"	5/16"
7/8"	8"	6"	5/16"
1"	9"	7"	3/8"
1 1/8"	10"	8"	---
1 1/4"	11"	9"	---

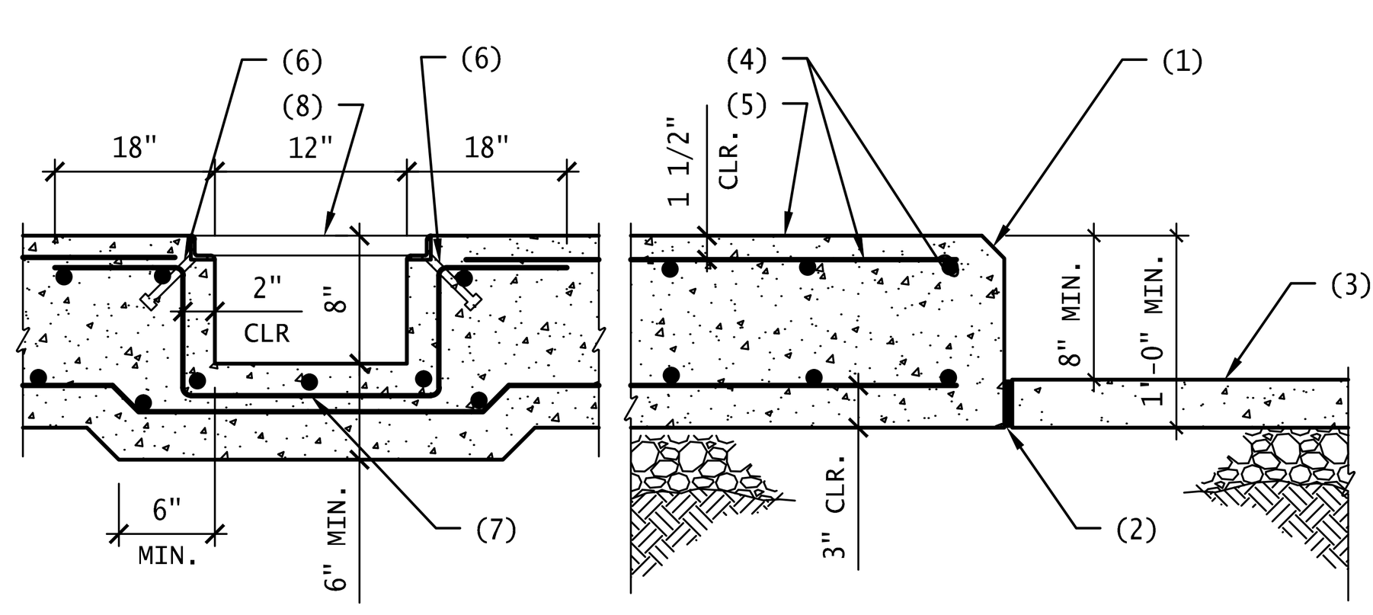
NOTES:
1. PROVIDE ANCHORS, ANCHOR BOLTS, EPOXY BOLTS AND EXPANSION BOLTS PER THIS SCHEDULE UNLESS NOTED ON PLANS OR DETAILS.
2. DRYPACK THICKNESS DOES NOT APPLY TOWARDS EMBEDMENT.



TYPICAL ANCHOR, ANCHOR BOLT, AND EXPANSION BOLT SCHEDULE

N.T.S.

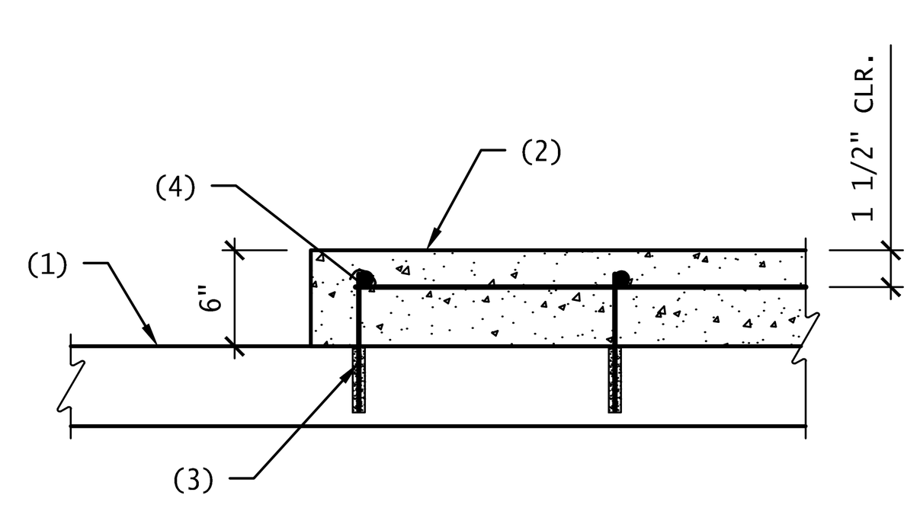
NOTES:
1. CHAMFER EDGES.
2. 1/2" EXPANSION JOINT MATERIAL.
3. CONCRETE SLAB ON GRADE.
4. #4 @ 12" O.C. EACH WAY TOP AND BOTTOM.
5. CONCRETE EQUIPMENT PAD.
6. CONT. L1 1/2x1 1/2x1/4 W/1/2"Ø x 4" HEADED STUDS @ 24"O.C. (TYP)
7. #3 @ 12"O.C. EACH WAY.
8. TRENCH DRAIN PER MECHANICAL DRAWINGS.



TYPICAL CONCRETE EQUIPMENT PAD ON GRADE

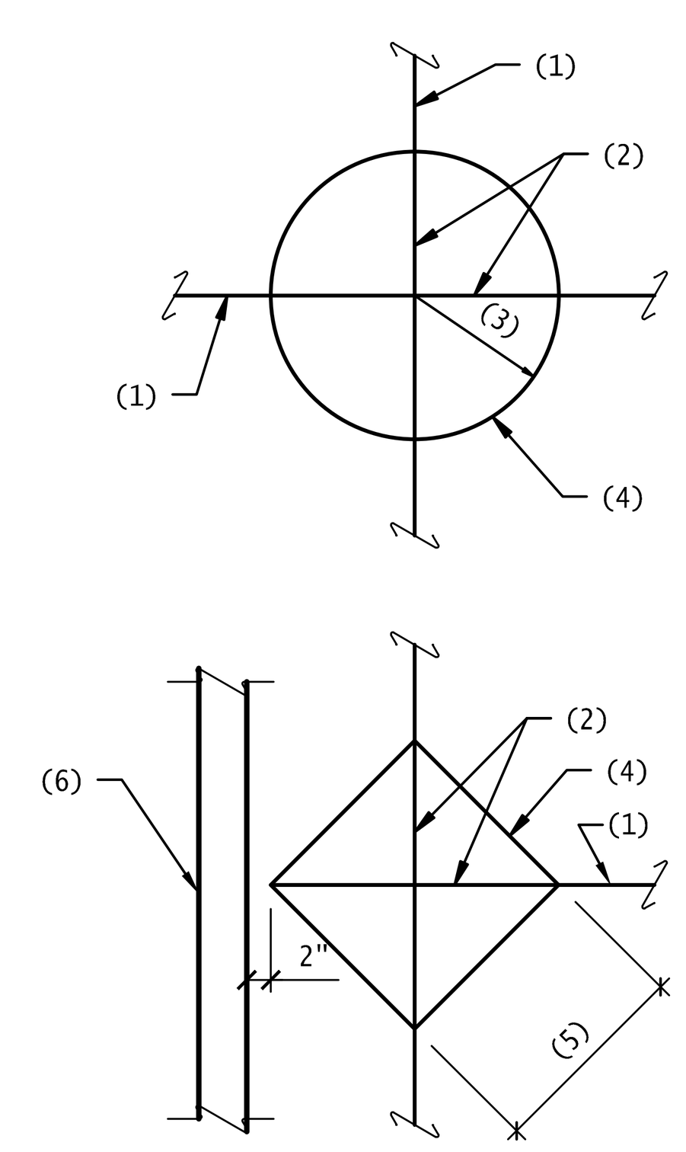
N.T.S.

NOTES:
1. CONCRETE SLAB ON GRADE.
2. CONCRETE CURB.
3. #3 DOWELS @ 16" O.C.E.W. DRILL AND EPOXY W/3" EMBEDMENT.
4. #3 CONTINUOUS AT 16" O.C.E.W.



TYPICAL HOUSEKEEPING PAD

N.T.S.

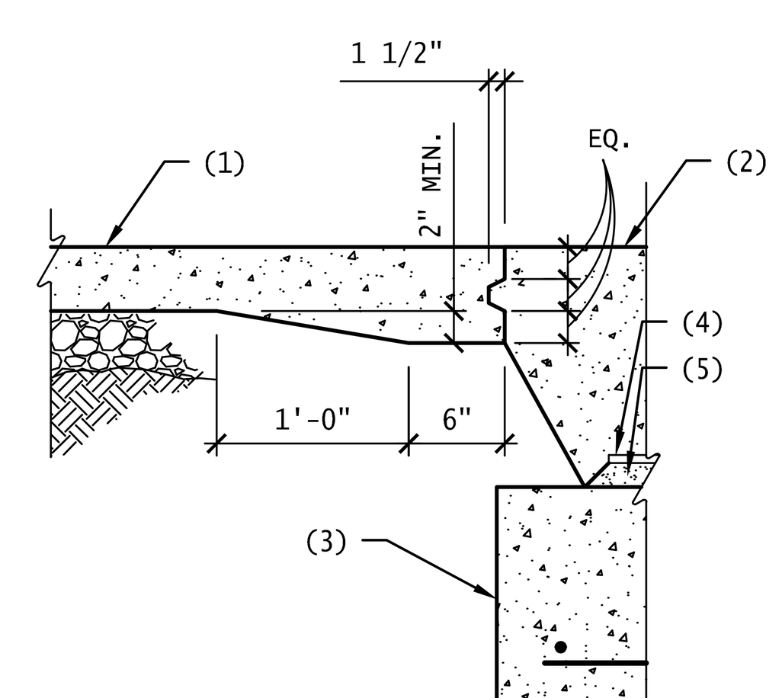


NOTES:
1. "CONC. C.J." WHERE SHOWN ON PLAN.
2. CENTERLINE OF COLUMN.
3. RADIUS 1'-6" MINIMUM TO 3'-0" MAXIMUM.
4. KEYED JOINT - SEE TYPICAL THICKENED SLAB AT CONCRETE CLOSURE POUR DETAIL.
5. 2'-0" MINIMUM TO 4'-0" MAXIMUM.
6. 2 - #4 x 4'-0" LONG CENTERED IN SLAB AT 4" O.C. TYP. AT LOCATIONS WHERE NO C.J. OCCURS.

TYPICAL COLUMN CLOSURE POUR AT CONCRETE SLAB ON GRADE

N.T.S.

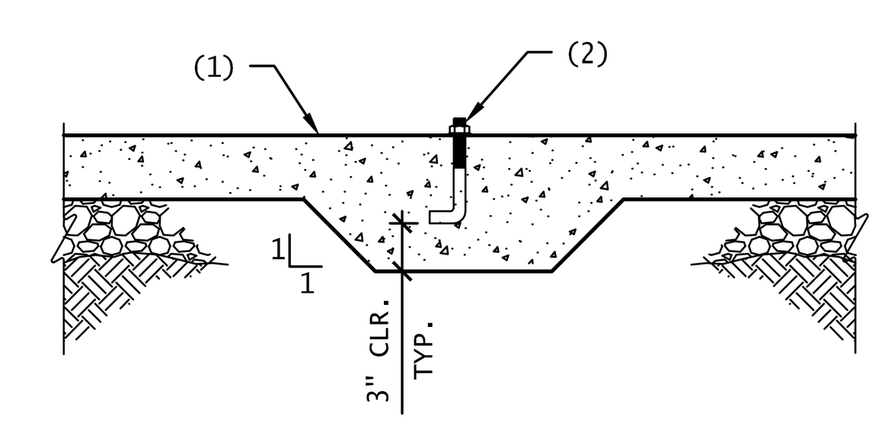
NOTES:
1. CONCRETE SLAB ON GRADE.
2. CONCRETE CLOSURE POUR.
3. CONCRETE FOOTING.
4. STEEL BASE PLATE
5. 1 1/2"± DRYPACK.



TYPICAL THICKENED SLAB AT CONCRETE CLOSURE POUR DETAIL

N.T.S.

NOTES:
1. CONCRETE FOOTING.
2. ANCHOR BOLT.



TYPICAL THICKENED FOOTING AT ANCHOR BOLT

N.T.S.

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Proj. No. 15-049

GERALD R. KESLER, INC.
ARCHITECTS

Holiday Inn Express & Suites

1170 S. Arizona Ave.
Chandler, Arizona, 85286

JOB NUMBER
1401

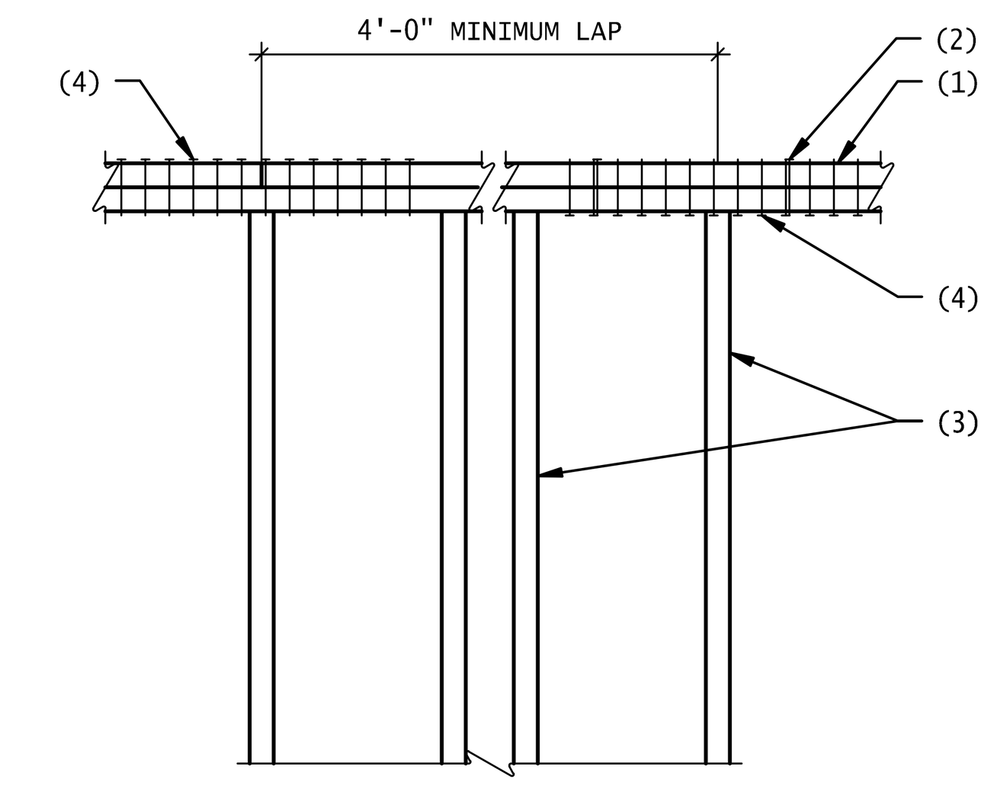
DATE
07-23-2015

1 1st City Comments
09-11-2015

TYPICAL FOUNDATION DETAILS

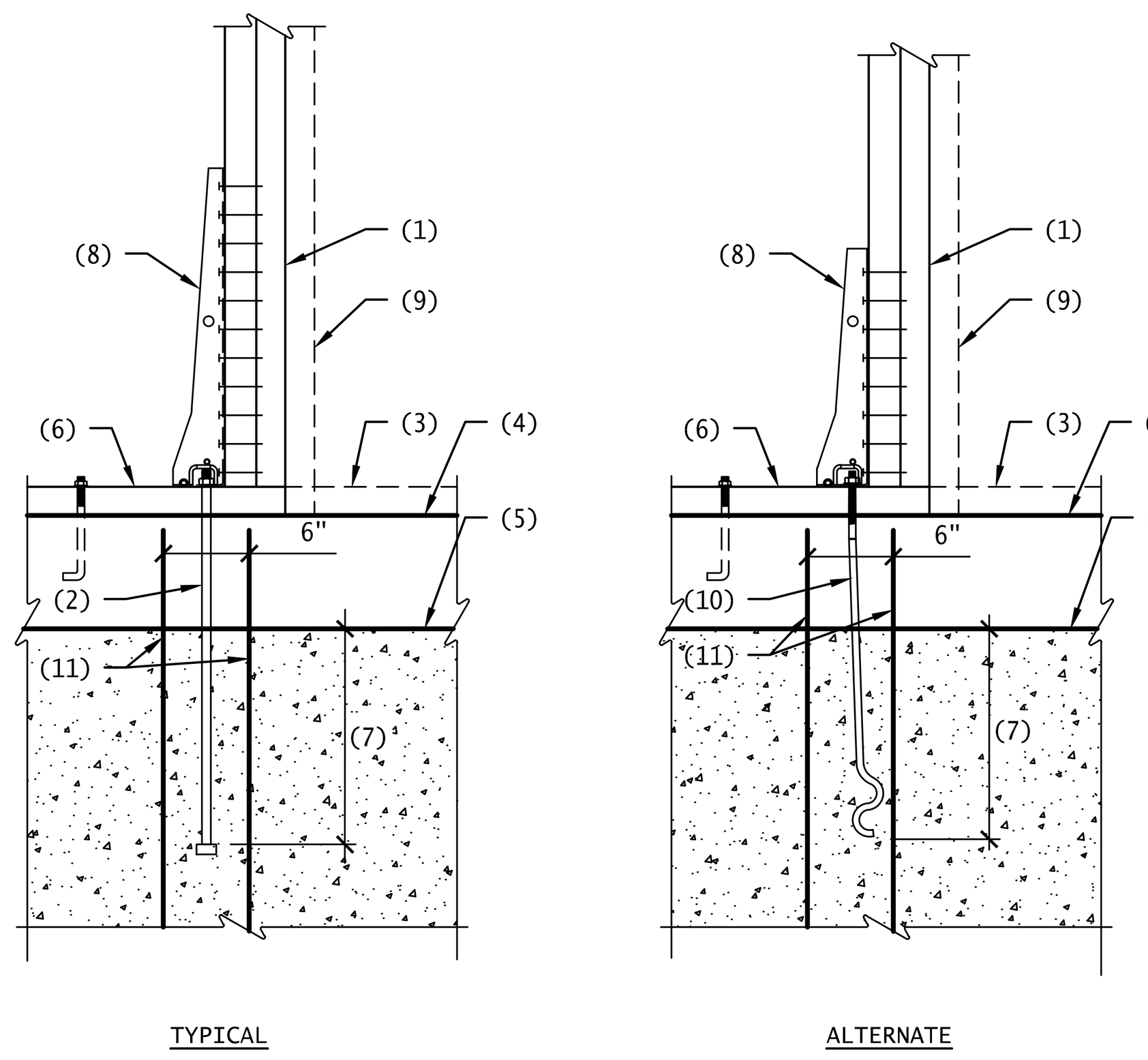
S011

3042



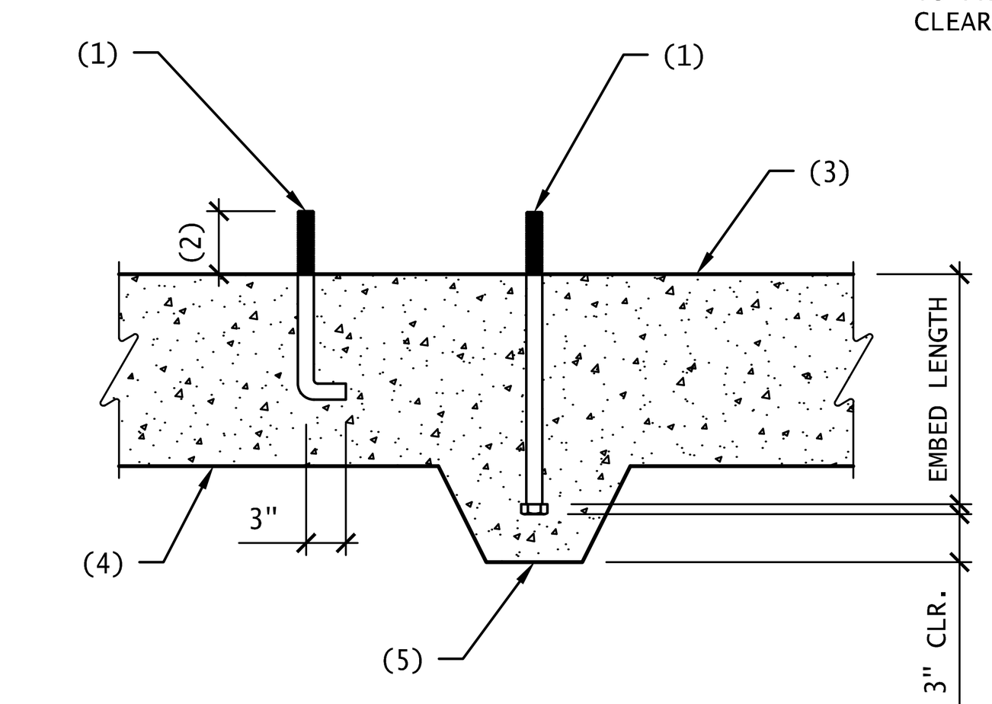
NOTES:
1. DOUBLE TOP PLATE.
2. 16d AT 12" O.C.
3. WOOD STUDS.
4. SIMPSON CMST16 FLAT STRAP W/32-16d NAILS EACH SIDE.

TOP PLATE SPLICE
N.T.S.



NOTES:
1. CHORD STUDS OR POST WHERE REQUIRED AT EACH HOLDOWN.
2. THREADED ROD PER SCHEDULE.
3. SILL PLATE CONTINUOUS WHERE SHOWN ON DETAILS.
4. CONCRETE STEM WALL OR STRUCTURAL SLAB, SEE PLANS FOR ADDITIONAL INFO. (OTHER REINFORCING NOT SHOWN FOR CLARITY).
5. TOP OF FOOTING.
6. 2x WOOD PLATE AND ANCHOR BOLTS PER SHEAR WALL SCHEDULE.
7. FOR EMBEDMENT SEE TYPICAL ANCHOR, ANCHOR BOLT AND EXPANSION BOLT SCHEDULE, AND TYPICAL ANCHOR BOLT DETAIL.
8. HOLDOWN SEE FOUNDATION PLANS FOR TYPE AND LOCATION.
9. CHORD STUDS.
10. SIMPSON SSTB ANCHOR PER SCHEDULE.
11. PROVIDE ADDITIONAL 2 - #4 VERTICAL DOWELS AT STEM WALL LOCATIONS.

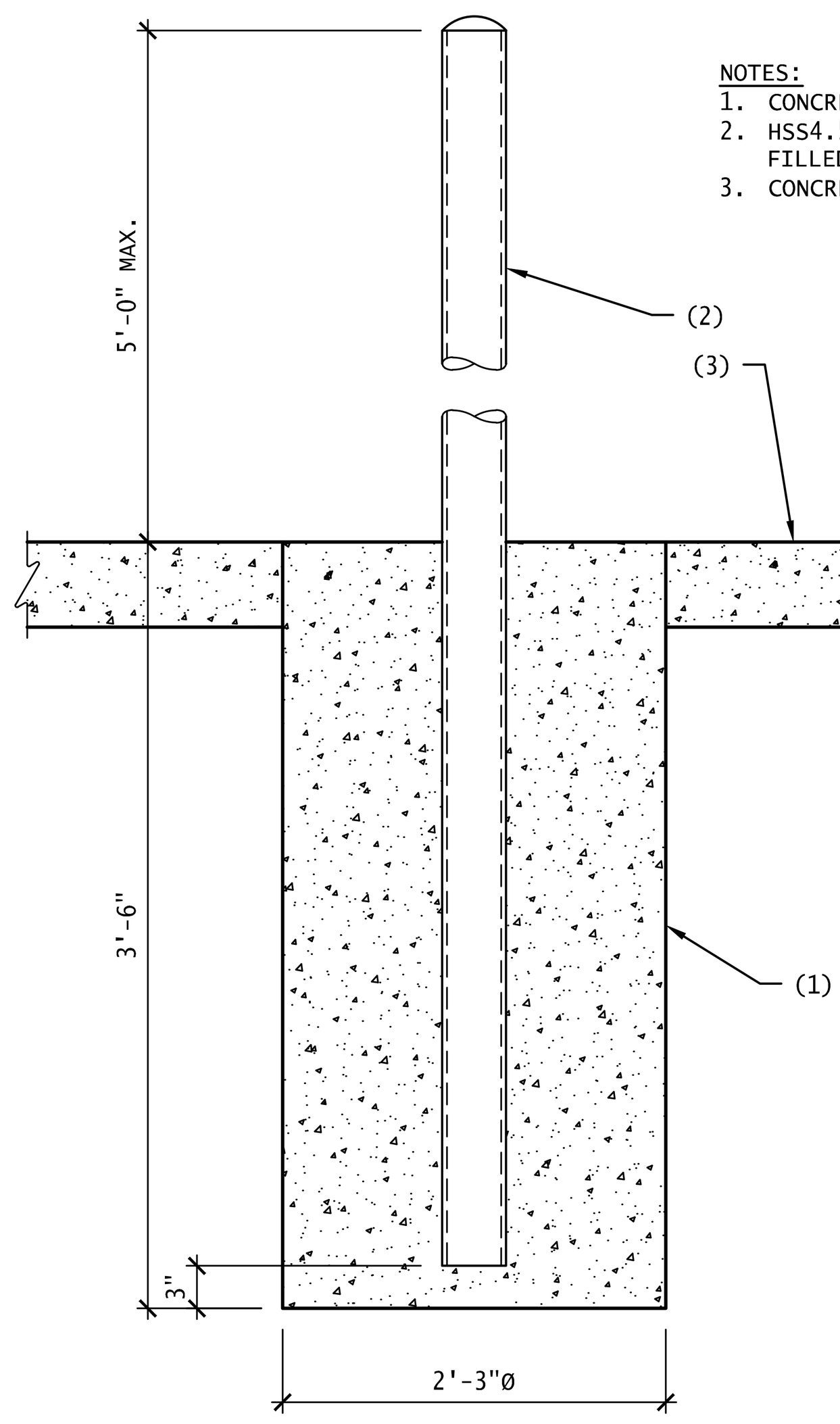
TYPICAL HOLDOWNS AT WOOD STUD SHEAR WALLS
N.T.S.



NOTES:
1. ANCHOR BOLT. SEE COLUMN SCHEDULE FOR SIZE AND EMBED DEPTH.
2. PROJECTION AS REQUIRED.
3. TOP OF FOOTING.
4. BOTTOM OF FOOTING.
5. DEEPEN FOOTING AS REQUIRED TO PROVIDE MINIMUM BOLT CLEARANCES SHOWN.

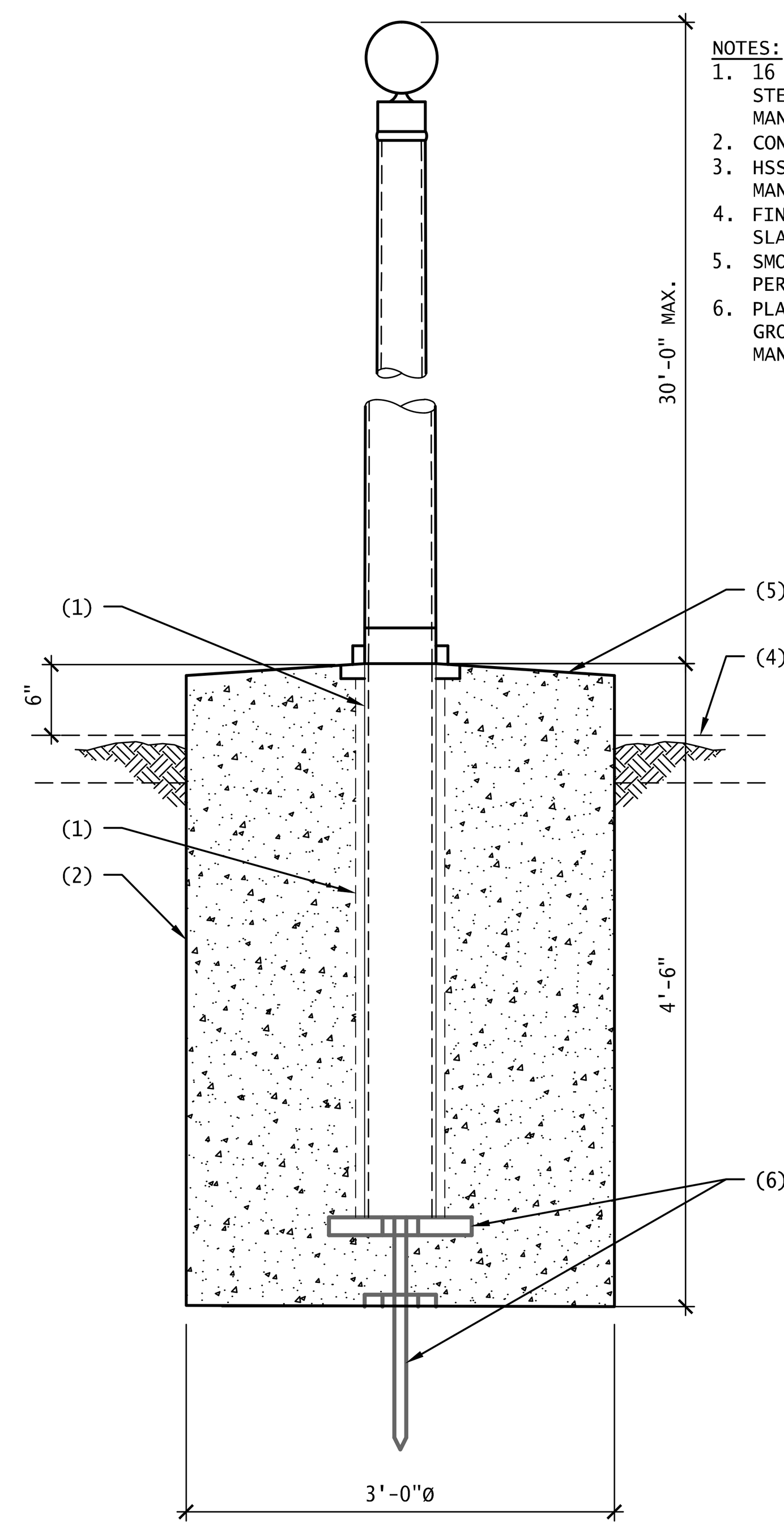
NOTE:
FOOTING REINFORCING NOT SHOWN.

TYPICAL ANCHOR BOLT DETAIL
N.T.S.



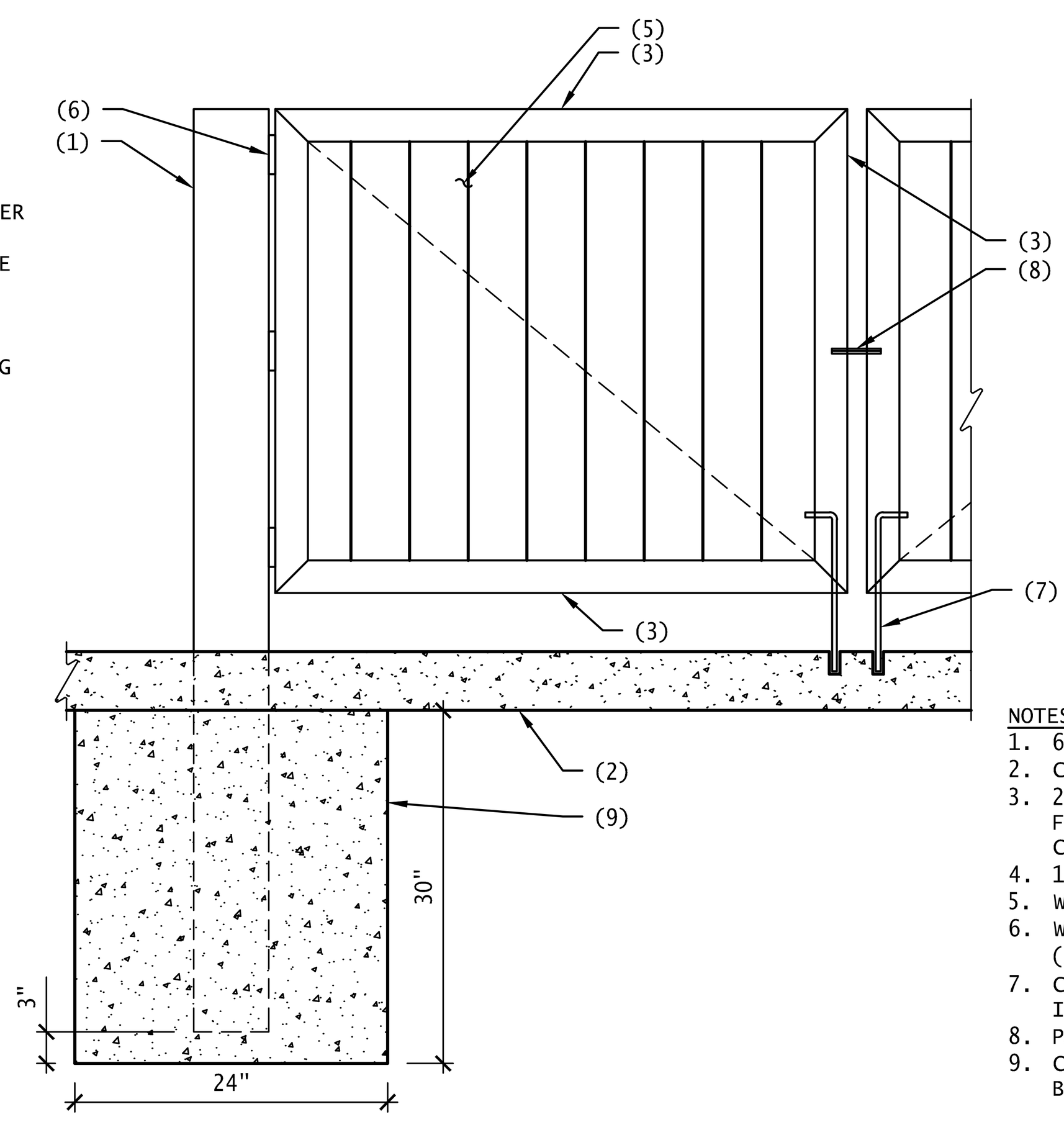
NOTES:
1. CONCRETE FOOTING.
2. HSS4.5x0.337 CONCRETE FILLED PIPE BOLLARD.
3. CONCRETE SLAB ON GRADE.

TYPICAL BOLLARD DETAIL
N.T.S.



NOTES:
1. 16 G.A. CORR GALVANIZED STEEL FOUNDATION TUBE PER MANUFACTURER.
2. CONCRETE FOOTING.
3. HSS6.0x0.337 STEEL POLE PER MANUFACTURER.
4. FINISHED GRADE OR CONCRETE SLAB AS OCCURS.
5. SMOOTH SLOPPED MORTAR CAP PER ARCHITECTURAL.
6. PLATE, SUPPORT & LIGHTNING GROUND SPIKE PER MANUFACTURER.

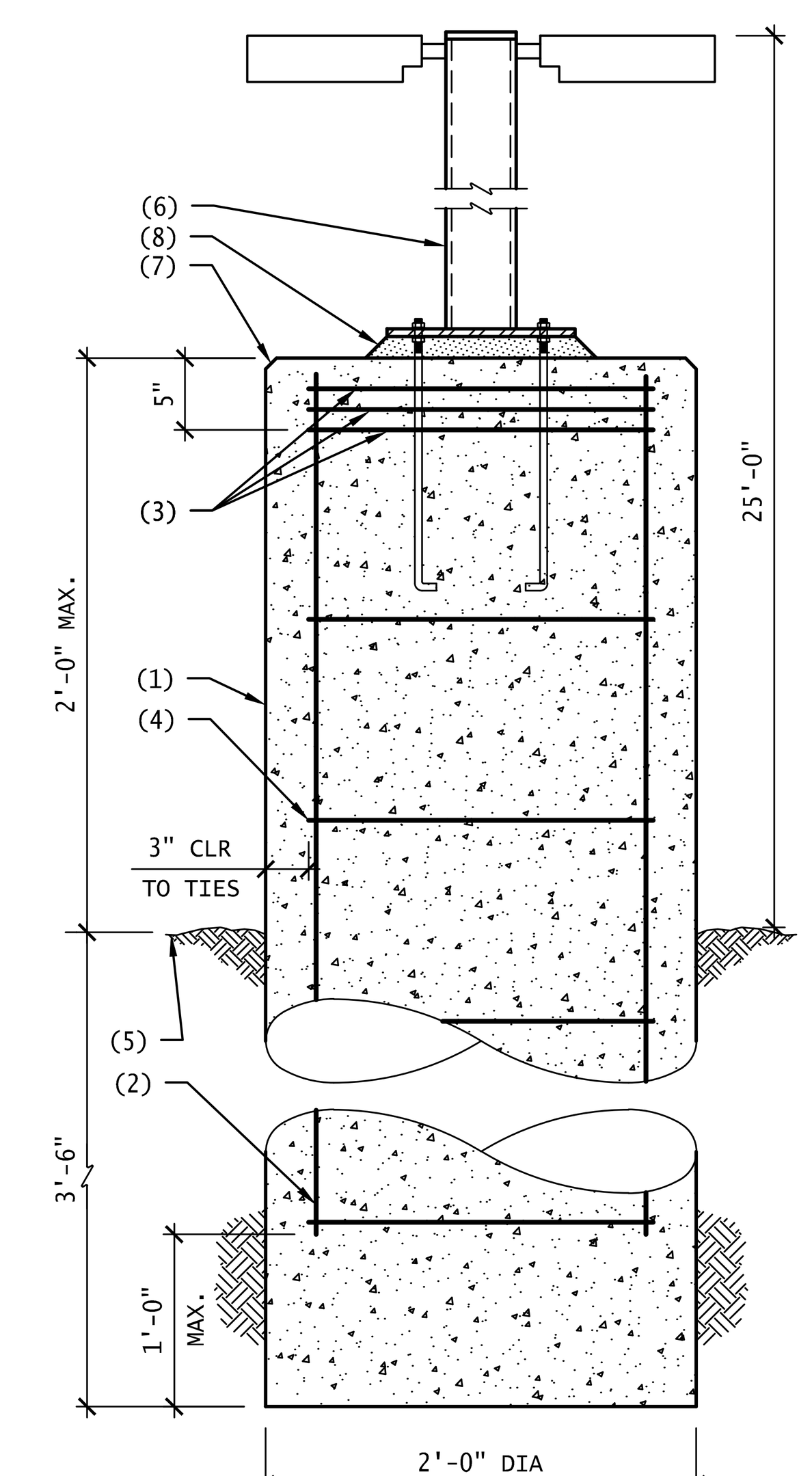
TYPICAL FLAG POLE DETAIL
N.T.S.



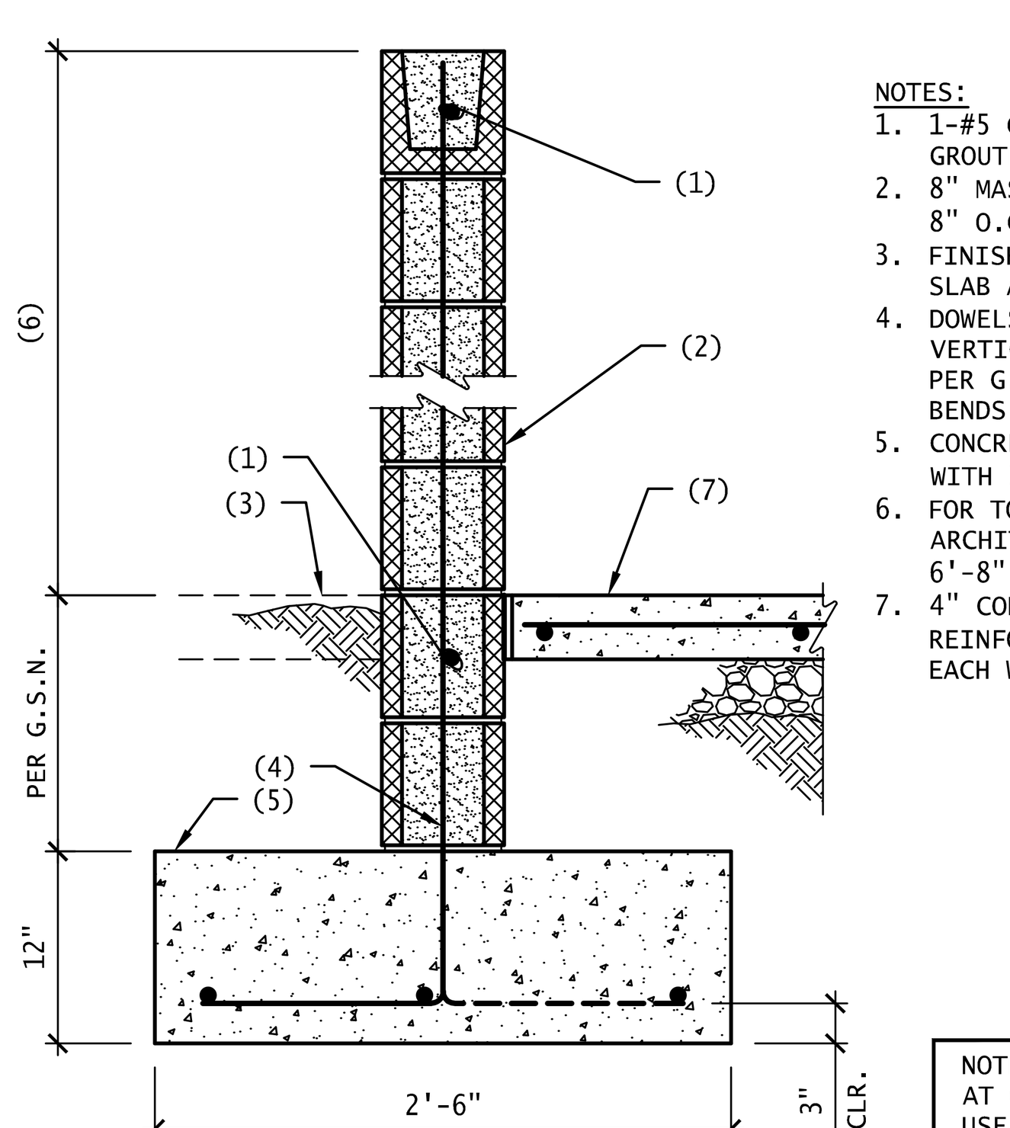
NOTES:
1. 6" STEEL PIPE BOLLARD.
2. CONCRETE SLAB.
3. 2 1/2" TUBE STEEL GATE FRAME. GRIND WELDS AND CORNERS SMOOTH.
4. 1/2" STEEL ROD BRACE.
5. WESTERN RED CEDAR OR EQUAL.
6. WELDED HEAVY DUTY HINGE (3 REQUIRED).
7. CANE BOLT W/PIPE SLEEVES INTO CONCRETE SLAB.
8. PADLOCK HASP WITH PADLOCK.
9. CONCRETE FOOTING FOR BOLLARD.

TRASH ENCLOSURE GATE PANEL
N.T.S.

NOTES:
1. CONCRETE CAISSON.
2. 6 - #6 VERTICAL REINFORCING.
3. 3-#3 TIES IN TOP 5".
4. #3 TIES @ 12" O.C. MAX.
5. FINISHED GRADE.
6. STEEL LIGHT POLE WITH 4 - 1/2"Ø ANCHOR BOLTS x 18" EMBEDMENT MIN.
7. 3/4" CHAMFER ALL AROUND.
8. 1 1/2" MIN, 5000 PSI NON-SHRINK GROUT.



TYPICAL LIGHT POLE SUPPORT
N.T.S.



NOTES:
1. 1-#5 CONTINUOUS IN 8" DEEP GROUTED BOND BEAM.
2. 8" MASONRY WALL WITH #4 AT 8" O.C., GROUT SOLID.
3. FINISHED GRADE OR CONCRETE SLAB AS OCCURS.
4. DOWELS TO MATCH AND LAP VERTICAL WALL REINFORCING PER G.S.N., ALTERNATE BENDS.
5. CONCRETE FOOTING REINFORCED WITH 3-#5 CONTINUOUS.
6. FOR TOP OF WALL, SEE ARCHITECTURAL DRAWINGS, 6'-8" MAX.
7. 4" CONCRETE SLAB ON GRADE REINFORCED #4 @ 16" O.C. EACH WAY, CENTERED IN SLAB.

NOTE:
AT CONTRACTOR'S OPTION, USE CONCRETE STEM WALL WITH SAME REINFORCING.

TRASH ENCLOSURE
N.T.S.

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JOB NUMBER
1401
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1st City Comments
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Proj. No. 15-049

TYPICAL FOUNDATION & SITE & MASONRY DETAILS

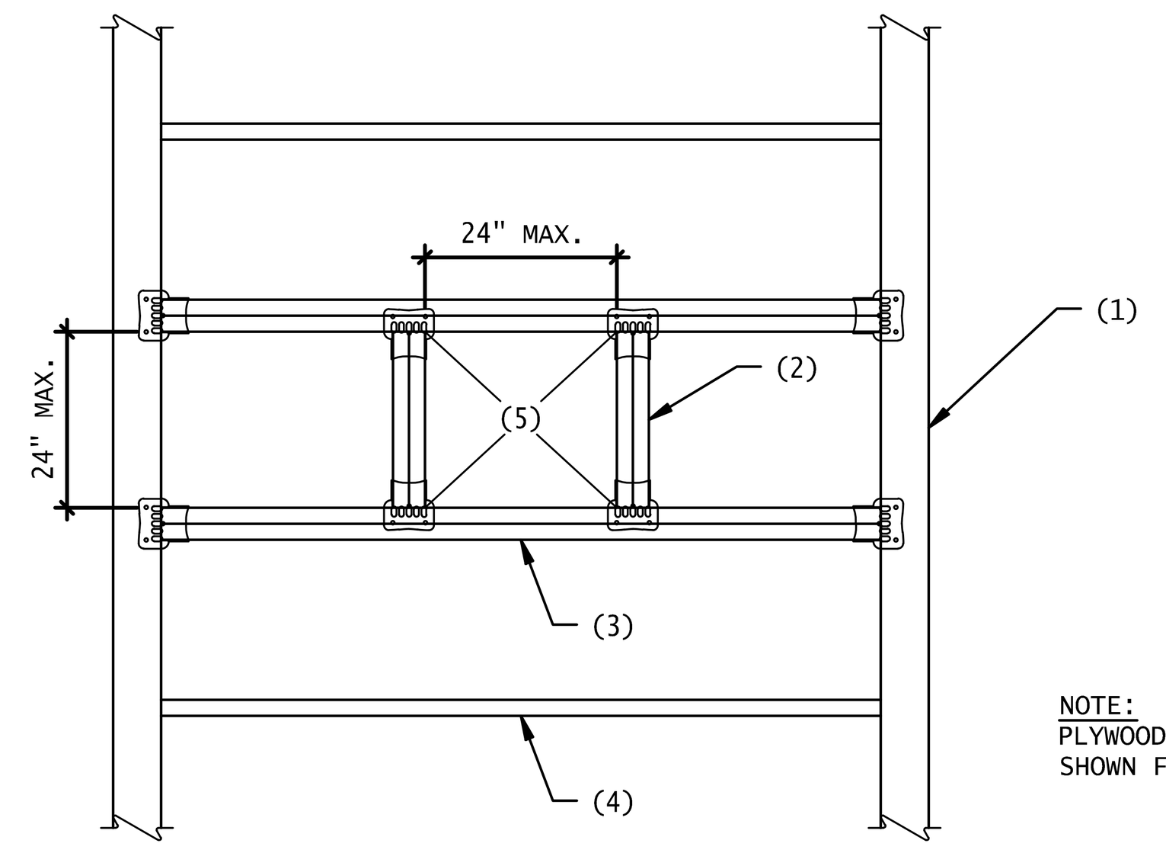
S012

TYPICAL NON- SCHEDULED CEILING JOIST			
NOTES: 1. CEILING JOISTS AT 24" O.C. MAXIMUM. 2. CONTINUOUS 2x4 BRIDGING WITH 1-16d PER JOIST SHALL BE PROVIDED AT INTERVALS NOT TO EXCEED "S" (SPACING). 3. FOR GENERAL NOTES REGARDING LEDGERS, SEE LEDGER SCHEDULE. JOISTS DESIGNED FOR 5 P.S.F. DEAD LOAD AND 10 P.S.F. LIVE LOAD. 4. IF LOADS ENCOUNTERED ARE GREATER, NOTIFY THE STRUCTURAL ENGINEER.			
JOIST SIZE	MAXIMUM SPAN	"S" (SPACING)	LEDGER AT STUD WALL
2x4	8'-0"	2'-9"	2x4 WITH 2-16d NAILS PER STUD.
2x6	14'-0"	4'-8"	2x6 WITH 2-16d NAILS PER STUD.
2x8	18'-0"	6'-0"	2x8 WITH 3-16d NAILS PER STUD.

TYPICAL CEILING JOIST SCHEDULE

N.T.S.

NOTES:
1. WOOD PURLIN.
2. 2-2x6 W/16d NAILS @ 12"O.C., STAGGERED & SIMPSON F26-2 HANGERS EACH END, TYPICAL.
3. DOUBLE JOIST W/SIMPSON F TYPE HANGERS EACH END.
4. WOOD JOIST.
5. OPENING.

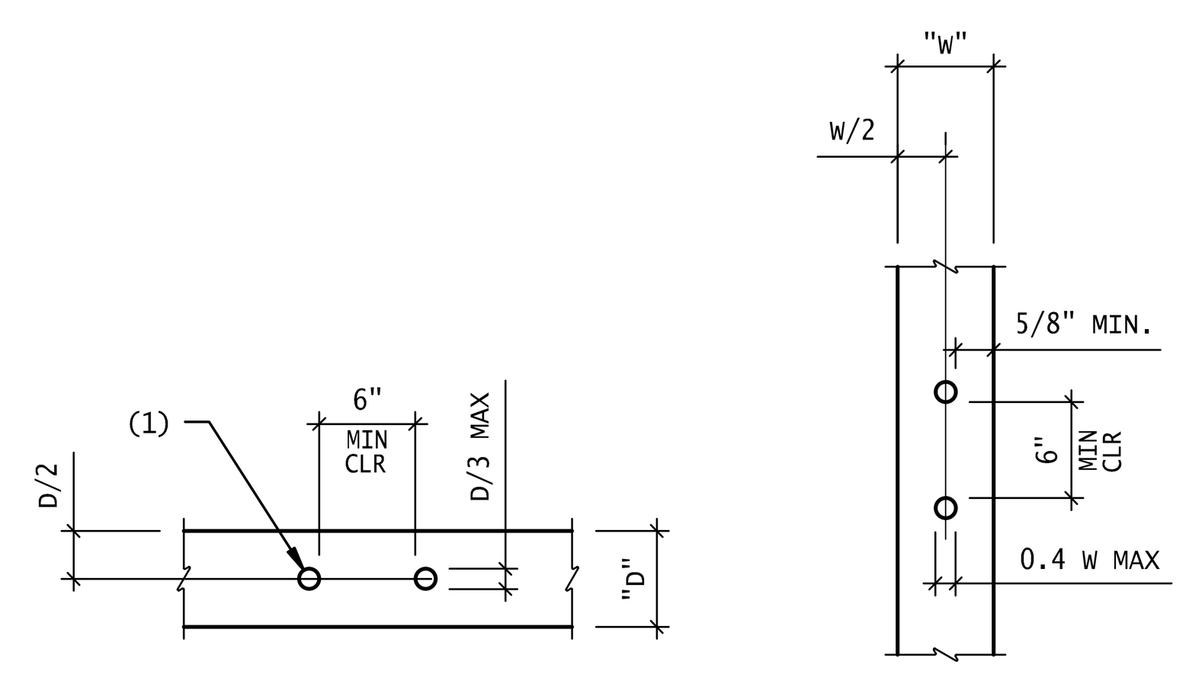


NOTE:
PLYWOOD SHEATHING NOT SHOWN FOR CLARITY.

TYPICAL SMALL ROOF OPENING

N.T.S.

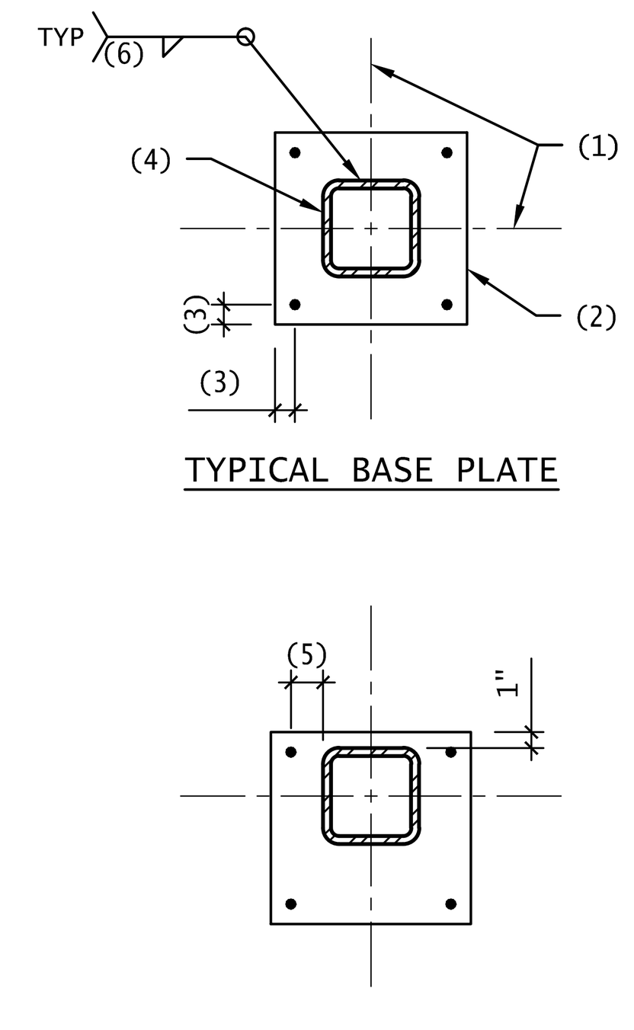
NOTES:
1. DRILLED HOLE, TYPICAL.



NOTE:
1. NOTCHING OF JOISTS, BEAMS, AND STUDS IS NOT ALLOWED.
2. AT JOIST AND BEAMS, HOLES ARE PERMITTED ONLY IN THE MIDDLE 1/3 OF THE MEMBER CLEAR SPAN.

TYPICAL DRILLED HOLES IN WOOD FRAMING

N.T.S.



NOTES:
1. CENTERLINE OF COLUMN AND STEEL BASE PLATE.
2. 3/4"x12"x1'-0" STEEL BASE PLATE.
3. 1 1/4" MINIMUM OR PER A.I.S.C. TABLE J3.5.
4. STEEL COLUMN. SEE SCHEDULE FOR TYPE, SIZE, BASE PLATE AND ANCHOR BOLTS.
5. 2" MINIMUM OR AS REQUIRED FOR WRENCHING CLEARANCE.
6. 1/4" OR MINIMUM SIZE REQUIRED BY TABLE J2.4 OF THE MANUAL OF STEEL CONSTRUCTION - ALLOWABLE STRESS DESIGN, WHICHEVER IS GREATER.

TYPICAL STEEL COLUMN BASE PLATE

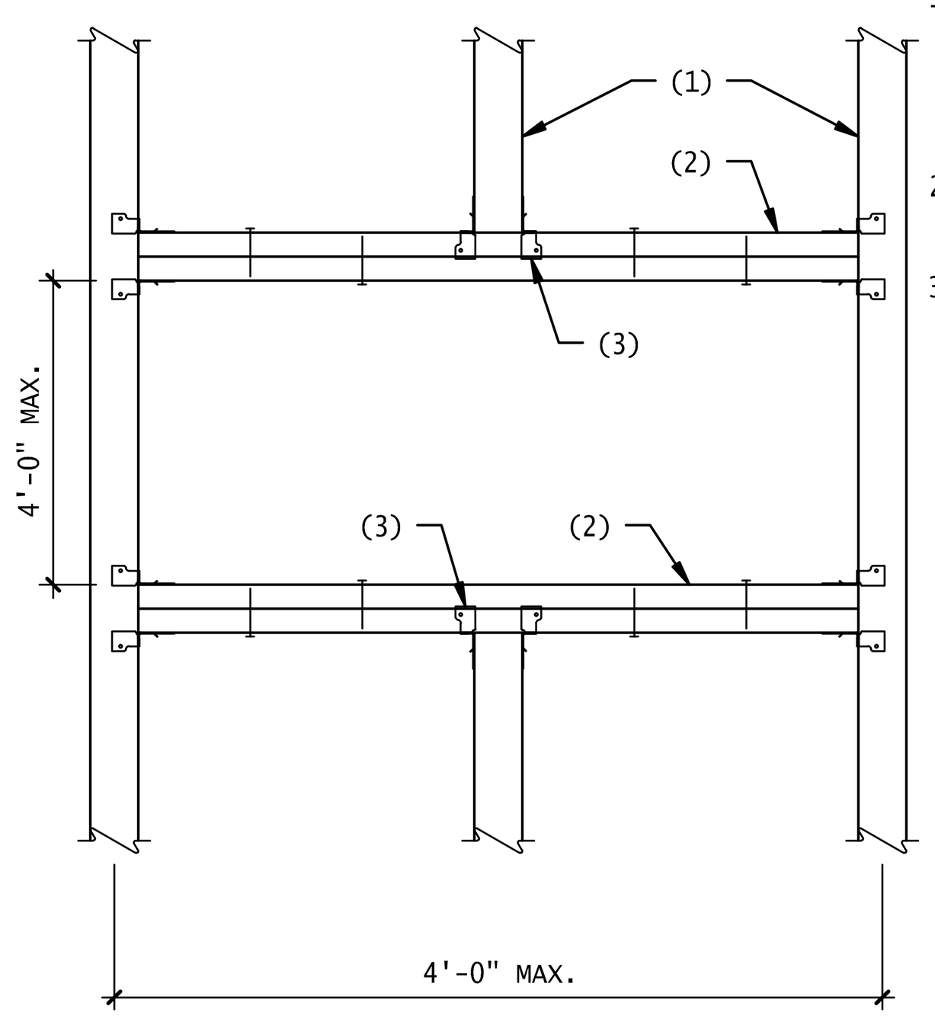
N.T.S.

NOTES:
1. THE TYPICAL CONNECTION CONSISTS OF 3/8" SINGLE SHEAR PLATES WITH 3/4"Ø ASTM A325 BOLTS. USE 5/8" SHEAR PLATES WHERE WEIGHT OF BEAM EXCEEDS 40 LBS.
2. ALL BOLTS SHALL BE INSTALLED USING SHORT SLOTTED HOLES IN EITHER THE BEAM WEB OR THE SHEAR PLATE PER LATEST AISC SPECIFICATIONS.

NOMINAL BEAM DEPTH "D"	NUMBER OF 3/4"Ø ASTM, A325 BOLTS
UP TO 7"	2
8" - 11"	2
12" - 14"	3
15" - 17"	4
18" - 20"	5
21" - 23"	6
24" - 29"	7
30" - 32"	8
33" - 35"	9
36"	10

TYPICAL BOLT SCHEDULE FOR STEEL CONNECTIONS

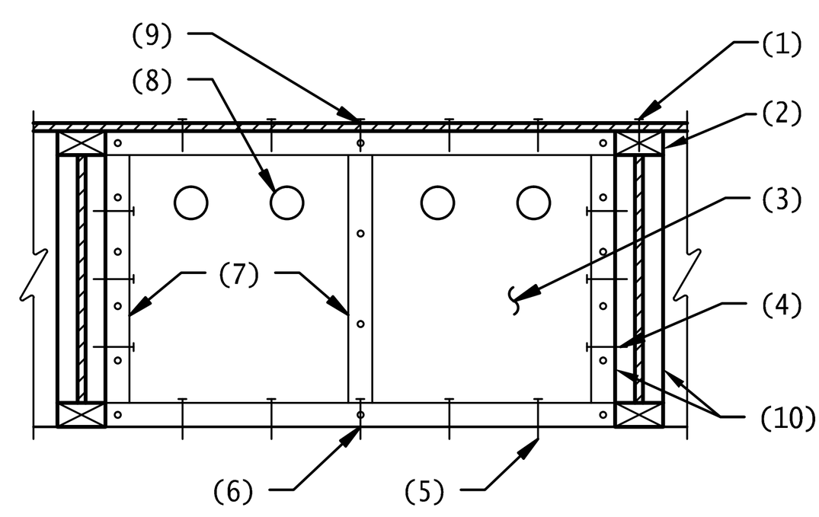
NOTES:
1. WOOD I-JOIST OR PREFABRICATED WOOD TRUSS. MANUFACTURER TO VERIFY CAPACITY FOR MECHANICAL LOADS - MODIFY AS REQUIRED (MAINTAIN DEPTH).
2. DOUBLE 2x8 WITH SIMPSON HUS28-2TF HANGERS EACH END AND 16d NAILS AT 12" O.C.
3. TOP FLANGE HANGERS AS REQUIRED PER WOOD I-JOIST OF PREFABRICATED WOOD TRUSS MANUFACTURER.



TYPICAL LARGE ROOF OPENING

N.T.S.

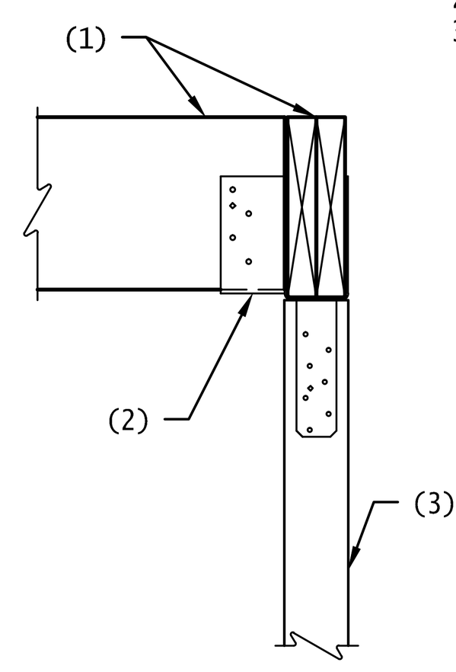
NOTES:
1. EDGE NAILING.
2. WOOD CHORD I-JOIST.
3. 1/2" PLYWOOD SHEAR PANEL WITH 8d AT 6" O.C. AT PANEL EDGES AND 8d AT 12" O.C. AT INTERMEDIATE SUPPORTS.
4. 16d AT 6" O.C. TO JOIST VERTICAL WEB.
5. 16d TO MATCH "EDGE NAILING" SPACING.
6. TOP OF WOOD PLATE OR BEAM AS OCCURS.
7. 2x4 AT FOUR SIDES AND INTERMEDIATE.
8. 2" DIA. VENTILATION HOLES AT 6" O.C. AS OCCURS.
9. PLYWOOD SHEATHING.
10. WEB STIFFENER EACH SIDE.



TYPICAL PLYWOOD SHEAR PANEL

N.T.S.

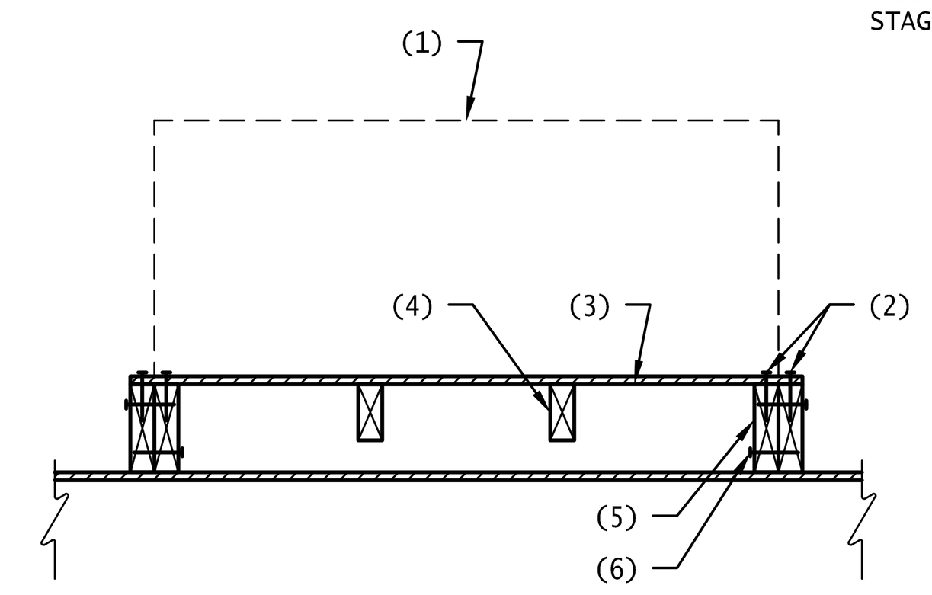
NOTES:
1. WOOD BEAM.
2. SIMPSON ECCLQ POST CAP.
3. WOOD POST.



TYPICAL WOOD BEAM POST CONNECTION

N.T.S.

NOTES:
1. LINE OF MECHANICAL UNIT.
2. DOUBLE EDGE NAILING.
3. PLYWOOD SHEATHING.
4. 2x4 AT 16" O.C. FROM JOIST TO JOIST.
5. DOUBLE 2x6 FROM JOIST TO JOIST.
6. 16d AT 12" O.C. - STAGGERED BOTH SIDES.

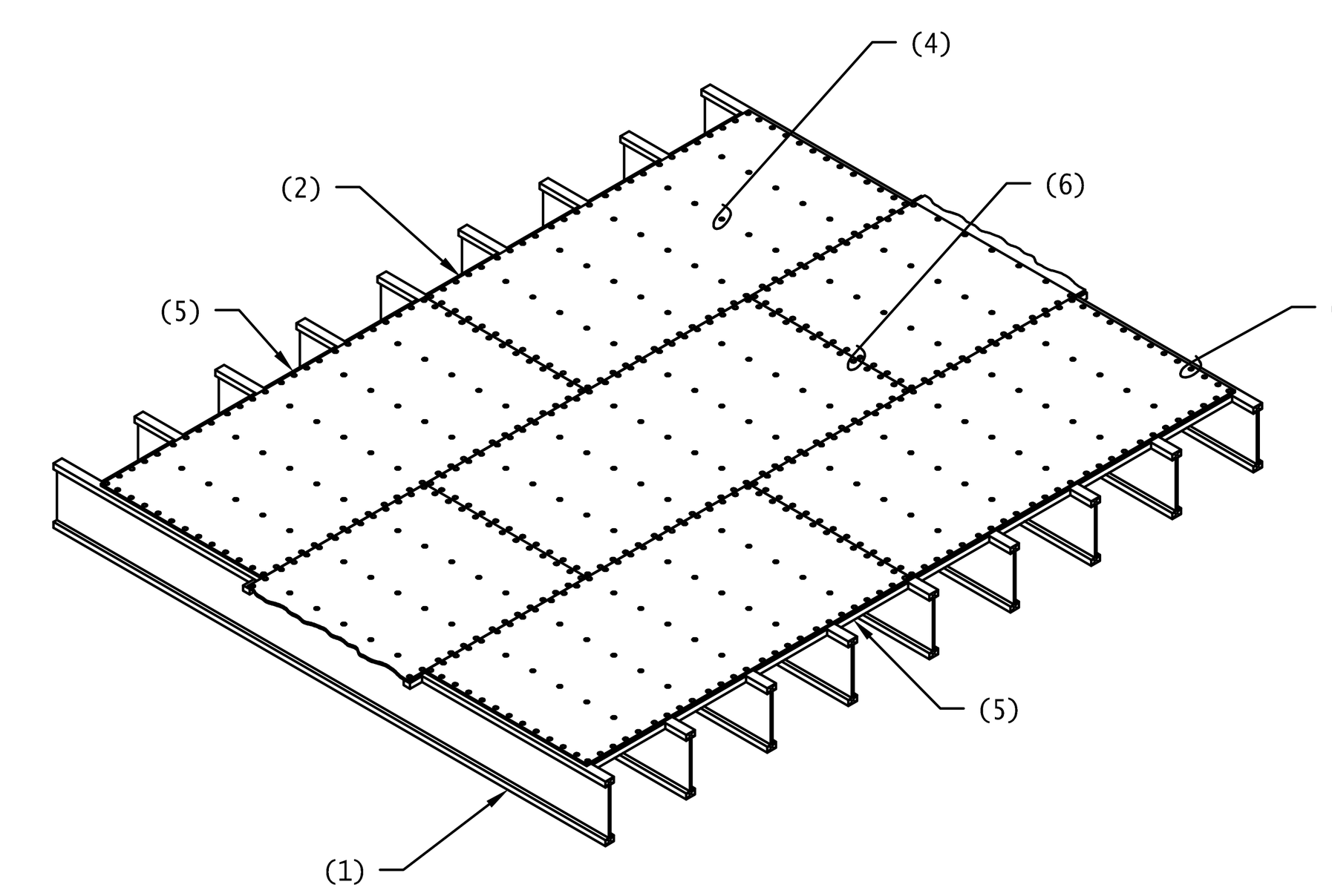


NOTE:
FOR ADDITIONAL INFORMATION, SEE TYPICAL OPENING IN ROOF DETAIL.

TYPICAL MECHANICAL PLATFORM

N.T.S.

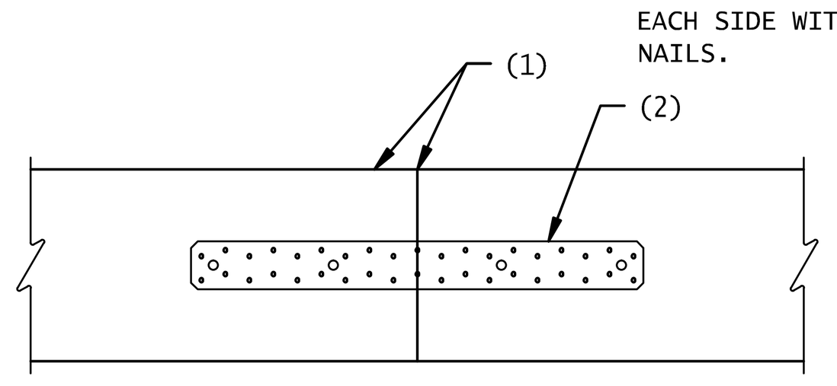
NOTES:
1. WOOD BEAM OR JOIST.
2. PLYWOOD SHEATHING.
3. BOUNDARY NAILING.
4. INTERMEDIATE NAILING.
5. BLOCK ALL EDGES.
6. EDGE NAILING.



TYPICAL SHEATHING ATTACHMENT DETAIL

N.T.S.

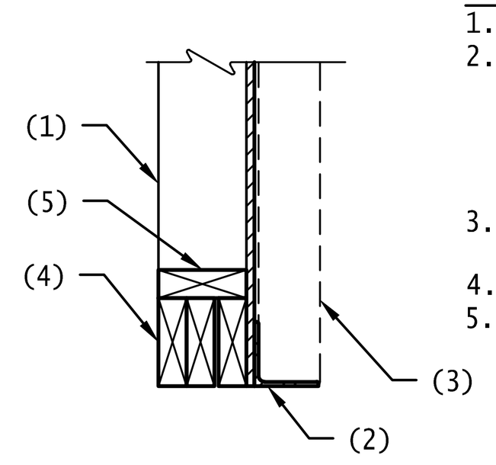
NOTES:
1. iLEVEL RIMBOARD.
2. SIMPSON MSTC28 STRAP EACH SIDE WITH 36 - 16d NAILS.



TYPICAL iLEVEL RIMBOARD SPLICE DETAIL AT ALL EXTERIOR WALLS

N.T.S.

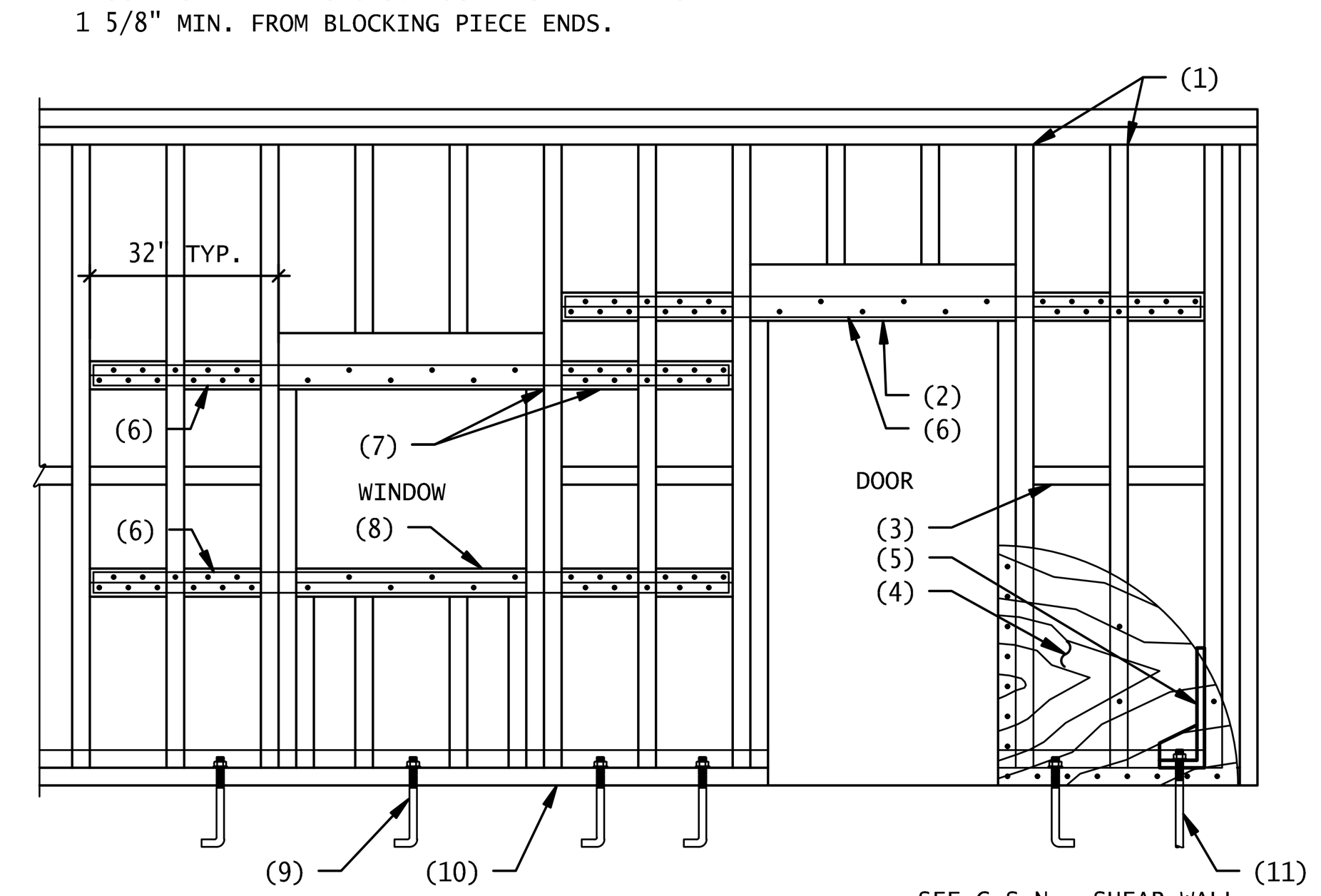
NOTES:
1. WOOD STUD WALL.
2. CONTINUOUS SHELF ANGLE L1 1/2x4x1/8, CONNECT ANGLE TO FACE OF PLYWOOD W/#8 WOOD SCREWS @ 12" O.C. IN PRE-DRILLED HOLES.
3. STONE VENEER PER ARCHITECTURAL.
4. WOOD LINTEL.
5. 2x WOOD PLATE.



STONE VENEER ANGLE AT WOOD STUD WALL DETAIL

N.T.S.

NOTES:
1. 2x WOOD STUDS AT 16" O.C. MAX.
2. WOOD LINTEL.
3. SOLID BLOCKING AT SHEATHING PANEL JOINTS.
4. SHEAR WALL SHEATHING.
5. HOLDOWN.
6. CONTINUOUS CMSTC14 STEEL STRAP NAILED INTO BLOCKING BETWEEN STUDS. LOCATE STRAP NAILS 1 5/8" MIN. FROM BLOCKING PIECE ENDS.
7. ALIGN BOTTOM EDGE OF BLOCKING WITH BOTTOM EDGE OF LINTEL.
8. DOUBLE 2x WOOD SILL WHERE STRAP OCCURS.
9. ANCHOR BOLTS AT FOUNDATIONS/NAILS IN SILL PLATE AT WOOD FRAMING BELOW.
10. SILL PLATE.
11. HOLDOWN ANCHOR.



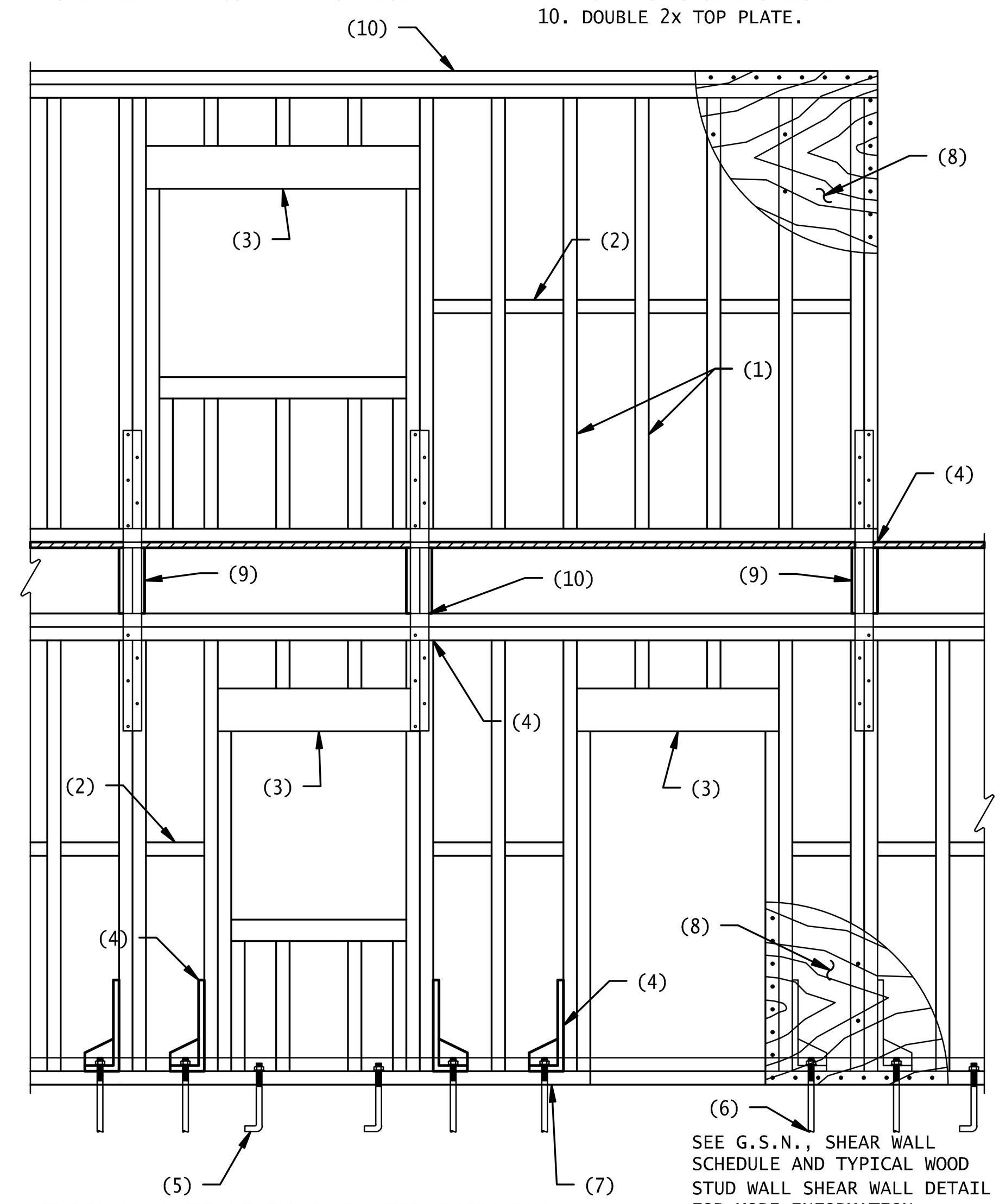
SEE TYPICAL MULTI STORY WOOD STUD SHEAR WALL DETAIL FOR CONNECTIONS BETWEEN FLOORS

SEE G.S.N., SHEAR WALL SCHEDULE AND TYPICAL WOOD STUD WALL EXTERIOR/BEARING DETAIL FOR MORE INFORMATION

TYPICAL WOOD STUD SHEAR WALL

N.T.S.

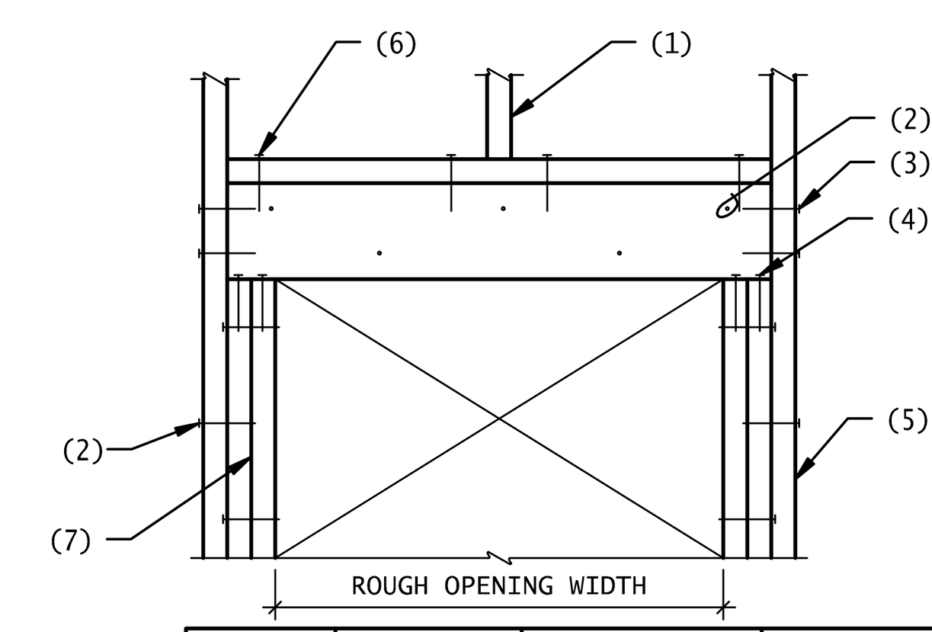
NOTES:
1. 2x WOOD STUDS AT 16" O.C. MAX.
2. SOLID BLOCKING AT SHEATHING PANEL JOINTS.
3. WOOD LINTEL.
4. HOLDOWN.
5. ANCHOR BOLTS AT FOUNDATIONS/NAILS IN SILL PLATE AT WOOD FRAMING BELOW.
6. HOLDOWN ANCHOR.
7. SILL PLATE.
8. SHEAR WALL SHEATHING.
9. TYPICAL AT BUILT-UP STUD POSTS, AT HOLDOWN LOCATIONS, AND AT TRIMMER/KING STUDS OF OPENINGS ABOVE. SQUASH BLOCKING IN FLOOR BEAM SPACE TO MATCH STUDDING ABOVE.
10. DOUBLE 2x TOP PLATE.



SEE G.S.N., SHEAR WALL SCHEDULE AND TYPICAL WOOD STUD WALL SHEAR WALL DETAIL FOR MORE INFORMATION

TYPICAL MULTI STORY WOOD STUD SHEAR WALL

N.T.S.



NOTES:
1. WOOD STUD WALL.
2. 16d @ 12" O.C. STAGGERED BOTH SIDES.
3. 2-16d EACH SIDE AS SHOWN.
4. 2-16d TOENAILS, EACH SIDE, EACH END.
5. FULL HEIGHT JAMB STUDS.
6. 16d @ 12" O.C. @ WOOD LINTEL.
7. STUDS UNDER LINTEL EQUAL TO NUMBER OF JAMB STUDS.

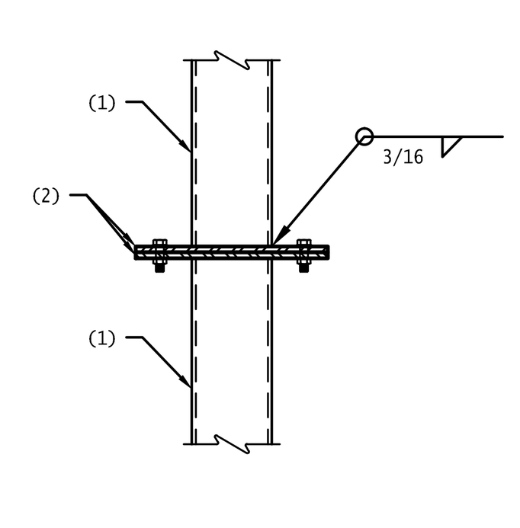
LEVEL	LOCATION	OPENING SIZE	LINTEL SIZE	JAMB STUDS
FIRST FLOOR	CORRIDOR WALLS	0'-0'-3'6"	2-2x10	3-2x4
	DEMISING WALLS	0'-0"-3'-6"	2-2x10	
		3'-7" - 4'-6"	2-2x12	
	EXTERIOR WALLS	0'-0"-3'-6"	5 1/2 x 7 1/2 GLB	3-2x6
		3'-7" - 7'-6"	5 1/2 x 10 1/2 GLB	3-2x6
		7'-7" - 11'-0"	5 1/2 x 15 GLB	4-2x6
SECOND FLOOR	CORRIDOR WALLS U.N.O.	0'-0"-3'6"	2-2x8	2-2x4
	DEMISING WALLS	0'-0"-3'-6"	2-2x10	2-2x4
	EXTERIOR WALLS	0'-0"-3'-6"	3-2x8	2-2x6
		3'-7"-5'-8"	3-2x10	2-2x6
		5'-9"-7'-6"	5 1/2 x 9 GLB	3-2x6
THIRD FLOOR	CORRIDOR WALLS	0'-0"-3'6"	2-2x8	2-2x4
	EXTERIOR WALLS	0'-0"-3'-6"	3-2x8	2-2x6
		3'-7"-5'-8"	3-2x10	2-2x6
		5'-9"-7'-6"	5 1/2 x 9 GLB	3-2x6
FOURTH FLOOR	CORRIDOR WALLS	0'-0"-3'6"	2-2x8	2-2x4
	EXTERIOR WALLS	0'-0"-3'-6"	3-2x8	2-2x6
		3'-7"-5'-8"	3-2x10	2-2x6
		5'-9"-7'-6"	5 1/2 x 9 GLB	3-2x6

2

WOOD LINTEL SCHEDULE

N.T.S.

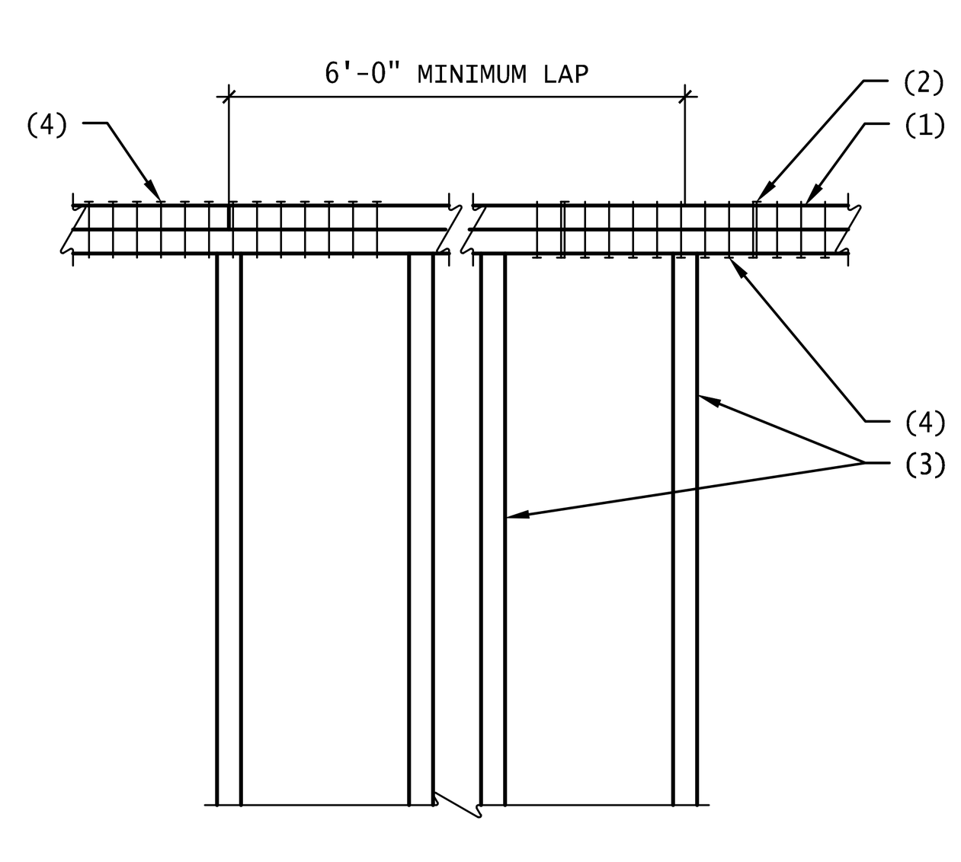
NOTES:
1. HSS5x5x1/4 STEEL COLUMN.
2. PL 3/8"x5 1/2"x12" W/ 4-1/2"Ø BOLTS.



TYPICAL DIVIDER BEAM COLUMN SPLICE DETAIL

N.T.S.

NOTES:
1. DOUBLE TOP PLATE.
2. 16d AT 12" O.C.
3. WOOD STUDS.
4. (16) 16d EACH SIDE OF SPLICE, TYPICAL.



1 TOP PLATE SPLICE

N.T.S.

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JOB NUMBER
1401

DATE
07-23-2015

1 1st City Comments
09-11-2015

2 RFI #1
01-19-2016

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TYPICAL FRAMING DETAILS

S031

3042

REQUIRED VERIFICATION AND INSPECTION OF SOILS (1704.7)

VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. VERIFYY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	X
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	X
3. PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.	-	X
4. VERIFY USE OF PROPER MATERIALS, DENSITIES, AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.	X	-
5. PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	-	X

NOTES:
1. TABLES TAKEN DIRECTLY FROM IBC FOR REFERENCE.

REQUIRED VERIFICATION AND INSPECTION OF EPOXY AND EXPANSION ANCHORS

VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. INSTALLATION OF ANCHORS PER MANUFACTURER'S REPORTS.	X	-

NOTES:
1. TABLES TAKEN DIRECTLY FROM IBC FOR REFERENCE.

REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION (1704.4)

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD (NOTE 2)	IBC REFERENCE
1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.	-	X	ACI 318: 3.5, 7.1-7.7	1913.4
2. INSPECTION OF REINFORCING STEEL, WELDING IN ACCORDANCE WITH TABLE 1704.3, ITEM 5b.	-	-	AWS D1.4 ACI 318: 3.5.2	---
3. INSPECT BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED.	X	-	---	1911.5
4. VERIFYING USE OF REQUIRED DESIGN MIX.	-	X	ACI 318: CH. 4, 5.2-5.4	1904.2.2, 1913.2, 1913.3
5. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	-	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1913.10
6. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	-	ACI 318: 5.9, 5.10	1913.6, 1913.7,1913.8
7. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	X	ACI 318: 5.11-5.13	1913.9
8. INSPECTION OF PRESTRESSED CONCRETE:				
A. APPLICATION OF PRESTRESSING FORCES.	X	-	ACI 318: 18.20 ACI 318: 18.18.4	---
B. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC-FORCE-RESISTING SYSTEM.	X	-	ACI 318: 18.20 ACI 318: 18.18.4	---
9. ERECTION OF PRECAST CONCRETE MEMBERS.	-	X	ACI 318: CH. 16	---
10. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING TENDONS IN POSTTENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.	-	X	ACI 318: 6.2	---
11. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	X	ACI 318: 6.1.1	---

NOTES:
1. FOR SI: 1 INCH = 25.4mm.
WHERE APPLICABLE, SEE ALSO SECTION 1707.1, SPECIAL INSPECTION FOR SEISMIC RESISTANCE.
TABLES TAKEN DIRECTLY FROM IBC FOR REFERENCE.

REQUIRED VERIFICATION OF WOOD CONSTRUCTION (1704.6)

VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. OBSERVE FABRICATION WORKMANSHIP AND COMPLETENESS.	-	X
2. DIAPHRAGM SHEATHING SIZE AND GRADE AND NAILING.	-	X
3. FRAMING MEMBERS, SIZES AND FASTENERS.	-	X

REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION (1704.3)

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD (NOTE 2)	IBC REFERENCE
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS, AND WASHERS:				
A. IDENTIFICATION MARKING TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	X	APPLICABLE ASTM MATERIAL SPECIFICATIONS: AISC 360, SECTION A3.3	---
B. MANUFACTURERS'S CERTIFICATE OF COMPLIANCE REQUIRED.	-	X	---	---
2. INSPECTION OF HIGH-STRENGTH BOLTING:				
A. BEARINGE CONNECTIONS.	-	X	AISC 360, SECTION M2.5	1704.3.3
B. SLIP-CRITICAL CONNECTIONS.	X	X	AISC 360, SECTION M2.5	1704.3.3
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL:				
A. IDENTIFICATION MARKING TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	-	ASTM A 6 OR ASTM A 568	1708.4
B. MANUFACTURERS'S CERTIFIED MILL TEST REPORTS.	-	-	ASTM A 6 OR ASTM A 568	1708.4
4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:				
A. IDENTIFICATION MARKING TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	-	AISC 360, SECTION A3.5	---
B. MANUFACTURERS'S CERTIFICATE OF COMPLIANCE REQUIRED.	-	-	---	---
5. INSPECTION OF WELDING:				
A. STRUCTURAL STEEL:	-	-		
1) COMPLETE AND PARTIAL PENETRATION GROOVE WELDS.	X	-	AWS D1.1	1704.3.1
2) MULTIPASS FILLET WELDS.	X	-	AWS D1.1	1704.3.1
3) SINGLE-PASS FILLET WELDS > 5/16".	X	-	AWS D1.1	1704.3.1
4) SINGLE-PASS FILLET WELDS < 5/16".	-	X	AWS D1.1	1704.3.1
5) FLOOR AND ROOF DECK WELDS.	-	X	AWS D1.3	---
B. REINFORCING STEEL:	-	-		
1) VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706.	-	X	AWS D1.4 ACI 318: 3.5.2	---
2) REINFORCING STEEL-RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAR WALLS AND SHEAR REINFORCEMENT.	X	-	AWS D1.4 ACI 318: 3.5.2	---
3) SHEAR REINFORCEMENT.	X	-	AWS D1.4 ACI 318: 3.5.2	---
4) OTHER REINFORCING STEEL.	-	X	AWS D1.4 ACI 318: 3.5.2	---
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS, AND WASHERS:		X	---	1704.3.2
A. DETAILS SUCH AS BRACING AND STIFFENING.	-	-	---	1704.3.2
B. MEMBER LOCATIONS.	-	-	---	1704.3.2
C. APPLICATION OF JOINT DETAILS @ EACH CONNECTION.	-		---	1704.3.2

NOTES:
1. FOR SI: 1 INCH = 25.4mm.
2. WHERE APPLICABLE, SEE ALSO SECTION 1707.1, SPECIAL INSPECTION FOR SEISMIC RESISTANCE.
3. TABLES TAKEN DIRECTLY FROM IBC FOR REFERENCE.

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Proj. No. 15-049

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1 1st City Comments
09-11-2015

S.S.I. TABLES

S061

3042

TYPICAL WOOD FASTENING

CONNECTION	FASTENING	LOCATION
JOIST TO SILL OR GIRDER	(3) 8d COMMON (2 1/2"x0.131") (3) 3"x0.131" NAILS	TOENAIL
BRIDGING TO JOIST	(2) 8d COMMON (2 1/2"x0.131") (2) 3"x0.131" NAILS	TOENAIL EACH END
SOLE PLATE TO JOIST OR BLOCKING	16d (3 1/2"x0.135") AT 16" O.C. 3"x.131 NAILS AT 8" O.C.	TYPICAL FACE NAIL
TOP PLATE TO STUD	(2) 16d COMMON (3 1/2"x0.162) (3) 3"x0.131" NAILS	END NAIL
STUD TO SOLE PLATE	(4) 8d COMMON (2 1/2"x0.131") (4) 3"x0.131" NAILS (2) 16d COMMON (3 1/2"x0.162) (3) 3"x0.131" NAILS	TOENAIL END NAIL
BUILT-UP STUDS	16d (3 1/2"x0.135") AT 12" O.C. 3"x0.131" NAILS AT 8" O.C.	FACE NAIL
DOUBLE TOP PLATES	16d (3 1/2"x0.135") AT 16" O.C. 3"x0.131" NAILS AT 12" O.C. (8) 16d COMMON (3 1/2"x0.162) (12) 3"x0.131" NAILS	TYPICAL FACE NAIL LAP SPLICE
BLOCKING BETWEEN JOISTS OR RAFTERS TO TOP PLATE	(3) 8d COMMON (2 1/2"x0.131") (3) 3"x0.131" NAILS	TOENAIL
RIM JOIST TO TOP PLATE	8d (2 1/2"x0.13") AT 6" O.C. 3"x0.131" NAIL AT 6" O.C.	TOENAIL
TOP PLATES, LAPS AND INTERSECTIONS	(2) 16d COMMON (3 1/2"x0.162") (3) 3"x0.131" NAILS	FACE NAIL
CONTINUOUS HEADER TWO PIECES	16d COMMON (3 1/2"x0.162")	16" O.C. ALONG EDGE
CONTINUOUS HEADER TO STUD	(4) 8d COMMON (2 1/2"x0.131")	TOENAIL
WOOD JOIST TO PLATE	(3) 8d COMMON (2 1/2"x0.131") (3) 3"x0.131" NAILS	TOENAIL
BUILT-UP CORNER STUDS	16d COMMON (3 1/2"x0.162") 3"x0.131" NAILS	24" O.C. 16" O.C.
BUILT-UP GIRDER AND BEAMS	20d COMMON (4"x0.192") AT 32" O.C. 3"x0.131 NAILS AT 24" O.C. (2) 20d COMMON (4"x0.192) (3) 3"x0.131" NAILS	FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES FACE NAIL AT ENDS AND AT EACH SPLICE

NOTES:
1. COMMON OR BOX NAILS ARE PERMITTED TO BE USED EXCEPT WHERE OTHERWISE STATED.
2. STAPLES SHALL HAVE A MINIMUM CROWN OF 7/16".
3. FASTENING SCHEDULE APPLIES U.N.O., WHERE DIFFERENCES OCCUR, GREATER REQUIREMENT GOVERNS.

WOOD STUD WALL SCHEDULE
ALL STUDS SHALL BE DOUGLAS FIR #1

MARK	NUMBER AND SIZE OF STUDS	SPACING
W1	2-2x6	16" O.C.
W2	2-2x6	24" O.C.
W3	1-2x4	24" O.C.
W4	2-2x4	24" O.C.

2

WOOD JAMB/TRIMMER SCHEDULE

OPENING SIZE	FLOOR	LOCATION	TRIMMER STUDS	JAMB STUDS	REMARKS
ALL (U.N.O.)	ALL	INTERIOR	1 STUD	1 STUD	---
0'-3'-0"	1ST	EXTERIOR	1 STUD	2 STUDS	---
3'-1" TO 7'-0"	1ST	EXTERIOR	2 STUDS	3 STUDS	---
7'-1" TO 12'-0"	1ST	EXTERIOR	3 STUDS	5 STUDS	---
0'-3'-0"	2ND	EXTERIOR	1 STUD	2 STUDS	---
3'-1" TO 7'-0"	2ND	EXTERIOR	2 STUDS	3 STUDS	---
7'-1" TO 12'-0"	2ND	EXTERIOR	2 STUDS	4 STUDS	---
0'-3'-0"	3RD & 4TH	EXTERIOR	1 STUD	1 STUDS	---
3'-1" TO 7'-0"	3RD & 4TH	EXTERIOR	1 STUDS	2 STUDS	---
7'-1" TO 12'-0"	3RD & 4TH	EXTERIOR	1 STUDS	4 STUDS	---

HOLDOWN SCHEDULE

MARK	HOLDOWN TYPE	CONNECTION TO STUDS	CONNECTION AT BASE
HD1	SIMPSON CS14	26 10d NAILS	---
HD2	SIMPSON HDU2	SEE "TYPICAL HOLDOWNS AT WOOD STUD SHEAR WALLS" SHEET S012	SEE "TYPICAL HOLDOWNS AT WOOD STUD SHEAR WALLS" SHEET S012
HD3	SIMPSON CMSTC14	66 16d SINKER	---
HD4	SIMPSON CMSTC16	58 10d NAILS	---
HD5	SIMPSON CS20	12 10d NAILS	---
HD6	SIMPSON CS16	20 10d NAILS	---

COLUMN SCHEDULE

SIZE	BASE CONNECTION	REMARKS
HSS4x4	1/2"x12"x12" BASE PLATE WITH 4-5/8" DIA. x 9" EMBED ANCHOR BOLTS	---
HSS5x5	1/2"x12"x12" BASE PLATE WITH 4-5/8" DIA. x 9" EMBED ANCHOR BOLTS	---
HSS6x6	3/4"x12"x12" BASE PLATE WITH 4-5/8" DIA. x 9" EMBED ANCHOR BOLTS	---
HSS6x3 1/2	3/4"x4"x12" BASE PLATE WITH 4-1/2" DIA. x 9" EMBED ANCHOR BOLTS	---

FOUNDATION SCHEDULE

MARK	SIZE "W" x "L" x "T"	BOTTOM REINFORCING		TOP REINFORCING		REMARKS
		BARS LW	BARS SW	BARS LW	BARS SW	
WF2.0	2'-0"x CONT.x 1'-0"	3-#5	#4@48"O.C.	---	---	---
WF3.0	3'-0"x CONT.x 1'-0"	3-#5	#4@48"O.C.	---	---	---
WF3.5	3'-6"x CONT.x1'-0"	3-#5	#4@48"O.C.	---	---	---
WF4.0	4'-0"x CONT.x1'-0"	13-#6	#6@24"O.C.	---	---	---
WF5.5	5'-6"x CONT.x1'-0"	17-#6	#6@24"O.C.	---	---	---
CF2.0	2'-0"x2'-0"x 1'-0"	3-#5	3-#5	---	---	---
CF4.0	4'-0"x4'-0"x 1'-0"	5-#5	5-#5	---	---	---
CF4.5	4'-6"x4'-6"x 1'-0"	4-#5	---	---	---	---
CF4.0x5.0	4'-0"x5'-0"x 1'-0"	3-#5	5-#5	---	---	---
CF5.0	5'-0"x5'-0"x 1'-0"	5-#5	5-#5	---	---	---
CF6.0	6'-0"x6'-0"x 1'-0"	5-#5	5-#5	---	---	---
CF7.0	7'-0"x7'-0"x 1'-0"	6-#5	6-#5	---	---	

SHEAR WALL SCHEDULE

MARK	SHEATHING MATERIAL AND ATTACHMENT				SILL PLATE ATTACHMENT	
	SHEATHING TYPE	SHEATHING THICKNESS	NUMBER OF FACES	EDGE ATTACHMENT	ATTACHMENT AT FOUNDATION/LEVEL BELOW	ATTACHMENT AT FRAMING
SW1	BLOCKED PLYWOOD	1/2"	1	8d NAILS AT 6" O.C.	---	16d NAILS AT 4" O.C.
SW2	BLOCKED TWO-PLY DRYWALL	5/8"	2	8d NAILS AT 3" O.C. FOR THE BOTTOM & TOP SHEATHING	5/8" A.B. AT 14" O.C. W/10" EMBED	16d NAILS AT 4" O.C.
SW3	BLOCKED DRYWALL	5/8"	2	8d NAILS AT 4" O.C.	---	16d NAILS AT 4" O.C.
SW4	BLOCKED DRYWALL	5/8"	2	8d NAILS AT 6" O.C.	---	#14 x 5" LONG WOOD SCREWS AT 6" O.C.
SW5	BLOCKED DRYWALL	5/8"	1	8d NAILS AT 6" O.C.	---	16d NAILS AT 4" O.C.
SW6	BLOCKED PLYWOOD	1/2"	1	8d NAILS AT 6" O.C.	5/8" A.B. AT 44" O.C. W/10" EMBED	16d NAILS AT 4" O.C.

GENERAL NOTES:
1. SEE TYPICAL DETAILS FOR ADDITIONAL INFORMATION.
2. WHERE SHEATHING IS REQUIRED ON BOTH FACES OF WALL AND NAIL SPACING IS LESS THAN 6" O.C. EACH FACE, PANEL JOINTS SHALL OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS, OR COMMON FRAMING MEMBER SHALL BE 3x OR THICKER AND NAILS ON EACH FACE SHALL BE STAGGERED.
3. WHERE 8d NAILS ARE SPACED AT 2" O.C. OR WHERE 10d NAILS ARE SPACED AT 3" O.C. OR LESS, FRAMING MEMBER SHALL BE 3x OR THICKER AND NAILS SHALL BE STAGGERED.
4. 2 - 2x STUDS STITCH-NAILED WITH 2 ROWS OF 16d NAILS AT 12" O.C. STAGGERED MAY BE SUBSTITUTED FOR 3x STUDS.
5. MAXIMUM STUD SPACING IS 16" O.C.
6. ORIENT PANELS HORIZONTALLY OR VERTICALLY. ALL PANEL EDGES SHALL BE BACKED WITH 2x FRAMING (3x AS REQUIRED). BLOCK BETWEEN STUDS AT HORIZONTAL PANEL EDGES, U.N.O. AT GYPSUM BOARD SHEAR WALLS ONLY, BLOCKING BETWEEN PANEL EDGES IS NOT REQUIRED.
7. EDGE ATTACHMENT SPACING APPLIES TO ALL STUDS AT PANEL EDGES, TOP AND BOTTOM PLATES AND BLOCKING AT PANEL EDGES. LOCATE NAILS 3/8" MINIMUM FROM EDGES.
8. NAILS AT GYPSUM BOARD SHEATHING SHALL BE COOLER OR WALLBOARD NAILS. NAILS SHALL BE COMMON OR GALVANIZED (HOT DIPPED OR TUMBLED) BOX NAILS. #6 TYPE S OR W DRYWALL SCREWS MAY BE SUBSTITUTED FOR NAILS AT GYPSUM BOARD SHEATHING.
9. STAPLES MAY BE SUBSTITUTED FOR NAILS AT GYPSUM BOARD SHEATHING AT SAME ATTACHMENT SPACINGS. STAPLES MAY BE SUBSTITUTED FOR NAILS AT WOOD STRUCTURAL PANELS WHEN NAILS ARE SPACED AT 6" O.C., WITH STAPLES SPACED AT 6" O.C., AT ONE-HALF THE INDICATED EDGE ATTACHMENT SPACING. STAPLES SHALL BE 1 1/2" x 16 GAGE WITH A MINIMUM CROWN WIDTH OF 7/16".
10. SILL PLATE ATTACHMENT AT FRAMING. ALSO APPLIES TO ATTACHMENT OF FRAMING ABOVE WALL TO WALL TOP PLATE, U.N.O.

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Registered Professional Engineer (Structural)
CERTIFICATE NO. 46644
W. DAVID BIXLER
DATE SIGNED 10/20/15
ARIZONA, U.S.A.
Expires 6/30/16

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SCHEDULES

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